

DEVELOPING HOMOGENEOUS SEQUENCES OF RIVER FLOWS AND
PERFORMING COMPARATIVE ANALYSES OF FLOW CHARACTERISTICS

A Dissertation

by

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ABSTRACT

The objectives of the dissertation research are (1) to improve and expand capabilities for developing naturalized or otherwise homogenous river flows representing specified conditions of basin development for input to models for simulating river/reservoir system management and (2) to investigate characteristics of observed, naturalized, and simulated regulated river flows of relevance to water management, especially integration of environmental flow standards in water management in Texas. Key research results and conclusions are as follows:

SWAT was applied to the Sabine, Neches, and GSA River Basins to develop daily and monthly natural-condition flows at relevant sites from available rainfall records. Alternative calibration strategies were developed, tested, and compared. SWAT was combined with the maintenance of variance method, type 2, called MOVE2, to develop a new methodology for filling in gaps of missing naturalized monthly flows. Comparative testing of alternative variations of flow synthesis methods was performed.

The drainage area ratio method for transferring flows for gauged to ungauged sites was investigated and an alternative more refined method proposed. A comparative assessment of alternative methods for transferring flows is presented using available flow data. The drainage area ratio method is shown to be inaccurate for very small or very large drainage area ratios.

SWAT was applied to develop daily flow pattern hydrographs for the Sabine, Neches, and GSA WAMs for use in disaggregating monthly naturalized flows to daily. Various issues were resolved and the daily pattern hydrographs were implemented in the WAMs. Rainfall-runoff modeling with SWAT is demonstrated to be a feasible approach for developing daily pattern hydrographs for the WRAP/WAM models and complexities are explored.

The Dundee Hydrological Regime Alteration Method (DHRAM), which incorporates the Indicators of Hydrologic Alteration (IHA) methodology, was applied to assess the hydrological alterations of daily river flow sequences between user-defined impacted and un-impacted periods in the case study river systems. Frequency metrics for

regulated versus naturalized flows from the WAMs are compared to assess long-term changes in flow characteristics.

Changes in flow characteristics range from being negligible at some sites to very large at other sites. Frequency analysis of unappropriated flows resulting from WAM simulations with versus without Senate Bill (SB3) instream flow standards were performed to evaluate the impacts of the environmental flow standards on water availability. Impacts on unappropriated flows range from no impacts at some sites to very significant impacts at other sites.

DEDICATION

To my beloved wife, Jiyoung Kim, and our sons, Seungu and Seungwan

and to well-beloved my parents, Bunja Woo and Heeki Ryu

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CHAPTER I

INTRODUCTION

Many river/reservoir system management models are reported in the literature and applied by researchers and practitioners worldwide. These models simulate reservoir/river system operations for flood control, water supply, hydropower generation, navigation, recreation, and maintenance of environmental flows. Incorporation of environmental flow considerations in river system management and associated modeling has been growing in importance in recent years in Texas and throughout the world. Input datasets for these computer simulation models include homogeneous sequences of stream flows, representing natural undeveloped conditions or other specified conditions of water resources development, regulation, and use. The stream flow input datasets capture the hydrologic characteristics of a river basin, including severe multiple-year droughts, major floods, and the full range of more normal flow fluctuations. The results of the simulation computations performed by the models include river flows representing another specified scenario of water development, regulation, and use of interest to water managers.

This research combines databases and computational techniques provided by the Texas Water Availability Modeling (WAM) System and Soil and Water Assessment Tool (SWAT) to investigate and improve capabilities for developing and analyzing long (50 years or greater) sequences of homogenous river flows at multiple sites in major river/reservoir systems that represent either natural or specified conditions of river basin development. The WAM System, maintained by the Texas Commission on Environmental Quality (TCEQ), consists of the generalized Water Rights Analysis Package (WRAP) modeling system and WRAP input datasets for the 15 major river basins and eight coastal basins of Texas. The SWAT watershed modeling system develops stream flow sequences based on datasets of precipitation and watershed parameters.

Homogeneous sequences of river flows are fundamental input required for simulating management of river/reservoir systems with WRAP or the many other available river/reservoir system models. The WRAP/WAM and SWAT modeling systems provide

extensive datasets and simulation capabilities providing opportunities to improve and expand capabilities for generating river flow datasets that have broad applicability for other modeling systems as well as improving the WRAP/WAM system.

The research investigates: (1) methods for developing naturalized or otherwise homogenous river flows for input to models for simulating river/reservoir system operations and (2) characteristics of observed, naturalized, and simulated regulated river flows of relevance to water management and modeling thereof. This research on stream flow synthesis and analysis is applicable to all aspects of river/reservoir system management but focuses particularly on integrating environmental instream flow standards in comprehensive water management. The research deals with sequences of monthly and daily flows extending over several decades. The flows of interest are:

- observed actual flows recorded at stream gauging stations
- naturalized flows representing natural river basin conditions without water resources development, regulation, and use
- regulated flows computed in a simulation model representing a specified condition of water resources development, regulation, and use

1.1 Background

River and reservoir system analysis models, such as the WRAP/WAM modeling system, start with homogeneous sequences of monthly or daily stream flow volumes covering a hydrologic period-of-analysis at relevant sites. *Homogeneous* means that the flows represent a specified uniform condition of watershed and river system development, long-term climate, and water use. Non-homogeneities in historical gauged stream flows are typically caused primarily by construction of reservoir projects, growth or changes in water use, and other changes in water management practices over time. However, watershed land use changes, climate changes, and other factors may also affect the stationarity of recorded stream flow measurements.

The stream flows in the TCEQ WAM System WRAP input datasets are naturalized flows representing natural hydrology unaffected by the water resources development and

management activities reflected in the WRAP water rights input datasets. Other terms for naturalized flows found in literature include unregulated, unimpaired, or virgin flows. Alternatively, stream flow inflows input to a river/reservoir system simulation model may represent some other specified homogeneous condition of river basin development. The basic concept is to provide a homogeneous set of flows as model input representing hydrology for a specified set of conditions.

The following aspects of the TCEQ WAM System are particularly relevant to this discussion regarding motivation for the dissertation research:

- The datasets for the individual river basins in the TCEQ WAM System include naturalized monthly flows at a total of about 500 sites, most of which are stream gauging stations. Past observed flows were adjusted by consultants working for the TCEQ during 1997-2000 to remove non-homogeneities caused by human activities at these 500 primary control points.
- These monthly flow sequences at the 500 control points are distributed to over 12,000 other ungauged control points within the WRAP simulations.
- The original TCEQ WAM System datasets include naturalized monthly flows covering hydrologic periods-of-analysis beginning in most cases in 1940 and in all cases between 1934 and 1945. The datasets extend through either 1989, 1996, 1997, 1998, or 2000. The datasets have not been updated due to the difficulty and expense of developing naturalized flows. However, the flow datasets for six case study river basins have been updated by extending the flows through 2013 based on precipitation datasets in research studies at Texas A&M University sponsored by the TCEQ, but the hydrology extensions have not been officially adopted by the TCEQ for the WAM System.
- Establishment of environmental flow standards has driven recent expansions of WRAP and the WAM System. Motivated by expanded needs for modeling sophisticated environmental flow requirements, a daily WRAP modeling system has recently been developed that includes disaggregating monthly flows to daily and flow forecasting and routing.

Historical flows observed at gauging stations are adjusted to develop flow sequences representing natural undeveloped conditions. This requires appropriate methodologies and typically much time and effort. For highly developed river systems, the adjustments to gauged flows to develop naturalized flows may involve significant approximations. Water use data is typically incomplete and approximate. Flow travel times and the effects of seepage, evapotranspiration, and other unmeasured channel losses during the downstream propagation of the effects of diversions, return flows, and reservoir operations are difficult to determine. Stream gauging stations have fixed periods-of-record and often have periods of missing data. The U.S. Geological Survey has discontinued operation of a significant number of gauging stations. Models include a great number of sites that have never been gauged.

The adjustments to gauged flows in the flow naturalization process for essentially all of the TCEQ WAM System hydrology datasets included adjustments for large recorded diversions from surface water sources, return flows from diversions from both ground water and surface water sources, and reservoir evaporation, storage, and releases. Some WAM datasets also include adjustments for spring flows affected by groundwater use. At least one WAM dataset includes naturalized flows reflecting adjustments for land use changes. Numerous relatively small reservoirs, diversions, and return flows were omitted in the flow adjustment process.

Although there are various methods to fill in gaps of missing data, the missing data in the WAM datasets were mainly filled with synthetic data made by linear regression based on flow data sequences for the same periods at gauged sites. In many cases, two or three flow data sequences at gauged sites would be used to fill in gaps of missing data due to the data discontinuation of a significant number of stream gauging stations, and poorly correlated data sequence would be used because there is no highly correlated data sequences around the site that has missing data.

The monthly naturalized flow sequences are distributed to over 12,000 other ungauged control points within the WRAP simulation. The WRAP simulation includes three alternative methods to synthesize the flow sequences at ungauged sites based on the

flow sequences at gauged site (Wurbs, 2006). The drainage area ratio (DAR) methods with or without channel losses are usually used to synthesize the monthly naturalized flow sequences at ungauged sites (secondary control points) based on gauged sites (primary control points). The DAR method is widely used as a linear transfer method throughout the world, but it may lead to bias in the synthesized flow sequences at ungauged sites depending on the drainage area ratio. Thus, methods for removing bias in the DAR method and alternative methodologies have been reported in the literature.

Daily naturalized flow sequences are required for the daily version of the modeling system. Capabilities have been added to WRAP to disaggregate monthly naturalized flows to daily while preserving the monthly volumes based on daily flow pattern hydrographs. Hydrographs of daily flows adopted to define daily flow patterns could be unadjusted observed flows measured at gauging stations, daily gauged flows adjusted to remove the effects of human activity, or daily flows synthesized with a watershed rainfall-runoff model such as SWAT. The research explores situations in which each of these strategies is most appropriate.

Watershed precipitation-runoff models such as SWAT can be applied in alternative ways to accomplish various tasks in developing naturalized flow datasets. SWAT is combined in this research with modeling techniques included in WRAP to assess existing methods and develop expanded capabilities for developing naturalized monthly and daily flows. The applications of the SWAT precipitation-runoff modeling system investigated in the research include:

- synthesizing monthly and daily naturalized flows
- extending the period covered by the existing monthly naturalized flow sequences
- filling in gaps of missing naturalized flows
- synthesizing flows at ungauged sites based on existing naturalized flows at gauging stations
- disaggregating monthly naturalized flows to daily

The daily WRAP modeling system has been recently developed to support incorporation of environmental flow standards into the WAM system. Disaggregation of

monthly naturalized flows to daily is a major focus of the proposed research. Other aspects of analyzing stream flows in support of environmental flow studies are also important. Flow analysis methods are important both in establishing environment flow standards and evaluating the extent to which environmental flow standards can be achieved subject to stream flow availability.

Many rivers have been intensively developed for human requirements without consideration of the various negative impacts on river ecosystems during the last century around the world. This has been driven by population and economic growth in many countries. The extensive ecological degradation and loss of biological diversity resulting from river exploitation have been eliciting widespread concern for conservation and restoration of healthy river ecosystems for several decades among many developed countries (Poff *et al.* 1997). Society is now looking for the balance between economic and ecologic values. Furthermore, the allocation of environmental flow has become a fundamental paradigm in operation and management of a river system. Thus, environmental flow requirements are now legitimately recognized and addressed by establishing standards or related laws in many countries (Sophocleous, 2007).

Scientists face the challenge of how to assess and mitigate the impacts of human activities on river flows and establish and update environmental flow standards under current and future water use conditions. River/reservoir system models provide capabilities for evaluating long-term alterations in river flows. The following modeling and analysis components are essential to addressing the challenge of incorporating environmental flow standards in integrated water management:

- long-term naturalized flow sequences at sites of interest
- river/reservoir system models that incorporate various water management/use scenarios
- environmental flow standards
- strategies to quantify the mitigation effects achieved by environmental flow standards

1.2 Water Availability Modeling (WAM) System

The TCEQ WAM System consists of the WRAP generalized river/reservoir system simulation modeling system developed at Texas A&M University over the past two decades and 20 WRAP input datasets for the 23 river basins of Texas shown in Figure 1.1 originally developed during 1997-2001 by consulting firms working for the TCEQ (Wurbs, 2005). The monthly WRAP/WAM System is routinely applied in administration of the water right permit system and in regional and statewide planning (Wurbs, 2014). Expansion of WRAP and WAM capabilities over the past several years has been motivated largely by environmental flow standards established pursuant to the 2007 Senate Bill 3 (Wurbs and Hoffpauir 2013b). Research and development in expanding the WRAP/WAM System is currently focused on implementing the daily modeling system and integrating environmental flow standards.

1.2.1 WAM Datasets

The 20 WAM datasets covering the 15 major river basins and eight coastal basins of Texas include water rights, other water allocation mechanisms, and other constructed facilities. The hydrologic data include monthly naturalized flow sequences at primary (gauged) control points and net reservoir evaporation-precipitation depths. Periods covered by the monthly hydrologic data are typically from 1940s through late 1990s. WRAP includes methods for generating the hydrologic data at the secondary (ungauged) control points based on flows at the primary control points.

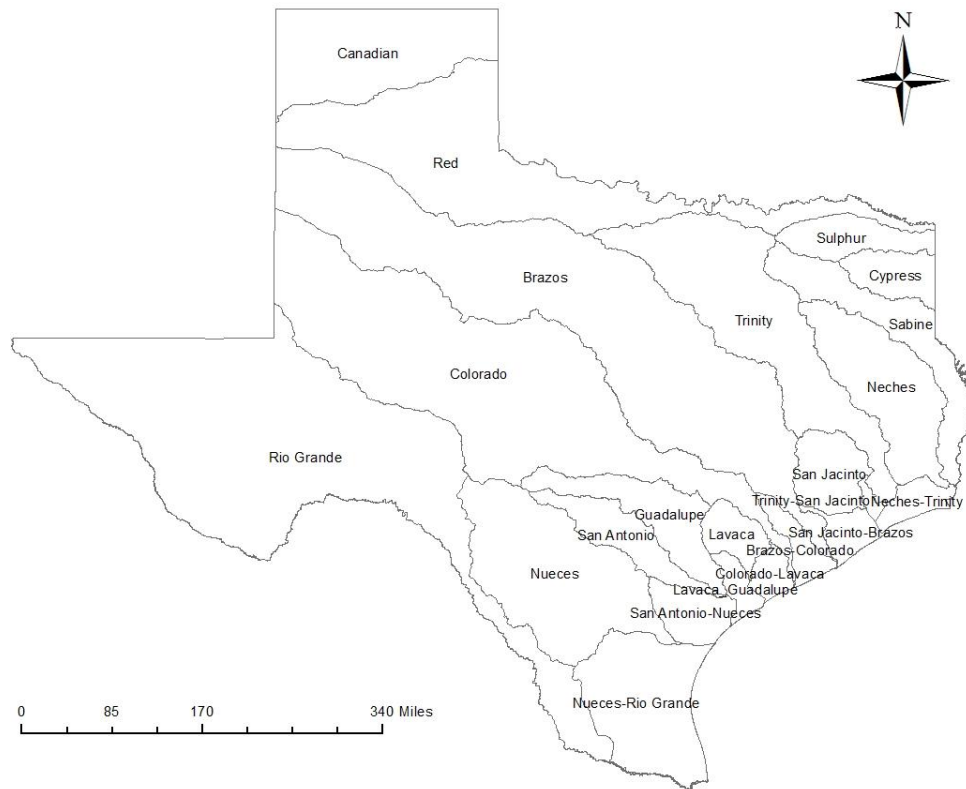


Figure 1.1 WAM System River Basins

There are datasets for two alternative water management scenarios each river basin reflecting combinations of premise regarding water use, return flows, and reservoir sedimentation. The premises of the full authorization scenario are as follows (Wurbs, 2005):

- Water use targets are set at the full amount authorized by the permits.
- Full reuse with no return flows is assumed.
- Reservoir storage capacities are those specified in the permits, which typically reflect no sediment accumulation.
- Term permits are not included

The premises of the current conditions scenario are as follows (Wurbs, 2005):

- The water use target for each water right is set based on the maximum annual amount actually use in any year during a recent 10-year period.
- Best estimates of actual return flows are adopted.
- Reservoir storage capacities and elevation-area-volume relationships for major reservoirs are adjusted to reflect year 2000 conditions of sedimentation.
- Term permits are included.

The TCEQ uses the full authorization scenario in the evaluation of new permanent water right permit or amendments to existing permanent water right permits and the current conditions scenario in the evaluation of term permit application. The period of record, number of primary and total control points, number of water right and instream flow right records, and number of reservoirs in each scenario are listed in Tables 1.1 and 1.2 respectively.

Table 1.1 Texas WAM system Full Authorization Scenario Datasets

Map NO.	River Basin	Original Period of Record	Number Primary Control Points	Total Number Control Points	Model Water Rights (WR/IF)	Model Reservoirs
1	Canadian River Basin	1948-98	12	85	50/0	47
2	Red River Basin	1948-98	47	447	489/103	245
3	Sulphur River Basin	1940-96	8	83	85/5	53
4	Cypress Bayou Basin	1948-98	10	189	163/1	91
5	Rio Grande Basin	1940-00	55	957	2,584/4	113
6	Colorado River Basin and Brazos-Colorado Coastal	1940-98	45	2,395	1,922/86	511
7	Brazos River Basin and San Jacinto-Brazos Coastal	1940-97	77	3,842	1,634/122	678
8	Trinity River Basin	1940-96	40	1,334	1,169/23	703

Table 1.1 (Continued)

Map NO.	River Basin	Original Period of Record	Number Primary Control Points	Total Number Control Points	Model Water Rights (WR/IF)	Model Reservoirs
9	Neches River Basin	1940-96	20	318	333/17	176
10	Sabine River Basin	1940-98	27	376	310/21	207
11	Nueces River Basin	1934-96	41	542	373/30	121
12	Guadalupe and San Antonio River Basins	1934-89	46	1,349	360/184	237
13	Lavaca River Basin	1940-96	7	185	71/30	22
14	San Jacinto River Basin	1940-96	16	411	148/13	114
15	Nueces- Rio Grande	1948-98	29	200	104/8	64
16	San Antonio-Nueces	1948-98	9	53	12/2	9
17	Lava-Guadalupe Coastal	1940-96	2	68	10/0	0
18	Colorado-Lavaca Coastal	1940-96	1	111	27/4	8
19	Trinity-San Jacinto	1940-96	2	94	24/0	13
20	Neches-Trinity Coastal	1940-96	4	245	138/9	31

Table 1.2 Texas WAM System Current Condition Scenario Datasets

Map NO.	River Basin	Original Period of Record	Number Primary Control Points	Total Number Control Points	Model Water Rights (WR/IF)	Model Reservoirs
1	Canadian River Basin	1948-98	12	85	56/0	47
2	Red River Basin	1948-98	47	450	486/110	246
3	Sulphur River Basin	1940-96	8	83	85/5	53
4	Cypress Bayou Basin	1948-98	10	189	159/1	91
5	Rio Grande Basin	1940-00	55	957	2,594/4	113
6	Colorado River Basin and Brazos-Colorado Coastal	1940-98	45	2,396	1,928/93	510
7	Brazos River Basin and San Jacinto-Brazos Coastal	1940-97	77	3,852	1,734/145	711

Table 1.2 (Continued)

Map NO.	River Basin	Original Period of Record	Number Primary Control Points	Total Number Control Points	Model Water Rights (WR/IF)	Model Reservoirs
8	Trinity River Basin	1940-96	40	1,338	1,190/35	709
9	Neches River Basin	1940-96	20	318	317/21	198
10	Sabine River Basin	1940-98	27	375	314/21	206
11	Nueces River Basin	1934-96	41	545	392/32	125
12	Guadalupe and San Antonio River Basins	1934-89	46	1,352	879/202	243
13	Lavaca River Basin	1940-96	7	184	68/30	21
14	San Jacinto River Basin	1940-96	16	413	156/15	114
15	Nueces- Rio Grande	1948-98	29	200	109/8	65
16	San Antonio-Nueces	1948-98	9	53	12/2	9
17	Lava-Guadalupe Coastal	1940-96	2	68	12/0	0
18	Colorado-Lavaca Coastal	1940-96	1	111	27/4	8
19	Trinity-San Jacinto	1940-96	2	94	26/1	13
20	Neches-Trinity Coastal	1940-96	4	245	138/9	31

1.2.2 WRAP Modeling System

WRAP is a generalized river/reservoir simulation modeling system that simulates the development, management, regulation, allocation, and use of the water resources of a river basin or multiple-basin region. Hydrologic and institutional water availability and reliability for water supply diversion, environmental instream flows, hydroelectric energy generation, and reservoir storage can be assessed by WRAP. WRAP models basin-wide interactions among a great number of water users and diverse water management facilities and practices under a variety of water allocation arrangements.

The WRAP modeling system produces sequences of naturalized flows, regulated flows, unappropriated flows, reservoir storage, reservoir net evaporation volumes, incremental change in channel reach losses, water supply diversions, hydroelectric power generated, and other quantities. Simulation results are summarized with frequency

statistics and reliability indices and used for planning and water right regulatory applications.

The newer version of the WRAP modeling system includes features that are currently being implemented as well as capabilities that are routinely applied by agencies and consulting firms in Texas. The TCEQ WAM website provides WRAP input datasets. The WRAP software and documentation are available at <http://ceprofs.tamu.edu/wurbs/wrap.htm>.

The WAM system was originally implemented by the TCEQ, Texas Water Development Board (TWDB), Texas Parks and Wildlife Department (TPWD), and their contractors which consist of two universities and 10 consulting engineering firms pursuant to comprehensive water management legislation enacted by the Texas Legislature in 1997 (Wurbs, 2005) and continues to be maintained by the TCEQ. Applicants or their consultants apply the modeling system in the preparation of water right permit applications, and the TCEQ staffs apply the modeling system in the evaluation of permit applications. The TWDB applies the model to establish regional and statewide water management plans. River authorities and other water management entities also take advantage of the WAM system in endeavors not directly mandated by the TCEQ water right permitting or TWDB planning programs.

Environmental flows have been a key issue in water management and development in Texas for the last several decades. The Texas Instream Flow Program authorized by SB2 enacted by the Texas Legislature in 2001 and expanded by SB3 in 2007 is implemented jointly by the TCEQ, TWDB, and TPWD. The SB2 program includes scientific studies to decide flow conditions necessary for supporting a sustainable ecological environment in the river basins of Texas (TCEQ, TPWD, and TWDB 2008). Environmental instream flow requirements are defined within the framework of subsistence flows, base flows, high flow pulses, and overbank flood events. The daily version of WRAP which is being further expanded to model high flow pulses and overbank flood events was updated to address the issues of the environmental flow requirements (Wurbs and Hoffpauir, 2013b).

1.3 SWAT Modeling System

The Soil and Water Assessment Tool (SWAT) is one of several available generalized watershed models that simulate the hydrologic processes that convert precipitation to stream flow (Singh and Frevert, 2006). SWAT is a public domain computer model developed jointly by the U.S. Department of Agriculture (USDA) Agriculture Research Service and Texas A&M AgriLife Research. The software, user documentation (Neitsch *et al.*, 2011; Arnold *et al.*, 2012a), and a variety of other relevant publications are available at the SWAT website: <http://swat.tamu.edu/>.

SWAT is designed to simulate rainfall-runoff processes and associated erosion, sedimentation, and water quality processes at scales ranging from small watersheds to large river basins. The modeling system has been extensively applied to evaluate the effects of land use, agricultural practices, other land management practices, and other factors on the quantity and quality of watershed runoff in river basins throughout the United States and abroad.

SWAT is a physically-based, semi-distributed, continuous-simulation, daily rainfall-runoff model. SWAT simulates river basin hydrology for each day of a long period that may extend over many years based on rainfall and other input data characterizing climate, land cover, soil, and other watershed conditions. The modeling system provides options for automatically producing parameter values from geographical information system (GIS) data such as digital elevation models (DEMs) and soil and land cover databases. Hydrological response units (HRUs) are the basic computational units in SWAT. The model divides a watershed into subbasins and further divides a subbasin into homogeneous spatial units, called HRUs, characterized by similar soil, land cover, and topographical conditions.

Sequences of daily rainfall at rain gauge sites are provided as input. The SWAT weather generator can be applied to automatically generate daily rainfall and other weather data. Periods of missing rainfall data can be synthesized with the weather generator. The model uses climate data from the rainfall and weather gauge station that is nearest to the centroid of each subbasin. Surface runoff from daily rainfall is calculated by the Natural

Resources Conservation Service (NRCS) curve number method based upon soil, land cover, and antecedent rainfall conditions. Base flows are computed from the interaction among surface, subsurface, and ground water. The surface runoff in a HRU is routed through the river system within a subbasin using the Manning equation. The surface runoff from a subbasin is routed through channels to an outlet point using the variable storage or Muskingum hydrologic routing methods.

SWAT considers various natural hydrologic losses such as evapotranspiration, transmission losses, and infiltration. The model calculates evaporation from soil and transpiration from plants separately. Actual soil evaporation is calculated from exponential functions of soil depth and water content. Plant transpiration is computed by using a linear function of potential evapotranspiration and leaf area index.

Digital elevation model (DEM) data is used by SWAT to delineate watersheds and estimate topographic parameters such as channel length, channel slope, and overland slope. DEM data at a 30-meter resolution can be obtained from the Natural Resources Conservation Service (NRCS) Data Gateway website (<http://datagateway.nrcs.usda.gov/>).

Land cover data necessary for calculating NRCS curve numbers are also obtained from the NRCS Data Gateway website. SWAT uses the general soil map for the United States (STATSGO) to estimate NRCS curve numbers, interaction characteristics between surface and subsurface water, and geological parameters. These data were developed in 1995 and updated in 2006 by the NRCS. The data noted in this paragraph are used in SWAT to determine HRUs.

SWAT applications reported in the literature could be broadly defined as hydrologic only, hydrologic and pollutant loss, or pollutant loss only assessment (Gassman *et al.*, 2007). Hydrologic assessment and prediction are fundamental for all SWAT watershed applications. These applications have performed with some type of graphical and/or statistical calibration and validation for an interest watershed. The regression correlation coefficient (R^2) and the Nash-Stucliffe model efficiency (NSE) coefficient (Nash and Stucliffe, 1970) are most widely used for hydrologic calibration and validation.

SWAT produced reasonable results in the simulation of runoff on a daily, monthly, and annual basis in various basins, however, it is clear that poor results have resulted in parts or all of some studies (Gassman *et al.*, 2007). Inadequate representation of rainfall inputs, due to either a lack of adequate rain gauges in the simulated watershed or subwatershed configurations that were too coarse to capture the spatial detail of rainfall inputs, may lead to poor results for the SWAT model (Gassman *et al.*, 2007). Thus, if there are appropriate rainfall data which are well spatially distributed in the simulated watershed, SWAT model can provide reasonable hydrologic simulation results.

1.4 Literature Review

Key topics are reviewed and related references are cited in the following discussion prior to outlining the research methodology in the last section of this introductory Chapter I.

1.4.1 Naturalized Flow

Naturalized flows represent flows in a river that would have historically occurred exclusive of human influences, that is, without the occurrence of upstream reservoir operations, diversions, and return flows (TNRCC, 1997). Naturalized flows represent natural hydrology unaffected by any human influences such as water resources development and management (Wurbs, 2013a). Alternative terms for naturalized flows are unregulated, virgin, and unimpaired flows. Homogeneous means that the flows represent a specified uniform condition of watershed and river system development, long-term climate, and water use. Naturalized flow datasets have been typically developed with two difference methods: (1) adjusting recorded flows at gauging stations to remove the past impacts of upstream major reservoirs, water supply diversions, return flows from surface and ground water sources, and possibly other factor and (2) synthesizing the datasets from precipitation data with a watershed precipitation-runoff model. Adjusting gauged flows to remove hydrologic effects of human activities is generally more accurate than synthesizing flows with a watershed model (Wurbs, 2013a).

There are few cases reported in the published literature in which naturalized flows have been developed for establishing a very large database like the TCEQ WAM System datasets, probably because these require tremendous time, effort, and funding (Kim and Wurbs, 2010). Monthly naturalized streamflow datasets had been developed for establishing a database of updated streamflow data from 1931 to 2001 for 35 sites in the Red River of the North Basin (Emerson, 2005). These are the recorded data for the entire data-development period for 4 sites, partially recorded data for 10 sites, and no gauged data for 21 sites. Thus, data transfer methods such as a modified drainage area ratio, a maintenance of variance extension type 1, and a water-balance method were used for filling in gaps of missing data and generating synthetic data at ungauged sites based on gauged sites. Naik and Jay (2005) developed monthly virgin flow sequence by adjusting irrigation depletion from the river from 1879 to 1928 at the Dalles station in the Columbia River Basin. Monthly irrigation depletion was estimated based on the irrigation area and a correction factor.

Naturalized flow data have been developed in studies reported in the literature at one or two sites to evaluate regulated effects by human influence such as a dam, diversion, and return flows or to assess the flow records in rivers. The monthly naturalized flows (1883-1992) for the River Thames in the United Kingdom have been developed for assessing the monthly flow record using a unit hydrograph model with effective rainfall (Littlewood and Marsh, 1996). Daily naturalized flow data at two sites (up and downstream) in Peace River in Canada during 1972 to 1996 were developed for evaluating the regulation effects by the W.A.C. Bennett hydroelectric dam using daily recorded data for an upstream site and a hydraulic model for a downstream site (Peters and Prowse, 2001). Daily naturalized flow sequence (1987-2000) at a point in the Han River, South Korea was constructed for evaluating the effects of flow regulation by two major multipurpose dams, Soyangan and Chungju Dams, on the flow regime using SWAT model (Kim et al, 2012). In these cases, precipitation-runoff models were mostly used.

The methodology for developing naturalized flow dataset for the WAM system was established by the Texas Natural Resources Conservation Commission (TNRCC)

(renamed TCEQ in 2003) in 1997 and was published as WAM Technical Paper #1. In this research, the TNRCC recommended the traditional methodology to determine naturalized flow through comparing three different methodologies. The general equation for calculating naturalized flow is:

$$NF = GF + D_h - RF_h + S_h + E_h \quad (1.1)$$

Where, NF is naturalize flow, GF is gauged flows, D_h is diversions, RF_h is return flows, S_h is storage change in a reservoir, and E_h is evaporation.

Naturalized flow files are included in the 20 Texas WAM system datasets available at the WAM web site maintained by the TCEQ (Wurbs, 2005). Consulting engineering companies contracted with TCEQ to construct the WAM datasets with the methodology noted above during 1998-2004 (Wurbs, 2005). The naturalized flow datasets were generally constructed by adjusting recorded data at gauge stations. Gaps of missing data were filled by linear regression based on recorded data at adjacent sites at the same period (Wurbs, 2013a).

Considerable time and effort were required to develop the original long-term sequences of monthly naturalized flows. Updating the datasets to the present time also requires time and costs (Kim and Wurbs, 2010). A hydrologic model (Wurbs, 2013b) for updating the naturalized flow datasets to present using monthly precipitation and reservoir evaporation databases maintained by the Texas Water Development Board (TWDB) was recently developed and applied to update the WAM datasets for the Brazos, Trinity, Colorado, GSA, Neches, and Sabine River Basins.

1.4.2 Methodologies to Fill in Gaps of Missing Naturalized Flows

Filling in gaps of missing data is an important task in developing the sequences of monthly naturalized flows. However, WRAP does not include a routine to assist with this task, though WRAP has options to develop sequences of naturalized flows at gauging stations and to distribute naturalized flows from gauged to ungauged locations (Wurbs, 2013a). There are two typical problems in dealing with discontinuous streamflow data. The first is to select appropriate transfer methods or techniques, and the second is to select

donor stations having a strong correlation with partially recorded data at a target station. The most widely used transfer method of data infilling is linear regression assuming homogeneity (Khalil *et al.* 1998). The linear regression method cannot preserve any particular statistical characteristics of the recorded data while it can provide the best estimate (minimum square error) for each individual streamflow values (Hirsch 1982). To address this problem of the linear regression approach, Hirsch (1982) suggested two different techniques, called maintenance of variance extension type 1 and 2, MOVE1 and MOVE2. These techniques are widely used for reconstructing flow sequences or filling in gaps of missing data.

However, these linear regression models cannot consider seasonality in hydrological data and heterogeneous relationship between both recorded data. These problems have led to the development of transfer techniques that consider relationships among heterogeneous groups of observations (Khalil *et al.* 1998). Time-series methods were suggested as an alternative to linear regression methods through comparison of both methods (Beauchamp *et al.*, 1989). A group-based approach that treats flow data as groups rather than as single-valued records with the techniques mentioned above was proposed for estimating missing values when data are significantly auto-correlated like naturalized flows (Elshorbagy *et al.*, 2000). Khalil *et al.* (1998) proved that the group-based neural network models may infill the missing peak flows and also the moderate flows in retaining relevant properties of the historical streamflow both at the auto-and cross-variate series levels. However, these models have not been widely used due to their complexity, difficulties in application, and the same problem as linear models.

Selecting a donor station with long continuous streamflow records is more important than selecting transfer methods in filling in gaps of missing data in flow sequences (Hirsch 1979 and Harvey *et al.* 2012). This is because missing data at a target station should be filled based on a source of information, temporal variability and hydrologic extremes, of donor stations (Hirsch 1979). However, scientists often face the problem that base stations highly correlated with a target station do not exist around a target station in reality.

1.4.3 Development of Naturalized Flows at Ungauged Sites Based on Flows at Gauged Sites

As WRAP input datasets include sequences of monthly naturalized flows at about 12,400 ungauged sites that are estimated based on the sequences at 500 gauged sites, the accurate synthesizing of naturalized flows at ungauged sites is a difficult challenge (Wurbs, 2006). The WRAP modeling system includes the following three flow distribution methods: (1) drainage area ratio (DAR), (2) modified Natural Resources Conservation Service (NRCS) curve-number based method (NRCS CN method), and (3) a generic equation. Of the three methods, the NRCS CN method was originally recommended as the default (Wurbs, 2006). However, due to difficulties in accurately estimating curve numbers, the DAR method with or without considering channel losses is commonly used in applications of the Texas WAM System. The NRCS CN method can calculate monthly flows at ungauged sites using the relationship between precipitation depth and runoff volumes determined by the ratio of drainage area, annual precipitation, and NRCS curve numbers at the both gauged and ungauged watersheds (Wurbs, 2006).

Nationwide and worldwide, the DAR may be the mostly widely used method for estimating flows at ungauged sites based on flows at gauged sites since no additional information other than the streamflow data at a gauged site and the drainage areas of the gauged and ungauged sites is required (Farmer and Vogel, 2013). However, the quality of synthetic flow sequences by the DAR method may be highly inaccurate if the drainage area ratio is not between about 0.3 and 1.5 (Ries and Friesz, 2000). In order to address this problem on the DAR method, the basic equation of the DAR had been modified with bias correction factor and exponent parameter as followings (Emerson et al, 2005):

$$Q_y = K \left(\frac{A_y}{A_x} \right)^\phi Q_x \quad (1.2)$$

Where Q_y is streamflow at an ungauged site, Q_x is streamflow at a gauged site, A_y is the drainage area of the ungauged site, A_x is the drainage area of the gauged site, K is a bias correction factor, and ϕ is an exponent parameter. The bias correction factor is estimated using the nonparametric method described by Duan (1983), and the exponent parameter

is calculated by a regression equation between flow sequence and drainage area at a gauged site (Emerson et al, 2005).

Another approach is a regional statistical method for estimating flow sequences at an ungauged site based on the flows sequences at gauged sites suggested by Hirsch (1979), is called standardization with mean and standard deviation (SMS) by Farmer and Vogel (2013). SMS is based on hypothesizing that the standardized flows at both an ungauged and a gauged site are approximately equal, and can be expressed as:

$$\frac{Q_x - \mu_x}{\sigma_x} = \frac{Q_y - \mu_y}{\sigma_y} \quad (1.3)$$

Where, Q is stream flow at each subscripted site, μ is mean at each subscripted site, and σ is standard deviation at each subscripted site. Statistical parameters at gauged sites are extracted from recorded data, and statistical parameters at ungauged sites are estimated from regional regression equation between the statistical parameters of flow sequences and regional hydrologic features. Likewise, a streamflow transposition method using the standard deviation with beta coefficient and a regression formula was also proposed based on a linear regression equation without intercept (Gan et al, 1991).

1.4.4 Disaggregation of Monthly Flows to Daily

The disaggregation models have focused on space or time disaggregation and on annual to seasonal or seasonal to sub-seasonal flows (Kumar et al, 2000). These models are typically divided into parametric or nonparametric and stochastic or deterministic approaches at single or multiple sites (Acharya and Ryu, 2014). However, the stochastic approaches should be excluded in this research because disaggregated datasets from monthly to daily are required for a river/reservoir system simulation model.

Kumar *et al.* (2000) presented the disaggregation method for simultaneously disaggregating monthly to daily streamflows at a number of sites based on an index site using an optimization algorithm. Ganju *et al.* (2008) disaggregated monthly to daily streamflows for 1851-1929 by using the unimpaired flows for 1967 to 1987 that is closely related to the monthly hydrograph that will be subject to disaggregation for temporal downscaling of decadal sediment load estimates to a daily interval. A nonlinear

deterministic approach to generate sequences of daily streamflows by taking account of streamflow sequences of successively doubled time resolutions between daily and 16 days were developed by Sivakumar *et al.* (2004). A simple method for streamflow disaggregation from monthly to daily at a target station by using a sources station that is selected based on minimum error criteria, which are calculated regarding to streamflow volume within a three-month time window was proposed by Achary and Ryu (2014). Although numerous methods, algorithms, and techniques have been developed at different times with modifications and improvements reported in the literature, disaggregation of streamflow from monthly to daily time steps is still a challenge. The major constraints consist of intensive computational resources, high dimensionality of the disaggregation problem, degree of feasibility to meet targets, and the uncertainty included in estimating parameters (Nowak *et al.*, 2010).

The monthly WRAP/WAM system was expanded to the daily version primarily to address the issues of the environmental flow requirements in Texas (Wurbs and Hoffpauir, 2013a). It is necessary to incorporate daily flow sequences into the daily simulation. Thus, monthly naturalized stream flows are converted to daily in the daily simulation based on daily flow pattern hydrographs while preserving the monthly volumes. The daily WRAP/WAP system has six options to disaggregate monthly datasets to daily datasets: (1) uniform distribution, (2) linear interpolation, (3) variability adjustment, (4) flow pattern, (5) drainage area ratio transfer, and (6) regression equation transfer (Wurbs and Hoffpauir, 2013a). The flow pattern option is generally used. Pattern Hydrographs have been derived from two alternative sources: (1) observed USGS daily flows, (2) computed unregulated daily flows from a USACE modeling system. For unregulated daily flows, the Fort Worth District of the U.S. Army Corps of Engineers (USACE) has developed reservoir operation models to support operations of USACE reservoirs.

1.4.5 Generalized River/Reservoir System Management Models

Numerous reservoir/river system models have been developed for analyzing river system development and management, and are described in the published literature. The

models continue to be improved and expanded. Although there are many models used for general or specific purposes, this review concentrates on user-oriented generalized modeling systems. User-oriented implies that a model is designed for use by professional practitioners other than the model developers (Wurbs, 2012). Generalized means that a model is designed for applications to a range of concerns dealing with river systems of various configurations and locations, rather than being site-specific customized to a particular system (Wurbs, 2012).

The generalized models combining a specific scenario of water resources development, control, allocation, management, and use with a specific condition of historical river basin hydrology generally track the movement of water through a river system including various man-made river structures based on volume-balance. Historical river basin hydrology is represented by natural unregulated streamflow, and net reservoir surface evaporation-precipitation rates for each time step of a hydrologic period-of-analysis (Wurbs, 2012). The results of a simulation are expressed as reservoir storage and streamflow frequency statistics and water supply reliability metrics for establishing alternative management strategies and practices.

The four modeling systems, ResSim, MODSIM, WRAP, and RiverWare are widely applied in the United States, are also applied in other countries, providing a broad range of analysis capabilities, and are representative of the state-of-the-art from the perspective of practical applications dealing with complex river systems (Wurbs, 2012).

The Hydrologic Engineering Center (HEC) of the US Army Corps of Engineers (USACE) revealed ResSim to the public in 2003. ResSim can simulate multi-purpose, multiple-reservoir systems for real-time decision support of USACE reservoir control personnel. The computational time-step can be selected by a user from 15 minutes to one day.

MODSIM is a generalized river/reservoir system model developed by Colorado State University (CSU) sponsored by United State Bureau of Reclamation (USBR). MODSIM has a graphical user interface and general framework for river/reservoir system

modeling (Wurbs, 2012). A model user can select various time-step from monthly, weekly, and daily for various term planning, management, and operations.

The Center for Advanced Decision Support for Water and Environmental Systems (CADSWES) of the University of Colorado sponsored by USBR and Tennessee Valley Authority (TVA) developed RiverWare. This model can calculate volume balance at reservoirs, hydrologic routing in river reaches, evaporation and other losses, diversions and return flows, and additionally provides optional features, groundwater interactions, water quality, and electric power economics with daily or hourly time-steps (Wurbs, 2012).

The WRAP modeling system (Wurbs 2013a and 2013b) adopted in the TCEQ WAM System and applied in this dissertation research is a generalized river/reservoir simulation modeling system that simulates the development, management, regulation, allocation, and use of the water resources of a river basin or multiple-basin region. Hydrologic and institutional water availability and reliability for water supply diversion, environmental instream flows, hydroelectric energy generation, and reservoir storage can be assessed by WRAP. WRAP may model basin-wide interactions among a great number water uses and diverse water management facilities and practices under a variety of water allocation arrangements. The WRAP modeling system can produce sequence of naturalized flows, regulated flows, unappropriated flows, reservoir storage, reservoir net evaporation volumes, incremental change in channel reach losses, water supply diversions, hydroelectric power generated, and other quantities using input datasets such as naturalized flows, net reservoir evaporation less precipitation rates adjusted for site runoff, and stream flow channel loss parameters. These results are summarized with frequency statistics and reliability indices.

1.4.6 Environmental Flow Standards

Impacts of water resources development on stream flow

Flow regime change is caused by a complex mix of drivers that include climate, topography, land cover, land use, and water resources developments, but water resources

development projects such as the construction of dams, diversion of water supplies for agricultural, industrial, and municipal uses, and land use change have mainly led to serious alterations in rivers that may affect stream ecosystems (Vogl and Lopes, 2009). Water use has sharply increased due to population and economic growth during the last several decades worldwide. Even though the rate of increase recently tends to go down, it was expected that humans will appropriate 75% of the world's accessible surface water by 2025 (Postel *et al.*, 1996). Accordingly, water resources developments are still going on and will be continued due to social and economic requirements around the world. The main impacts by water resources developments on river hydrology represent frequency, magnitude, and shape: annual flood peak, annual flow through, annual flood volume, time to flood peak, flood drawdown time and interarrival time (Singer, 2006). Of the water resources developments, it has been well known that a dam is a representative structure that has great impacts on river hydrology.

The impacts by dams are representatively decreasing the frequency and magnitude of high flow events and stabilizing low flows (Vogl and Lopes, 2009). Peters and Prowse (2001) examined how much flow regulation affects alteration of downstream hydrographs. In their research, it was revealed that a hydroelectric facility and associated reservoir in the Peace River, Canada leads to significant changes to the river hydrograph at even some 1,100 km downstream through comparing the hydrologic characteristics of daily regulated and naturalized flows (Peters and Prowse, 2001). Dams have been found to significantly change hydrologic characteristics in rivers such as minimum and maximum flows through investigating pre- and post-dam hydrologic changes from dams across the United States (Magilligan and Nislow, 2005). Although it is obvious that dams affect flow regimes in rivers, the effect of scale is greatly dependent on the ratio of reservoir capacity to annual flow volume called impounded runoff index (Singer, 2006). These alterations have eventually led to an increase of habitat generalist fish species, a decrease of native obligate riverine fishes, and an overall homogenization of species assemblages (Vogl and Lopes, 2009).

Quantifying long-term changes in flow characteristics due to water resources development is challenging because river flow is very variable, its influencing factors are various, complex and interactive, and available recorded data are very limited.

Environmental flow requirements

Humans have expended great effort to control the dynamics of free-flowing waters for transportation, water supply, flood control, agriculture, and power generation (Poff et al, 1997). However, these efforts have led to extensive ecological degradation and loss of biological diversity, and many rivers could not provide socially valued native species or sustain healthy ecosystems. This recognition naturally led to the concern for conservation and restoration of healthy river ecosystems, and eventually the establishment of the science of environmental flow assessment for determining ecosystem conservation and resources protection. Furthermore, environmental requirement is now legitimately recognized and addressed by establishing standards or related laws in many countries (Sophocleous, 2007).

In the past, only water quantity (minimum flow) and quality were emphasized in the protection of river ecosystems. Environmental flows specified as constant minimum limits are still widely used as an environmental flow standard in many countries. Minimum flows in rivers and streams are specified for providing a certain level of protection for the aquatic environment (Jowett, 1997). However, it is now recognized that environmental dynamism is required to restore and conserve native species diversity and ecological integrity in rivers and other ecosystems (Poff *et al.*, 1997). The natural flow regime is defined as the characteristic pattern of a river's flow quantity, timing, and variability, and can be identified from long-term gauged data (Poff *et al.*, 1997). The National Research Council (NRC, 2006) also recommended that the natural flow regime is a central concept to conserve and restore regulated rivers through environmental flow requirements. NRC developed a range of recommended flows for the low flows in each month, high flow pulse throughout the year, and floods with targeted inter-annual frequencies based on inter-

disciplinary and scientific process, and applied the process to the Savannah River, Georgia, USA (Richter *et al.*, 2006).

Tharme (2003) reviewed the status of environmental flow methodologies identifying some 207 individual methodologies, recorded for 44 countries within six world regions. In his research, environmental flow methodologies were classified into four types, namely hydrological, hydraulic rating, habitat simulation (or rating), and holistic methodologies (Tharme, 2003). Hydrological method uses hydrological data for establishing environmental flow as a percentage of average annual flow or as a percentile from the flow duration curve. Hydraulic rating methods are based on historical flow records and physical dimensions of a river for determining biotopes of a river. Habitat simulation is a methodology using a simulation model based on hydrological, hydraulic, and biological response data, and holistic methodologies are frameworks including all three methodologies (Tharme, 2003).

Protection of environmental instream flows in the river systems of Texas has been a concern in water resource planning, allocation, and management historically, particularly since the 1980's (Wurbs and Hoffpauir, 2013b). The scientific studies were implemented to determine flow conditions that are needed for supporting a sound ecological environment in the river basins of Texas according to SB2 program. A new regulatory approach for protecting environmental flows by the efforts of the state agencies, water managers, stakeholder groups, environmental organizations, and science and engineering community culminating in rules to be administered by the TCEQ was mandated by SB3 (Wurbs and Hoffpauir, 2013b).

Environmental flow standards consist of a set of flow metrics and rules that vary seasonally or by hydrologic condition and by location that govern decisions to curtail junior rights to divert and/or store streamflows (Wurbs and Hoffpauir, 2013b). The SB2 Instream Flow Program and SB3 strategy for establishing standard are based on flow regimes that includes four components: subsistence flows, base flows, within-bank high flow pulses, and overbank high pulse flows (Wurbs and Hoffpauir, 2013b). The primary water management practices for satisfying instream flow requirements are to restrict water

supply diversions and modify reservoir operations. The SB3 instream flow standards in Texas are assigned priorities based on the dates that the standards are established (Wurbs and Hoffpauir, 2013b). The flow standard or regime refer to the environmental instream flow requirements that are specified to be satisfied or at least to be protected from junior water rights.

A Technical Review Group (TRG) established by the TCEQ provided a compilation of information regarding instream flow assessment tools and several conclusions and recommendations (TCEQ, TPWD, and TWDB, 2008). The methodologies dealt with environmental flow needs of riverine ecosystems are the Lyons method, Consensus Criteria for Environmental Flow Needs (CCEFN), Indicators of Hydrologic Alteration (IHA), Hydrology-based Environmental Flow Regime (HEFS), Texas Hydrologic Assessment Tool (TX-HAT), and the Instream Flow Incremental Methodology (IFIM). The Lyons method and CCEFN provide criteria for determining minimum instream flow limits, the IHA, HEFS, and TX-HAT methods may be used in quantifying instream flow requirement through computing an array of streamflow statistics, and the IFIM is a comprehensive approach using field studies and a set of multiple and analysis tools (Wurbs and Hoffpauir, 2013b).

1.4.7 Incorporating Environmental Instream Flows in WAM System

Recent additions to WRAP include capabilities for estimating impacts on water availability for other water users as well as to evaluate capabilities for satisfying environmental instream flow needs (Wurbs and Hoffpauir, 2013b). Environmental instream flow standards can be incorporated in the simulation model using various optional features. Frequency metrics for environmental flow targets and shortages as well as the frequency and characteristics of high pulse flow events can be analyzed using routines in a WRAP post-simulation program.

1.4.8 Methods for Evaluating Changes in River Flow Regime

Numerous studies have been investigated for quantifying impacts on stream flow changes by water resources developments based on statistical analyses comparing gauged streamflow data before and after or with and without the developments. Long-term changes in flow characteristics of streamflow data have been evaluated with statistical analysis based on monthly or daily gauged streamflow data, and the index for evaluating flow characteristics are definitely different depending on data time steps.

The Indicators of Hydrologic Alteration (IHA) method was proposed for assessing the degree of hydrologic alteration attributable to human influence (Richter *et al.*, 1996). IHA statistics are grouped into 5 groups that represent flow regime characteristics, and the 5 groups are further classified into 32 hydrologic parameters. Richter *et al.* (1998) suggested the Range of Variability Approach (RVA) method based on natural variability in streamflow characteristics using the IHA method for flow management or restoration. The RVA target can be generally made based on selected percentile levels or a simple multiple of the parameter standard deviations for the natural or pre-development stream flow regime (Richter *et al.*, 1998).

The Dundee Hydrological Regime Alteration Method (DHRAM) was developed to quantify the changes of hydrologic characteristics by water resources development based on the 32 hydrologic parameters of the IHA method (Black *et al.*, 2005). DHRAM can classify the degree of hydrologic alteration into five groups (class 1: un-impacted and class 5: severely impacted condition) using a score (from 0 to 30) yielded by DHRAM based on the overall percentage of changes in the parameters of the IHA method before and after streamflow regulation (Black *et al.*, 2005).

The nondimensional metrics of ecodeficit and ecosurplus based on Flow Duration Curve (FDC) were introduced for evaluating hydrologic alteration in rivers (Vogel *et al.*, 2007). In comparing both FDC of unregulated and regulated flows, the area below the unregulated FDC represents ecodeficit, and conversely, the area above the unregulated FDC represents ecosurplus. Likewise, the metric of ecodeficit is defined as the ratio of the area over the total area under the FDC of unregulated flow, and the metric of ecosurplus

is conversely defined as the ratio of the area over the total area (Vogel *et al.*, 2007). Gao *et al.* (2009) developed representative indicators hydrologic alteration based on the nondimensional metrics of ecodeficit and ecosurplus, and proved their availability through principal component analysis of the hydrologic parameters of the IHA method.

Regulation effects on the lower Peace River, Canada were evaluated based on comparing statistical parameters of annual, 1-day, 15-days, and 30-day high indices determined from daily regulated and unregulated flow data (Peters and Prowe, 2001). Changes in hydrologic regime by dams were accessed using the IHA method, and in the research, the serious alterations of hydrologic regime are minimum and maximum flows over different duration (Magilligan and Nislow, 2005). The influence of major dams on hydrology in the Sacramento River Basin, California, USA was evaluated by comparing pre- and post-dams flows with respect to hydrograph characteristics representing frequency, magnitude and shapes based on daily gauged data at 10 gauging stations, located at the downstream of dams (Singer, 2006). Kim *et al.* (2012) assessed flow regulation effects by major dams in the Han River, Korea by comparing FDC of regulated and unregulated flows, generated by the SWAT model.

Lajoie *et al.* (2007) compared monthly flow characteristics including monthly maximum and minimum, inter-annual variability, magnitude and inter-annual variability, frequency, and the coefficients of skewness and kurtosis between natural rivers and regulated rivers based on watershed size, using regression analysis. Impacts of water resources development on flow regimes in the Brazos River, Texas, USA was evaluated based on the monthly flow data of pre- and post-dams (Vogel and Lopes, 2009). In their research, they compared the statistics of annual and monthly data using K-S test.

In the WAM system, frequency metric serves as evaluating capabilities for river system to provide the flow regimes needed for the environment and flow regime alteration by water resources development, and reliability metric is used in evaluating the impact of environmental flow standards on various existing water users (Wurbs and Hoffpauir, 2013b). Trend analysis and flow duration curves are also useful for comparing flow characteristics of naturalized and regulated flows (Wurbs and Hoffpauir, 2013b).

1.5 Objectives, Scope and Organization of the Research

The objectives of the research are to:

1. Perform literature review and case study comparative evaluations of methodologies that have been applied in the past or could be applied in the future in accomplishing the following tasks.
 - computing sequences of homogeneous monthly or daily flows from either observed flows or precipitation
 - filling in gaps of missing data
 - updating flow sequences by extending the period covered to near the present
 - synthesizing flows at ungauged sites based on flows at gauged sites
 - disaggregation of monthly flows to daily
2. Develop expanded or improved capabilities for performing the tasks listed above.
3. Perform comparative analyses of flow characteristics that explore key differences between observed, naturalized, and simulated regulated flows that are relevant in modeling reservoir/river system management and especially in establishing environmental flow standards.

The datasets and modeling and analysis tools provided by the WRAP/WAM and SWAT modeling systems are applied in achieving these objectives. Other methods reported in the literature are also explored. The potential for expanding capabilities by adopting watershed precipitation-stream flow modeling is a key focus. The Sabine, Neches, and Guadalupe-San Antonio (GSA) River Basins serve as the primary case studies. Other river basins in Texas are investigated for particular aspects of the research. The research findings are applicable to river/reservoir system modeling in general as well as to the SWAT and WRAP/WAM modeling systems in particular.

The research is designed to support modeling of multiple-purpose reservoir/river system operations in general. However, environmental flow aspects of river basin management and associated modeling are of particular interest.

Chapter II describes the methodologies adopted for this research. Section 2.1 focuses on a method for infilling missing naturalized flows using the combination of the SWAT model and a transfer method. Section 2.2 describes a new approach to obtain the parameters for the drainage area ratio method and a method to remove transfer biases on the regional statistic. The methodology for disaggregation of monthly to daily naturalized flow sequences is described in Section 2.3. The section includes the strategies for the SWAT model calibrations and flow pattern evaluations. Section 2.4 introduces a strategy to evaluate environmental flow standards based on the simulation outputs of the daily WRAP model.

The WAM datasets for the Sabine, Neches, and GSA River Basins serve as the case studies for this research. Chapter III contains the basin descriptions and WAM datasets for the basins. The primary WAM water use scenario employed for this research is the full authorized use scenario.

Chapter IV introduces a proposed method for infilling missing naturalized flows. This chapter is organized into sections that describe a procedure, data used, the calibration strategy of the SWAT model, and MOVE2 method, respectively. Section 4.5 shows how much the method improves the results through comparative analyses.

Chapter V focuses on developing naturalized flows at ungauged sites based on flows at gauged sites. The procedures to optimize the correction factor and exponent for the drainage area ratio method are introduced. A new approach is proved through the comparative analyses in section 5.2. Section 5.3 demonstrates how to remove transfer biases in the regional statistical method through the case studies.

Chapter VI presents a new method to generate simultaneously daily flow patterns at multiple sites using the SWAT model. This chapter contains the watershed delineation methods, model calibration strategies with the monthly or daily flow datasets, and comparative evaluation of disaggregated daily flows. The selected disaggregated flow

datasets are finally used as input data for deriving routing parameters and daily streamflow patterns for the simulation studies in Chapter VII.

Chapter VII is organized into sections that individually focus on SB3 environmental flow standards for each WAM, environmental flow standards modeling strategies, their target at the control points based on the simulation results.

Chapter VIII investigates sequences of daily observed flows during historical periods prior to significant water resources development versus during more recent periods reflecting development and naturalized versus regulated flows from the daily WAMs at USGS gauging stations for Sabine, Neches, and Guadalupe and San Antonio River Basins.

Chapter IX focuses on evaluating environmental flow standards through the simulation studies using the daily WRAP model. Three case studies are performed with emphasis on hydrological regime alteration on river flows due to human influences and evaluation of their roles and influences on river flows.

The summary and conclusion of the research are presented in Chapter X. The methods and strategies developed by this research with respect to the effectiveness in addressing the key issues described in this Section are firstly summarized. The findings of the case studies through evaluating environmental flow standards are also summarized to provide feedback and guidance for modifying the existing environmental flow standards in the basins or establishing the new standards in other basins.

CHAPTER II

RESEARCH METHODOLOGY

This research includes the following component tasks corresponding to the research objectives discussed in the preceding Chapter I:

- developing a method for synthesizing missing monthly naturalized flows data using the SWAT model, including both filling in gaps in missing data and updating or extending sequences to the present
- suggesting a method to remove the bias on the drainage area ratio in transferring monthly naturalized flow sequences to ungauged sites based on flows at gauged sites
- developing a strategy for developing daily flow patterns simultaneously at multiple sites using the SWAT model that preserves monthly naturalized flow volumes while modeling temporal and spatial characteristics of daily natural flows
- exploring strategies for quantifying the effects of development on stream flow that are useful in studies of environmental instream flow requirements
- evaluating the effect on regulated and unappropriated flows in the case study river basins of the environmental flow standards recently established by the TCEQ through the Senate Bill 3 (SB3) process.

The Sabine, Neches, and Guadalupe-San Antonio (GSA) River Basins serve as the primary case studies, but the Trinity River Basin is also adopted to various extents in certain parts of the research. The Neches and Sabine River Basins represent conditions of high rainfall, forested watersheds, minimal surface/groundwater interactions, and relatively low population density. The GSA River Basin is characterized by lower rainfall, major surface/groundwater interactions, and much higher population densities.

2.1 A Proposed Method for Infilling Missing Naturalized Flows Using the SWAT Model

A method is developed in this research for filling in missing naturalized flows using the SWAT model. The method is applicable both to filling in periods of missing data associated with gaps in historical gauge records and to updating the hydrologic simulation period covered by the WAM datasets to the present. The process for filling in missing data consists of three steps. The first step is to adjust partially recorded data to naturalized data by removing human impacts. The second step is to develop a SWAT model to synthesize naturalized streamflow data after calibration with the available naturalized flow for the partial period. The third step is to fill in gaps in the missing data using a linear transfer method with the streamflow data for the full period generated with the SWAT model.

2.1.1 Transfer method

The models that can consider heterogeneous relationships between base and target stations are not necessary because the monthly streamflow sequence generated by the SWAT model is homogeneous, and the partially recorded streamflow data are also adjusted by removing human impacts prior to the application of this procedure. Thus, linear models can be utilized as a transfer method. While the regression can provide the best estimate (minimum square error) for each individual value, it cannot preserve any particular statistical characteristics of the record data (Hirsch 1982). Thus, Hirsch (1982) suggested that maintenance of variance extension, type 2 method (MOVE2) is the most effective of four linear methods in terms of infilling missing data in preserving the statistical properties of the recorded data they are intended to represent. The MOVE2 method is

$$\hat{y}(i) = \hat{m}(y) + \frac{\hat{s}(y)}{s(x)} [x(i) - m(x)] \quad (2.1)$$

In order to estimate the unbiased sample mean and standard deviation, the parameters $\hat{m}(y)$ and $\hat{s}(y)$ developed by Matalas and Jacobs (1964) were used as

$$\hat{m}(y) = m(y_1) + \frac{N_1}{(N_1 + N_2)} r \frac{s(y_1)}{s(x_2)} (m(x_2) - m(x_1)) \quad (2.2)$$

$$\hat{S}^2(y) = \frac{1}{N_1+N_2-1} \left\{ (N_1-1)S^2(y_1) + (N_2-1)r^2 \frac{S^2(y_1)}{S^2(x_1)} S^2(x_2) + (N_2-1)\alpha^2(1-r^2)S^2(y_1) + \frac{N_1N_2}{(N_1+N_2)} r^2 \frac{S^2(y_1)}{S^2(x_1)} (m(x_2) - m(x_1))^2 \right\} \quad (2.3)$$

$$\alpha^2 = \frac{N_2(N_1-4)(N_1-1)}{(N_2-1)(N_1-3)(N_1-2)} \quad (2.4)$$

Where,

i is an index of time,

$m(x)$ is the sample mean of recorded data at the base station

$m(y_1)$ is the sample mean of recorded data at the target station

x_1 is the flow values for recorded period of the target station at the base station,

y_1 is the flow values for recorded period at the target station,

x_2 is the flow values for target period at the base station,

N_1 is the number of recorded period of the target station,

N_2 is the number of target period at the target station,

$m(x_1)$ is the sample mean of recorded data of the target station at the base station,

$m(x_2)$ is the sample mean of recorded data for the target period at the base station,

$S(y_1)$ is the sample deviation of recorded data at the target station,

$S(x_2)$ is the sample deviation of recorded data for target period at the base station,

r is product moment correlation coefficient between the recorded data at the base and target stations.

2.1.2 Monthly SWAT Model

The SWAT model can perform well in predicting monthly streamflow rates (Gassman, 2007), and this has been successfully proved in the literature (Gassman, 2007 and 2014 and Douglas-Mankin *et al.*, 2010). Thus, the SWAT model can produce the monthly streamflow sequence that is strongly correlated with the streamflow sequence at a target station if there is partially recorded streamflow sequence at the station. Parameter calibration is a key process to enhance the accuracy of the SWAT model in most applications if accurate known streamflow data are available for calibration. The

calibrated parameters for the three case study river basins are selected based on expert judgement and on the recommendations from related literature as listed in Table 2.1.

The parameters are calibrated with monthly naturalized flow data, generated based on recorded data. The SWAT-CUP (2012 version), semi-automated calibration model is used for calibration with the sequential uncertainty fitting algorithm (Abbaspour *et al.*, 2004), and the coefficient of determination (R^2) is used as the objective function to get the most linear correlated synthesized flow sequences with naturalized flow data.

Table 2.1 Calibrated Parameters for SWAT Models

Classification	Parameters
Effective Rainfall	CN
Ground Water	Alpha_BF
	GW_Delay
	GWqmn
	Revapmn
	GW_Revap
	RCHARG_DP
Soil Water Content	Sol_AWC
	Sol_K
Evapotranspiration	ESCO
	EPCO
Routing	Surlag (surface, HRU)
	CH_N2 (Channel, subbasin)
Channel Loss	CH_K2

2.2 Refinement of Methods for Developing Naturalized Monthly Flows at Ungauged Sites Based on Flows at Gauged Sites

2.2.1 Drainage Area Ratio Method

The drainage-area ratio method with or without considering channel losses commonly is used to develop naturalized monthly flow sequences at secondary control

points based on naturalized monthly flow datasets at primary control points in the WRAP/WAM model as follows (Wurbs, 2006):

$$Q_{ungauged} = R_{DA} Q_{gauged} \quad (2.5)$$

$$R_{DA} = \frac{DA_{ungauged}}{DA_{gauged}} \quad (2.6)$$

Equation (2.5) may optionally be changed to Equation (2.7 or 2.8) in situations where the ungauged site is located upstream of the gauged sites with channel losses occurring between the sites at rates that are significantly greater than the loss rates in the watershed above the ungauged sites (Wurbs, 2006).

$$Q_{ungauged} = R_{DA}(Q_{gauged} + F_{CL}Q_{ungauged}) \quad (2.7)$$

or

$$Q_{ungauged} = Q_{gauged} \left(\frac{R_{DA}}{1 - R_{DA}F_{CL}} \right) \quad (2.8)$$

Emerson *et al.* (2005) suggested the following general equation in order to improve the accuracy of the DAR method:

$$Q_y = Q_x K \left(\frac{A_y}{A_x} \right)^\emptyset \quad (2.9)$$

Where Q_y is streamflow at ungauged site, Q_x is streamflow at gauged site, K is Bias correction factor, A_y/A_x is drainage area ratio (ungauged to gauged), and \emptyset is exponent.

The estimation of bias correction factor and exponent is based here on calibrating the two parameters using a simple optimization method with the General Reduced Gradient Algorithm (GRG) in Microsoft Excel. The DAR method is also one of linear equations, and this can be simply represented as:

$$Y = BX \quad (2.10)$$

Assuming Y is Q_y , and X is Q_x . B can be written as:

$$B = K \left(\frac{A_y}{A_x} \right)^\emptyset \quad (2.11)$$

Bs can be estimated by linear regression equation without intercepts, made from selected primary control point pairs with high correlate coefficient (more than $r=0.91$). Both drainage areas are known values, so unknown values (K and \emptyset) can be calibrated using an

optimization method. The automated calibration method needs an objection function for determining optimality of alternative sets of values for the decision variables (K and \emptyset). The optimization process is to find values of decision variable that minimize a specific objective function (OF) defined by Equation (2.12).

$$OF = \frac{1}{n} \sum \left(\frac{(B - B_e)}{B} \right)^2 \quad (2.12)$$

Where, B is a slope value from datasets, and B_e is calculated with Equation (2.11).

2.2.2 Regional Statistical Method

Hirsch (1979) suggested a regional statistical method, and showed the method has apparently better performance than the DAR, but Farmer and Vogel (2013) revealed that the regional statistical method may not always be superior to the DAR method in their research. The equation of the method is

$$Q_y = \mu_y + \frac{\sigma_y}{\sigma_x} (Q_x - \mu_x) \quad (2.13)$$

Where, Q_y is streamflow at ungauged site, Q_x is streamflow at gauged site, σ_y is standard deviation estimated regional regression, σ_x is standard deviation from gauged data, μ_y is mean estimated regional regression, and μ_x is mean from gauged data.

The regression equations used to determine μ_y and σ_y are based on the various hydrological features in the basin (Hirsh, 1979). Emerson *et al.* (2005) showed that average season flows at ungauged sites can be estimated by the regression equation, made by linear relationship between drainage area and average seasonal flows at gauged sites. Likewise, if mean and standard deviation for an ungauged site are estimated by a regional equation based on only drainage area, the regional statistics method may be used in developing flow sequences at ungauged based on flows at gauged sites under the same condition of the DAR method. In this proposed research, the reason why the regional statistical method may not synthesize flow sequences better than the DAR method will be explained, and a new approach to minimize bias on the method will be suggested.

The sample standard deviation from data may have bias as:

$$\log \sigma_x = \log S_x + E(\varepsilon_1) \quad (2.14)$$

Where, σ_x is population standard deviation, S_x is the sample standard deviation, and $E(\varepsilon_1)$ is the expected bias. Similarly, the estimated standard deviation by regional regression may also have bias as:

$$\log \sigma_y = \log S_y + E(\varepsilon_2) \quad (2.15)$$

Where, σ_y is population standard deviation, S_y is estimated standard deviation, and $E(\varepsilon_2)$ is expected bias. Equation (2.14) and Equation (2.15) can be transformed respectively to:

$$\sigma_x = S_x 10^{E(\varepsilon_1)} \quad (2.16)$$

$$\sigma_y = S_y 10^{E(\varepsilon_2)} \quad (2.17)$$

When Equations (2.14 and 2.15) are substituted into Equation (2.13), biases of both standard deviations will be retained in a different form like $10^{E(\varepsilon_2)} / 10^{E(\varepsilon_1)}$ in Equation (2.13). For this reason, the regional statistical method may result in poor performance compared with the DAR method. If both standard deviations from a same source are used, bias can be removed like $10^{E(\varepsilon_1)} / 10^{E(\varepsilon_1)} = 1$. Thus, if both statistic moments from regional regression equation are used, the regional statistical method will be an alternative of the DAR method.

2.3 Disaggregation of Monthly to Daily Naturalized Flow Sequences Using the SWAT Model

Multiple daily flow patterns should be necessary for disaggregation of monthly to daily naturalized flow datasets keeping their spatial consistency among all sites within a river basin. In other words, a daily flow pattern at a site should not only cover a period of monthly flow data but also contain natural flow characteristics without any human influences and spatial consistency such as routing effects between up and downstream sites within a river basin.

The daily SWAT model can be a useful solution to simultaneously generating daily flow patterns at all primary control points of a basin in WAM datasets, if there are reliable daily rainfall data, well-spatially distributed within a river basin. The National Climatic Data Center (NCDC) provides daily precipitation data nationwide, even though gaps of missing data may occur, and the SWAT model can generate other climate data.

2.3.1 Calibration Strategies

Parameter calibration is an important aspect of developing an appropriate daily SWAT model for synthesizing daily flow patterns. There are two possible strategies for calibrating the SWAT model. The first strategy is to calibrate the model with monthly naturalized flows at primary control points, and then the daily flow sequences at all sites of interest in the basin are generated using the calibrated model. The second approach is to calibrate the model with daily recorded data that can be considered as naturalized flow sequence for at least five years, and to validate the model with daily recorded data that can also be regarded as naturalized flows for at least five years at sites. The daily flow sequences at all sites of interest in the basin are synthesized using the calibrated and validated model.

The first strategy requires enough long period recorded data, and implementation of multiple-site calibration for considering the spatial consistency of the model. However, the calibrated model with monthly recorded data may not guarantee performance of the daily model. The second strategy should be the best solution in generating homogeneous daily flow sequence at pertinent sites in the basin, but it is very restricted by limited available recorded data at the sites of interest. It is actually impossible to get perfect daily naturalized flow records. In reality, if there are long period-of-record data, the data sequence prior to evident human influence such as initial dam impoundment can be considered naturalized flow sequence, and then used for the model calibration after dividing the data into pre- and post-human influence. Multiple-site calibration may be impossible because generally only a limited number of sites have available records.

Accordingly, the more appropriate strategy for the daily model calibration will be achieved through comparative evaluation of the two daily flow sequences, developed by the alternative strategies mentioned above because each basin has different hydrologic conditions and recorded data.

2.3.2 Comparative Evaluation

Recorded daily naturalized flow data are needed for comparative evaluation of disaggregated flow sequences by synthesized daily flow patterns. In reality, it may be impossible to get perfectly homogeneous recorded data. However, if there are recorded flow sequences at a site for a period before a dam impoundment, these may be considered daily flow data that are not exact but similar to naturalized flows without human influences because it is well-known that a dam impoundment has seriously changed flow regime in a river.

Disaggregated daily sequences from monthly should have highly linear correlation, similar flow characteristics, and overall hydrologic states (drought and flood years) with recorded daily data for a specific period at same sites. Four different methods are adopted for achieving these evaluation conditions as listed in Table 2.2.

Table 2.2 Four Different Evaluating Methods

Methods	Purpose	Quantification method
Nash-Sutcliffe Coefficient ($-\infty$ to 1)	Streamflow timing (higher is better)	<ul style="list-style-type: none">- Comparing USGS vs. Monthly base disaggregated with USGS vs. Daily based disaggregated- Total score of each method in a basin- Better is 2, similar is 1, and worse is 0
Flow Frequency Metric	Streamflow regime (Qualitative Evaluation)	
DHRAM (IHA) (0 to 30 points)	Hydrologic characteristics alteration (Lower is better)	
Median Annual Flow Duration Curve	Overall hydrologic state of a river (Drought and Flood years) (Qualitative Evaluation)	

All scores from each method are summed up to select the more appropriate calibration strategy based on the total scoring method adopted in this research. The total scoring method is shown in Table 2.3.

Table 2.3 Total Scoring Method

Methods	Score (0 to 8)	Selection
Nash-Sutcliff Coefficient	High score is 2 Same score is 1 Low score is 0	The calibration method with the highest total score is finally selected for disaggregation of monthly to daily flow sequences
Flow Frequency Metric	High score is 2 Same score is 1 Low score is 0	
DHRAM (IHA)	High score is 2 Same score is 1 Low score is 0	
Median Annual Flow Duration Curve	High score is 2 Same score is 1 Low score is 0	

2.4 Analyses of Environmental Flow Standards

Research and development to expand WRAP/WAM capabilities performed at Texas A&M University sponsored by the TCEQ over the past several years have focused on converting monthly WAM models to daily and incorporating SB3 environmental flow standards for six case study river basins: Brazos, Colorado, Trinity, Guadalupe-San Antonio, Neches, and Sabine. The SB3 environmental flow standards are defined in terms of four flow regimes: subsistence flows, base flows, within-bank pulse flows, and overbank pulse flows (Wurbs and Hoffpauir, 2013b; Pauls 2014). Pauls (2014) developed metrics for evaluating achievement of the environmental flow standards focusing on whether river flows meet to the environmental flow standards with variations in the flow standards or water right priorities and impacts on existing water rights.

As discussed Chapter I, the Indicators of Hydrologic Alteration (IHA) method (Richter *et al.* 1996) for assessing the degree of hydrologic alteration attributable to human influence has been applied throughout the world. The IHA methodology is based on computing values for groups of statistical parameters for the components of a flow regime. Annual median flow duration curves provide a graphical illustration of the overall hydrologic states of a river (Vogel *et al.*, 2007). A flow duration curve (FDC) can be easily made by the relationship between ordered median daily streamflow records and a function

of their exceedance probability representing the exceedance probability of daily flows in a median year (Vogel *et al.*, 2007).

The research reported in this dissertation further investigates statistical parameters that can be used to quantify long-term flow changes and the extent to which environmental flow changes can be achieved subject to water availability using the two different assessment methods mentioned above. The WRAP model and these methods are applied to develop an enhanced understanding of river flows in the GSA, Neches, and Sabine case study river basins.

CHAPTER III

CASE STUDY BASINS

The SWAT and WRAP/WAM modeling systems are applied in the dissertation research to the Guadalupe and San Antonio (GSA), Neches, and Sabine River Basins. Available flow data from other river basins are also included in some of the analyses performed in the research.

3.1 Sabine River Basin

3.1.1 Description of the Basin

The Sabine River Basin is located in east of Texas encompassing a part of Texas and a part of Louisiana as shown in Figure 3.1. The Sabine River Basin is about 300 miles long and has a maximum width of 48 miles. Its total drainage area is about 9,760 square miles, with 7,400 square miles (76 percent) in Texas and the remainder in Louisiana. The drainage area of the upper basin at the point near the town of Logansport, Louisiana, where the river becomes the state boundary, is 4,850 square miles. The Sabine River along with Toledo Bend Reservoir serves as a 265 miles segment of the state border. Major tributaries include Cow Bayou, Bayou Anacoco, Bayou Toro, Tenaha Creek, Martin Creek, Murvaul Bayou, Big Sandy Creek, and Lake Fork Creek. The Sabine River flows into Sabine Lake which is a 14-mile long, 90,000-acre estuary formed by the confluence of the Sabine and Sabine Rivers. Sabine Pass is the natural outlet of Sabine Lake into the Gulf of Mexico. The largest city in the river basin is Longview with a population of 80,500 located in the upper basin (Wurbs *et al.*, 2014a). Mean annual rainfall ranges from 44 inches in the upper basin to 56 inches near the Gulf of Mexico.

3.1.2 Sabine WAM

TCEQ contractors developed the original Sabine WAM as documented by a 2001 report entitled *Water Availability Modeling for the Sabine River Basin – Final Report*. The

TCEQ has periodically updated the Sabine WAM water rights data files along with the WAMs for the other river basins of the state. The authorized use scenario Sabine WAM dataset with latest TCEQ revisions dated August 8, 2007 was used for developing the daily WAM. The Sabine WAM files for the authorized use scenario (run 3) and current use scenario (run 8) have the filename roots *sabine3* and *sabine8*, respectively. *WRAP-SIM* prints a listing to its message file of the number of various system components. The *SIM* counts in Table 3.1 are from the August 2007 Sabine WAM full authorization and current conditions scenarios (Wurbs *et al.*, 2014a).

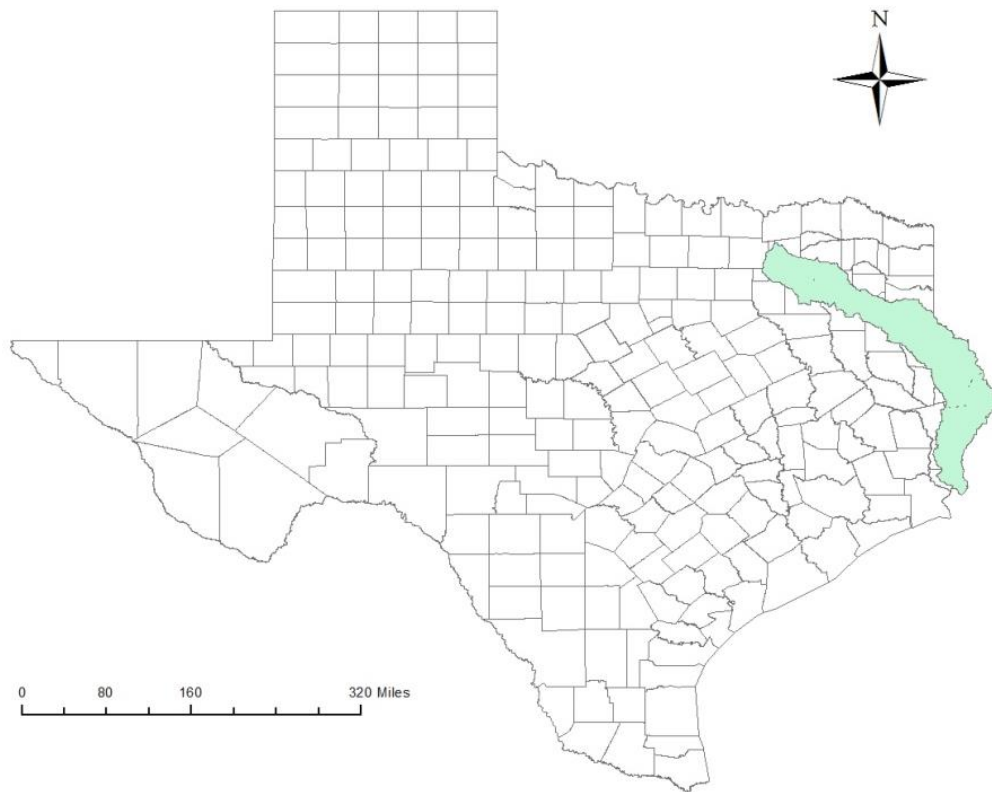


Figure 3.1 Location of Sabine River Basin

Table 3.1 Number of System Components in Sabine WAM Datasets

Latest Update of Datasets Water Use Scenario Filename	Aug 2007 Authorized sabine3	Aug 2007 Current sabine8
total number of control points	387	387
number of primary control points	27	27
control points with evaporation-precip rates	20	20
number of reservoirs as counted by <i>SIM</i>	212	213
number of <i>WR</i> record water rights	321	328
number of instream flow <i>IF</i> record rights	22	23
number of system water rights	18	18
number of hydropower rights	1	1
number of dual simulation rights	4	4
number of <i>FD</i> records in DIS file	358	358

Control points

Primary control points have monthly naturalized flow data on *IN* records in a FLO file in a *SIM* input dataset. However, the *SIM* simulation calculates naturalized flows at all other control points, called *secondary* control points based on the naturalized flows provided at the primary control points and watershed parameters (Wurbs *et al.*, 2014a).

There are 27 primary control points and 360 secondary control points in the Sabine WAM. Table 3.2 lists twenty-one of the control points. The 27 primary control points contain 17 USGS gauging stations listed in Table 3.2, the basin outlet (SRSL) also included in Table 3.2, but Table 3.2 does not list 8 control points used for referencing *EV* record reservoir evaporation rates, and another control point (CYPRES) also omitted from the table. Control point CYPRES is an accounting reservoir used in modeling the refilling of Brandy Branch Reservoir from an inter basin transfer. Only the 18 conventional primary control points serves as primary control points. The locations of the 18 primary control points are shown in Figures 3.2 and 3.3.

Table 3.2 Primary Control Points in the Sabine WAM

Control Point	Location	Gage Number	Area (mile ²)	Period of Record
CFGV	Cowleech Fork Sabine at Greenville	8017200	77.7	03/59 to present
SRWP	Sabine River near Wills Point, TX	8017410	756	10/70 to present
SRMN	Sabine River near Mineola, TX	8018500	1,357	5/39–9/59, 10/67 to present
LFQT	Lake Fork Creek near Quitman, TX	8019000	585	7/24-4/26, 3/39 to present
BSBS	Big Sandy Creek near Big Sandy, TX	8019500	231	02/39 to present
SRGW	Sabine River near Gladewater, TX	8020000	2,791	10/32 to present
SRBE	Sabine River near Beckville, TX	8022040	3,589	10/38 to present
MCTT	Martin Creek near Tatum, TX	8022070	148	4/74 to 1996
MBGR	Murvaul Bayou near Gary, TX	8022300	134	58-83
SRLP	Sabine River at Logansport, LA	8022500	4,842	7/03-2/68 (Q), 3/68-pres (stage)
TCSV	Tenaha Creek near Shelbyville, TX	8023200	97.8	03/52-06/81
BTTR	Bayou Toro near Toro, LA	8025500	148	10/55-09/86, 10/88-present
SRBU	Sabine River near Burkeville, TX	8026000	7,482	9/55 to present
BARP	Bayou Anacoco near Rosepine, LA	8028000	365	10/51-10/99
SRBW	Sabine River near Bon Wier, TX	8028500	8,229	10/23 to present
SRRL	Sabine River near Ruliff, TX	8030500	9,329	10/24 to present
CBMV	Cow Bayou near Mauriceville, TX	8031000	83.3	04/52-09/86
SRSL	Sabine River at Sabine Lake		9,756	

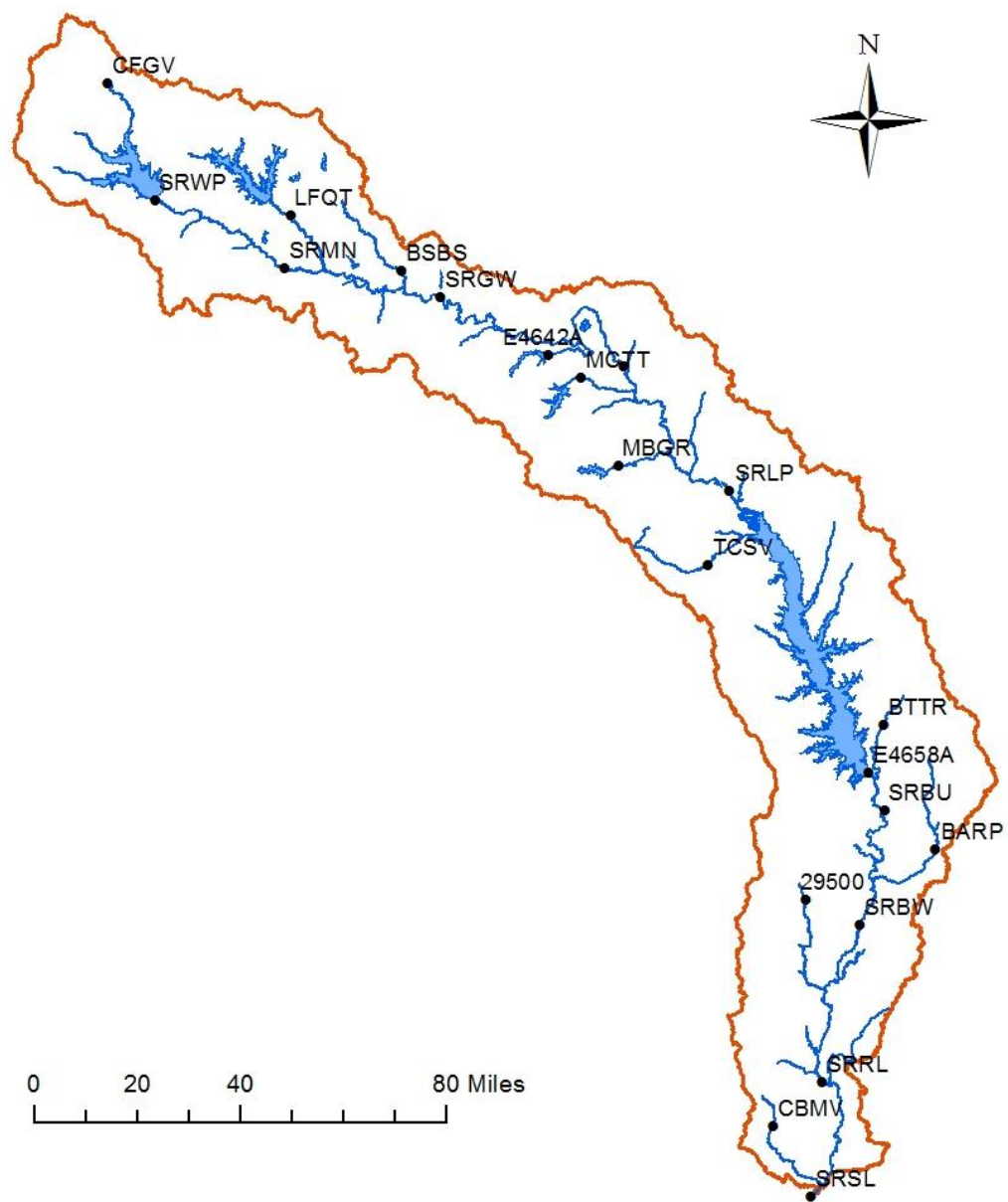


Figure 3.2 Map of Control Points in the Sabine WAM

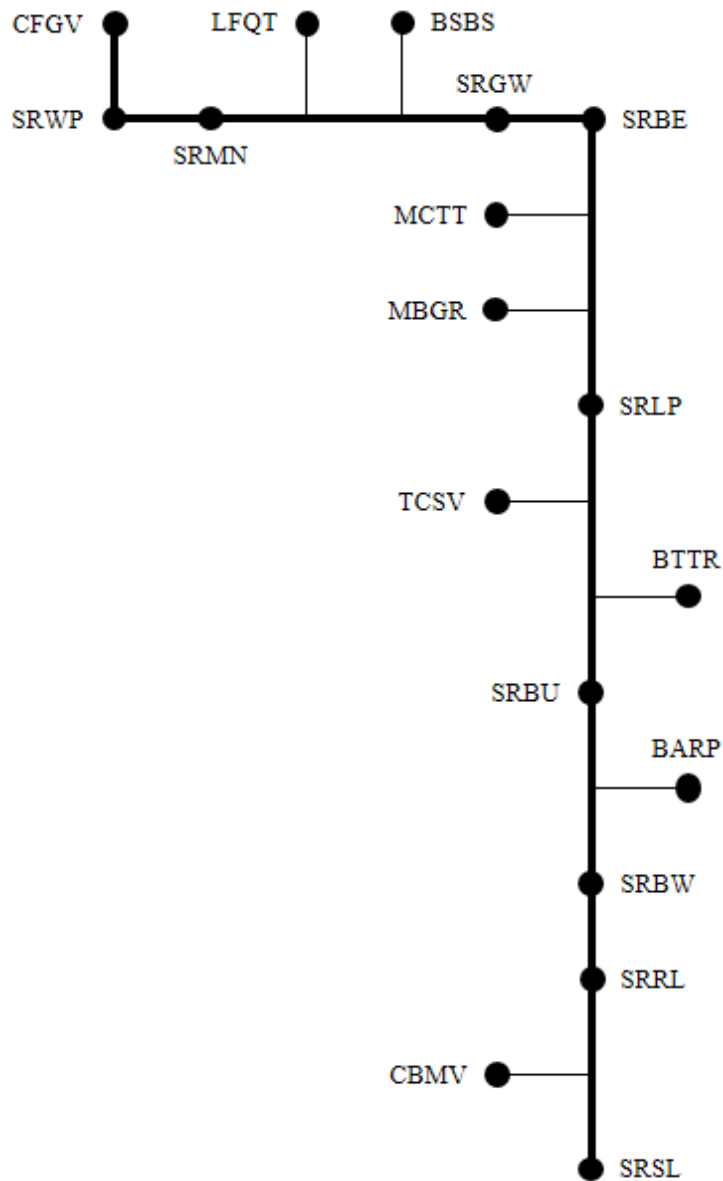


Figure 3.3 Schematic of Primary Control Points (Wurbs *et al.*, 2014a)

Reservoirs

There are 212 reservoirs in the authorized use (run 3) and 213 reservoirs in the current use (run 8) datasets last updated in August 2007. Table 3.3 lists the 13 major reservoirs with storage capacities of 5,000 acre-feet or greater located in Texas with their

storage capacities from the authorized use scenario DAT file. The total permitted conservation storage capacity of 6,343,780 acre-feet of the 13 major reservoirs capture 99.1 percent of the total storage capacity of 6,403,210 acre-feet in the 212 reservoirs. The last column of Table 3.3 accounts for the map identifiers and refers to Figure 3.4.

Table 3.3 Major Reservoirs in the Sabine WAM

Reservoir	Stream	Drainage Area (sq miles)	Initial Impoundment	Conservation Storage (acre-feet)	Reservoir ID	Control Map Point	Map ID
Toledo Bend	Sabine River	7,178	Oct 1966	4,477,000	TOLEDO	E4658A	1
Lake Tawakoni	Sabine River	756	Oct 1960	927,440	TAWAKO	E4670A	2
Lake Fork	Lake Fork Creek	493	July 1979	675,819	FORK	E4669A	3
Martin Lake	Martin Creek	130	April 1974	77,619	MARTIN	E4649A	4
Lake Cherokee	Cherokee Bayou	158	Oct 1948	62,400	CHEROK	E4642A	5
Lake Murvaul	Murvaul Bayou	115	Dec 1957	44,650	MURVAU	E4654A	6
Brandy Branch	Brandy Branch	4	1982	29,513	BRANDY	E4647A	7
Hawkins	Little Sandy	30	Aug 1962	11,890	HAWKIN	E4736A	8
Winnsboro	Big Sandy	27	June 1962	8,100	WINNSB	E4749A	9
Holbrook	Keys Creek	15	Sept 1962	7,990	HOLBRK	E4690A	10
Quitman	Dry Creek	31	May 1962	7,440	QUITMA	E4708A	11
Lake Gladewater	Glade Creek	35	Sept 1952	6,950	GLADE	E4762A	13
Greenville City Lakes	Cowleech Fork of Sabine River	Minimal (off-channel)	1888-1957	6,969	R4665A	E4665A	12

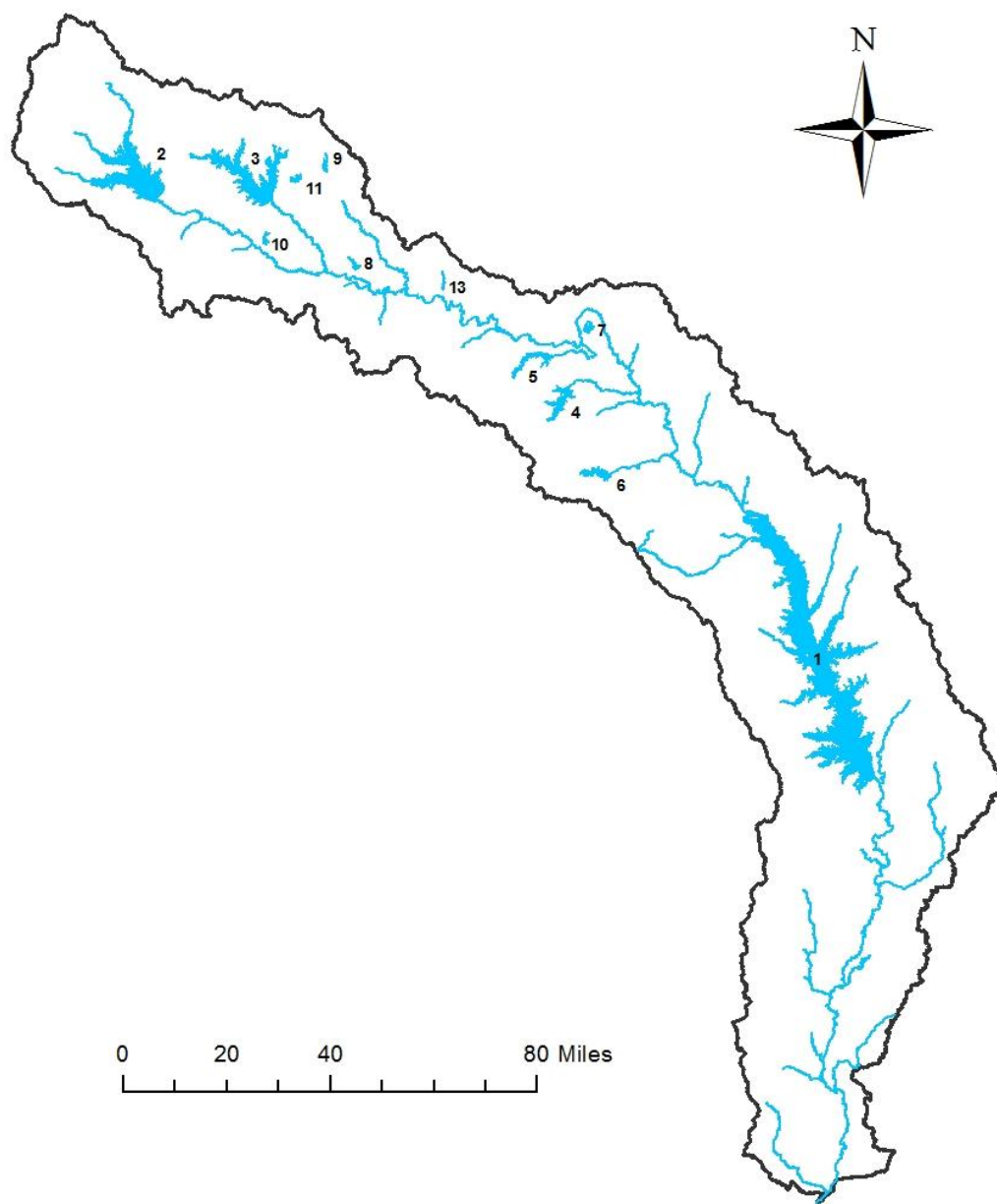


Figure 3.4 Major Reservoirs in the Sabine River Basin

Water rights

Table 3.4 summarizes the diversion data included in the 183 water rights in the Texas water right permit system, which excludes the water allocated to Louisiana under the Sabine River Basin Interstate Compact between Texas and Louisiana (Wurbs *et al.*, 2014a). Table 3.4 summarizes diversion rights with the diversions being categorized by location upstream or downstream of Stateline. Stateline serves as the border between Texas and Louisiana is located just upstream of Toledo Bend Reservoir. This is 265 mile far from the upstream end of the Sabine River.

Table 3.4 Texas Water Rights Diversion Summary from 2001 Sabine WAM Report

	<u>Diversion Rights Totals in acre-feet/year</u>					<u>Total</u>
	<u>Municipal</u>	<u>Industrial</u>	<u>Recreation</u>	<u>Mining</u>	<u>Irrigation</u>	
Upstream of Stateline	522,672	190,664	10	701	6,465	720,512
Downstream of Stateline	101,460	967,635	0	0	96,817	1,165,912
Total	624,132	1,158,299	10	701	103,282	1,886,424

There are 321 *WR* records and 22 *IF* records that model water allocated to Louisiana as well as Texas water right permits in the Sabine WAM authorized use scenario (run 3) DAT file and 328 *WR* records and 23 *IF* records in the current use scenario (run 8) DAT file. Most of the 387 control points have water rights. However, 53 *WR* records at the 19 control points listed in Table 3.5 with a total permitted diversion of 2,493,451 acre-feet/year capture 99.5 percent of the total permitted diversion of 2,505,650 acre-feet/year specified in the 321 *WR* records.

Table 3.5 Summary of Water Rights in Authorized Use Scenario Sabine WAM

Control Point	Number Rights	Permitted Diversions (ac-ft/yr)	Number Reservoirs	Storage Capacity (acre-feet)	Priorities Range From To	
USRBU	7	997,785	0	0	30001231	30001231
E4658A	3	502,215	1	4477000	30001231	30001231
E4670A	2	238,100	1	927440	19550912	19850813
E4669A	2	188,660	1	675819	19740626	19850813
E4662A	9	147,100	0	0	19260224	19781113
E4631A	1	134,500	0	0	19490919	19490919
4631G	1	112,000	0	0	19750428	19750428
E4642A	5	62,400	1	62400	19461005	19461005
E4649A	1	25,000	1	77619	19710719	19710719
E4631C	2	22,500	1	4900	19570107	19570107
E4654A	2	22,400	1	44650	19560719	19560719
E5090P	1	13,860	0	0	19860826	19860826
E4647A	2	11,000	1	29513	19780821	19780821
E4759B	5	5,600	1	183	19350424	19450713
E4665A	3	4,159	1	6969	19250630	19910925
E4762A	2	2,125	1	6950	19510517	20000808
E4675B	1	1,500	1	2261	19700105	19700105
E4657A	2	1,460	1	446	19220804	19520814
E4624A	2	1,087	1	180	19150707	19760412
Table Total	53	2,493,451	14	6,316,330	19150707	30001231
WAM Total	371	2,505,650	212	6,403,210	18711231	30001231

3.2 Neches River Basin

3.2.1 Description of the Basin

The Neches River Basin is located in east of Texas, as shown in Figure 3.5, and confined on the north and east by the Sabine River Basin, on the west by Trinity River Basin and on the south by the Neches-Trinity coastal basin. The drainage area of the Neches River Basin is about 10,000 square miles of which the drainage area of the Angelina River covers about one-third of the basin and the Neches River, Pine Island Bayou, and Village Creek covers two-thirds of the basin. The length of basin is about 200 miles. The Texas Water Development Board projected that the population of the Neches

River Basin, about 802,000 in 2010, will increase by 34% by the year 2030 (Wurbs *et al.*, 2014a). The mean annual precipitation is about 49 inches/year.

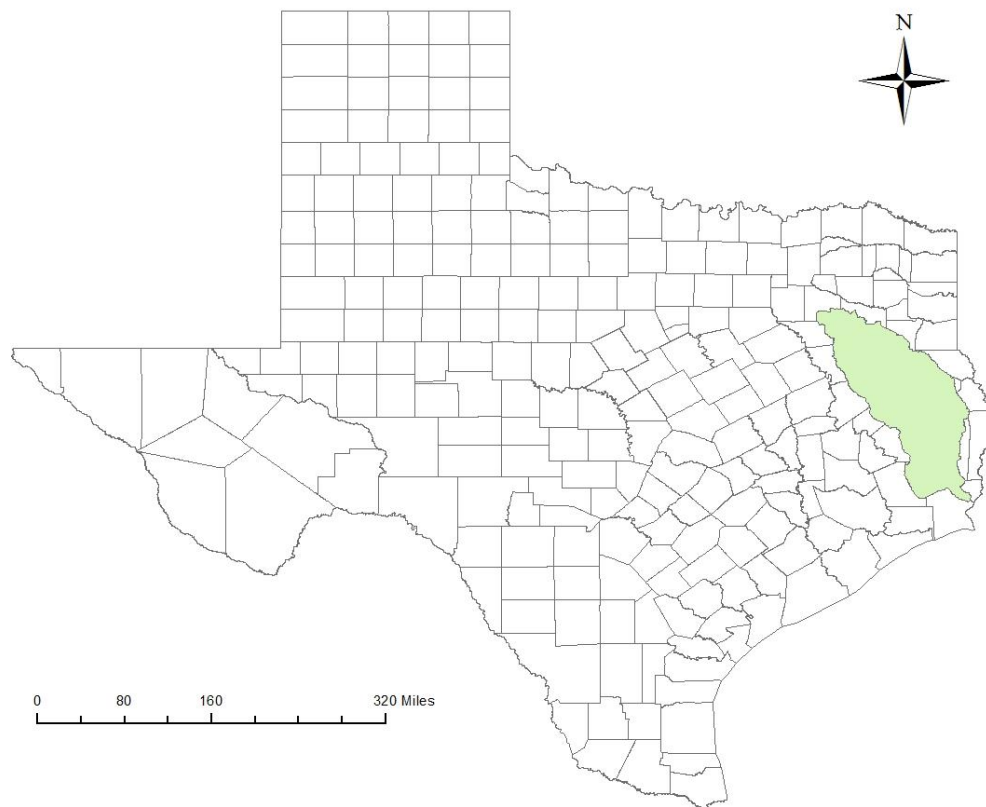


Figure 3.5 Location of Neches River Basin

3.2.2 Neches WAM

TCEQ contractors developed the original Neches WAM as documented by a 1999 report entitled *Neches River Basin Water Availability Study*. The TCEQ has periodically updated the Neches WAM water rights data files along with the WAMs for the other river basins of the state. The authorized use scenario Neches WAM dataset with latest TCEQ revisions dated October 1, 2012 was used for developing the daily WAM. The Neches

WAM files for the authorized use scenario (run 3) and current use scenario (run 8) have the filename roots *neches3* and *neches8*, respectively. *WRAP-SIM* prints a listing to its message file of the number of various system components. The *SIM* counts in Table 3.6 are from the April 2010 and October 2012 Neches WAM full authorized use scenario and September 2012 current conditions scenario datasets.

Table 3.6 Number of System Components in Neches WAM Datasets

Latest Update of Datasets Water Use Scenario Filename	Apr 2010 Authorized <i>neches3</i>	Oct 2012 Authorized <i>neches3</i>	Sep 2012 Current <i>neches8</i>
total number of control points	306	378	395
number of primary control points	20	20	20
control points with evaporation-precip. rates	12	12	12
number of reservoirs as counted by <i>SIM</i>	180	180	203
number of <i>WR</i> record water rights	328	399	385
number of instream flow <i>IF</i> record rights	19	75	78
number of system water rights	9	29	26
number of sets of water use <i>UC</i> records	33	43	43
number of <i>FD</i> records in DIS file	273	273	289

Control points

Primary control points have monthly naturalized flow data on *IN* records in a FLO file in a *SIM* input dataset. However, the *SIM* simulation calculates naturalized flows at all other control points, called *secondary* control points based on the naturalized flows provided at the primary control points and watershed parameters.

There are 20 primary control points in the Neches WAM as listed in Table 3.7 with locations and connectivity shown in Figures 3.6 and 3.7. The 16 primary control points are mostly same locations of the sites of USGS stream gaging stations, and 4 primary control points NEPA, MUTY, ANSR, and NESL corresponded to the locations of reservoirs and the river basin outlet (Wurbs *et al.*, 2014b).

Table 3.7 Primary Control Points in the Neches WAM

Control Point	USGS Gage No.	Location	Drainage Area (sq. miles)
KIBR	08031200	Kickapoo Creek near Brownsboro	232
NEPA	—	Neches River at Lake Palestine	837
NENE	08032000	Neches River near Neches	1,145
NEAL	08032500	Neches River near Alto	1,943
NEDI	08033000	Neches River near Diboll	2,724
NERO	08033500	Neches River near Rockland	3,631
MUTY	—	Mud Creek at Lakes Tyler and Tyler East Dams	114
MUJA	08034500	Mud Creek near Jacksonville	376
EFACU	08033900	East Fork Angelina River near Cushing	157
ANAL	08036500	Angelina River near Alto	1,273
ANLU	08037000	Angelina River near Lufkin	1,601
ATCH	08038000	Attoyac Bayou near Chireno	504
AYSA	08039100	Ayish Bayou near San Augustine	89
ANSR	—	Angelina River at Sam Rayburn Reservoir	3,452
NETB	08040600	Neches River near Town Bluff	7,571
NEEV	08041000	Neches River at Evadale	7,885
VIKO	08041500	Village Creek near Kountze	861
PISL	08041700	Pine Island Bayou near Sour Lake	368
NEBA	08041780	Neches River Saltwater Barrier at Beaumont	9,826
NESL	—	Neches River at Sabine Lake	10,025



Figure 3.6 Map of Primary Control Points in the Neches WAM

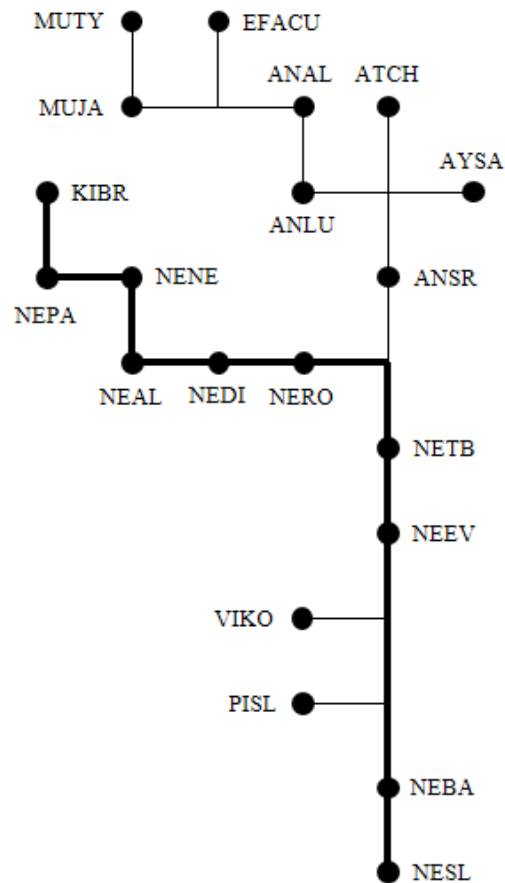


Figure 3.7 Schematic of Primary Control Points in the Neches WAM (Wurbs *et al.*, 2014b)

Reservoirs

Table 3.8 lists 11 existing reservoirs and two permitted but not yet constructed reservoirs in the Neches River Basin with storage capacities of greater than 5,000 acres with their authorized use scenario and current use scenario conservation storage capacities. Figure 3.8 shows their locations. There is the total permitted conservation storage capacity of 3,852,160 acre-feet of these 13 major reservoirs that captures 98.7 percent of the total storage capacity of 3,904,100 acre-feet in the 180 reservoirs in the October 2012 authorized use scenario Neches WAM. There are 203 reservoirs with a total conservation storage capacity of 3,656,259 acre-feet in the September 2012 current use scenario dataset.



Figure 3.8 Major Reservoirs in the Neches River Basin

Table 3.8 Major Reservoirs in the Neches River Basin

Reservoir	Dam	Stream	Initial Impound	Reservoir Identifier	Conservation Capacity	
					Authorized	Current
					(acre-feet)	(acre-feet)
Sam Rayburn	Sam Rayburn	Angelina River	1965	RAYBRN	2,898,200	2,887,736
B A	Town Bluff	Neches River	1951	STEINH	94,250	66,972
Steinhagen						
Palestine	Blackburn	Neches River	1962	PALEST	411,840	403,825
	Crossing					
Tyler East	Mud Creek Dam	Mud Creek	1966	TYLERW	43,100	36,158
Tyler	Whitehouse Dam	Prairie Creek	1949	TYLERE	44,000	44,000
Athens	Athens	Flat Creek	1962	ATHENS	32,840	29,475
Jacksonville	Buckner	Gum Creek	1957	JACKSN	30,500	30,239
Striker Creek	Striker Creek	Striker Creek	1957	STRIKR	26,960	22,618
Kurth	Kurth (off- channel)	Angelina River	1961	KURTH	16,200	14,600
Pinkston	Pinkston	Sandy Creek	1978	PINKST	7,380	7,349
Nacogdoches	Nacogdoches	Bayo Loco Creek	1976	NACH	42,318	39,427
<u>Proposed Projects Permitted but Not Yet Constructed</u>						
Columbia	Columbia	Mud Creek	—	COLUM	195,500	—
Nacconiche	Nacconiche	Nacconiche Creek	—	NACKNK	9,072	9,072

Water rights

A summary of the water rights in the October 2012 authorized use scenario dataset is listed in Table 3.9. Total permitted annual diversions are 1,730,431 acre-feet/year and these are allocated between types of use as follows: municipal (30.2%), industrial (43.4%), irrigation (25.7%), mining (0.07%), recreation (0.00%), and other (0.59%). The original 1999 WAM contained all of the water rights as listed in Table 3.9 with the exception of the addition of a water right with a diversion of 10,000 acre-feet/year and priority date of November 3, 2004.

Table 3.9 Water Rights Summary

Type of Use	Number of Rights	Permitted Diversions (ac-ft/yr)	<u>Range of Priority Dates</u>	
			from	to
municipal	29	523,077	1915	2000
industrial	49	751,607	1914	1990
irrigation	119	444,189	1913	1994
mining	6	1,287	1948	1977
recreation	99	0	1900	2002
other	<u>11</u>	<u>10,271</u>	1969	2010
total	313	1,730,431		

3.3 Guadalupe and San Antonio River Basins

3.3.1 Description of the Basins

The Guadalupe and San Antonio River (GSA) Basins are located in the southern part of Texas as shown in Figure 3.9. The Guadalupe River is the main stream, and the San Antonio River is the tributary of the Guadalupe River. The two rivers meet at a short distance upstream of Guadalupe Estuary and then flow into San Antonio Bay. The basin areas of the Guadalupe and San Antonio Rivers are 5,900 and 4,200 square miles each, for a combined total of 10,100 square miles. The two river basins are considered a single WAM because there are more than 30 percent of the total authorized consumptive diversion from the two rivers and their tributaries at the downstream of their confluence (Wurbs *et al.*, 2014b).

The lengths of the Guadalupe and San Antonio Rivers are about 230 miles long and about 240 miles long, respectively. The major tributaries of the Guadalupe River are the San Marcos River, Peach Creek, Sandies Creek, and Coleta Creek. The Blanco River and Plum Creek flow into the San Marcos River which flows into the Guadalupe River. The major tributaries of the San Antonio River are the Medina River, Leon Creek, Salado Creek, and Cibolo Creek. The range of average annual rainfall in the basins is from 39 inches near San Antonio Bay to 29 inches in the western portions of the basins. The

Edwards Aquifer, which significantly affects surface water availability through spring flows and recharge within the aquifer outcrop transects the basins.

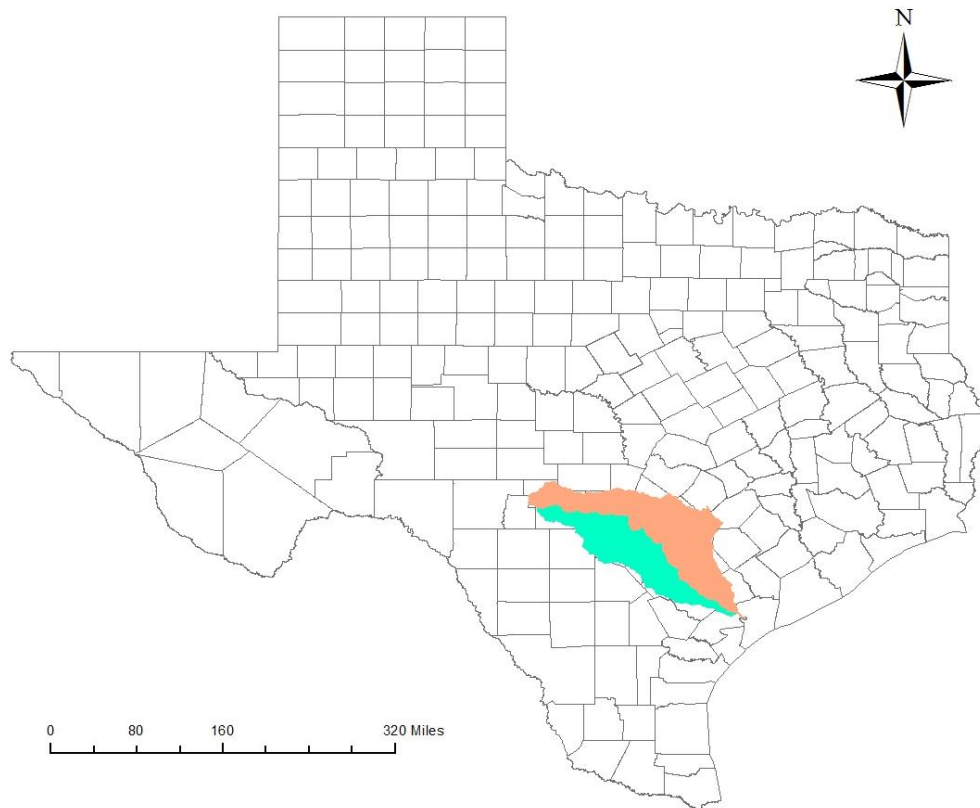


Figure 3.9 Location of Guadalupe and San Antonio River Basins

3.3.2 GSA WAM

Consulting firms working for the TCEQ developed the original Sabine WAM as documented by a 1999 report entitled *Water Availability in the GSA River Basin*. The TCEQ has periodically updated the GSA WAM water rights data files along with the WAMs for the other river basins of the state. The authorized use scenario Sabine WAM dataset with latest TCEQ revisions dated October 14, 2008 was used for developing the daily WAM. The GSA WAM files for the authorized use scenario (run 3) and current use

scenario (run 8) have the filename roots gsa_run3 and gsa_run8, respectively. *WRAP-SIM* prints a listing to its message file of the number of various system components. The *SIM* counts in Table 3.10 are from the October 2008 GSA WAM full authorization and current conditions scenarios.

Table 3.10 Number of System Components in GSA WAM Datasets

Latest Update of Datasets Water Use Scenario Filename	Oct 2008 Authorized gsa_run3	Oct 2008 Current gsa_run8
total number of control points	1,338	1,340
number of primary control points	46	46
control points with evaporation-precip. rates	11	13
number of reservoirs as counted by <i>SIM</i>	238	241
number of <i>WR</i> record water rights	848	872
number of instream flow <i>IF</i> record rights	200	214
number of system water rights	21	22
number of drought index <i>DI</i> records	6	6
number of <i>FD</i> records in DIS file	1,209	1,211

Control points

There are 46 primary control points in the GSA WAM as shown in Table 3.11 and Figures 3.10 and 3.11, with monthly naturalized flows provided in the FLO file. Naturalized flows at over 1,290 secondary control points are computed during execution of *SIM* based on naturalized flow input datasets at primary control points and information provided in the DIS file. Thirty of the primary control points are located at the same location of USGS gaging stations. Gaged stream flows at Canyon Lake (CP03) and Calaveras Lake (CP31) include computed reservoir inflows. The Guadalupe River Basin has Twenty-two of the primary control points, including CP38 at the San Antonio River Confluence and CPEST at the outlet at the estuary, and the San Antonio River Basin has Twenty-four of the primary control points.

Table 3.11 Primary Control Points in the GSA WAM

Control Point	USGS Gage No.	Location	Drainage Area (sq. miles)
<u>Guadalupe River Basin</u>			
CP01	08167000	Guadalupe River at Comfort	838
CP02	08167500	Guadalupe River near Spring Branch	1,315
CP03	08167800	Guadalupe River at Canyon Lake	1,432
CP04	08168500	Guadalupe River above Comal River at New Braunfels	1,519
CP05	08169000	Comal River at New Braunfels	130
CP06	—	Guadalupe River at Lake Wood	2,103
CP08	08171000	Blanco River at Wimberley	355
CP09	08171300	Blanco River near Kyle	412
CP10	08172000	San Marcos River at Luling	839
CP11	08173000	Plum Creek near Luling	311
CP12	08174600	Peach Creek below Dilworth	460
CP13	08175000	Sandies Creek near Westhoff	549
CP14	08175800	Guadalupe River at Cuero	4,935
CP15	08176500	Guadalupe River at Victoria	5,196
CP16	08177400	Coletto Creek Reservoir near Victoria	493
CP38	08188800	Guadalupe River near Tivoli	10,122
CP71	—	Sink Creek	43
CP72	—	Purgatory Creek	34
CP73	—	York Creek	12
CP74	—	Alligator Creek	4
CP75	—	San Marcos Springs	0.1
CPEST	—	Guadalupe Estuary	10,122
<u>San Antonio River Basin</u>			
CP17	—	Olmos Creek at Edwards	8
CP18	08178000	San Antonio River at San Antonio	44
CP19	08178700	Salado Creek at San Antonio Upper Station	136
CP20	08178800	Salado Creek at San Antonio Lower Station	187
CP21	08179500	Medina Lake	634
CP22	—	Tributaries to Diversion Lake	16
CP23	08180500	Medina River near Rio Medina	649
CP241	—	West Tributaries downstream of Diversion Lake	4
CP242	—	East Tributaries downstream of Diversion Lake	7
CP25	—	San Geronimo Creek at Edwards	58
CP261	—	Leon Creek at Edwards	60
CP262	—	Helotes Creek at Edwards	28
CP263	—	Government Creek at Edwards	12
CP27	08180800	Medina River near Somerset	962
CP28	08181500	Medina River at San Antonio	1,310

Table 3.11 (Continued)

Control Point	USGS Gage No.	Location	Drainage Area
CP29	08181800	San Antonio River near Elmendorf	1,737
CP30	—	Braunig Lake	9
CP31	08182500	Calaveras Lake	65
CP32	08183500	San Antonio River near Falls City	2,108
CP33	08183900	Cibolo Creek near Boerne	68
CP34	08185000	Cibolo Creek at Selma	274
CP35	08186000	Cibolo Creek near Falls City	825
CP36	08186500	Ecletto Creek near Runge	239
CP37	08188500	San Antonio River at Goliad	3,906

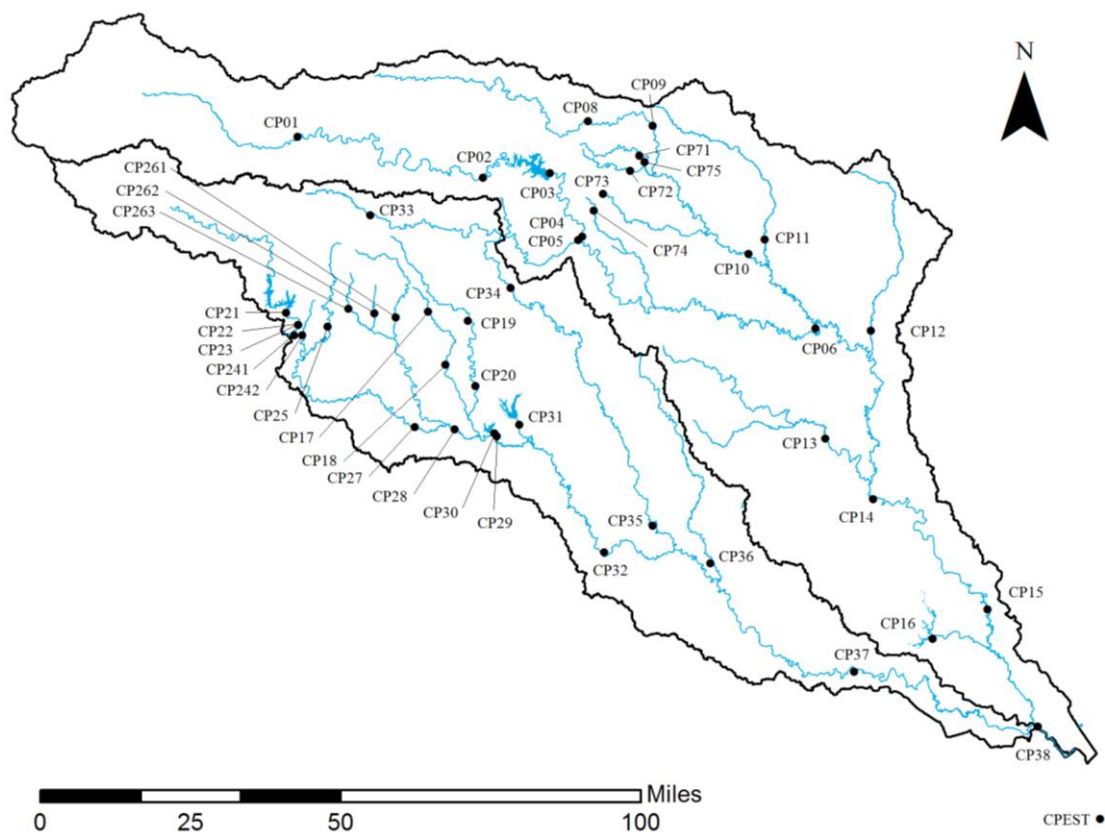


Figure 3.10 Map of Primary Control Points in the GSA WAM (Wurbs *et al.*, 2014c)

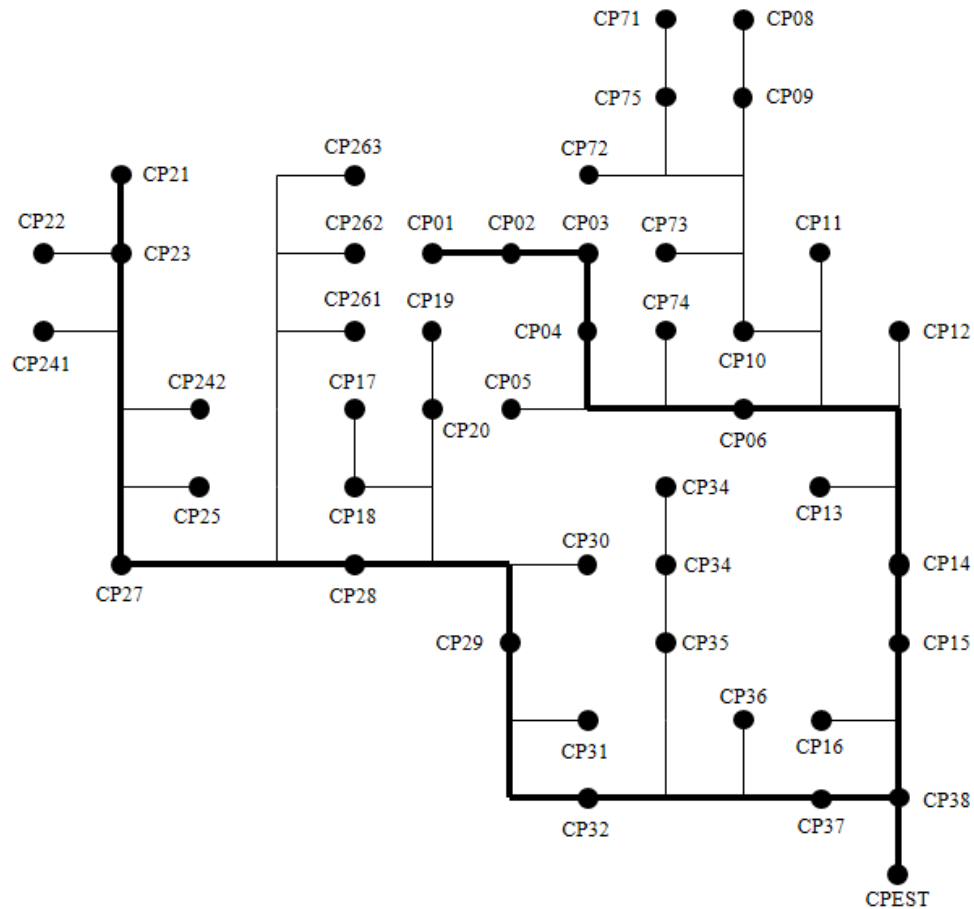


Figure 3.11 Schematic of Primary Control Points in the GSA WAM (Wurbs *et al.*, 2014c)

Reservoirs

Table 3.12 lists the nine largest of the 238 reservoirs included in the October 2008 authorized use scenario GSA WAM. There are 229 smaller reservoirs with storage capacities of 1,400 acre-feet or less in GSA WAM. The total permitted conservation storage capacity of 775,868 acre-feet of the 9 major reservoirs capture 96.1 percent of the total storage capacity of 806,875 acre-feet in the 238 reservoirs. Figure 3.12 shows the locations of nine major reservoirs. Canyon Lake cited in Table 3.12 does not contain the 394,900 acre-feet flood control pool which is not included in the WAM.

Table 3.12 Major Reservoirs in the GSA WAM

No.	Reservoir	Stream	Identifier	Control Point	Authorized Capacity (acre-feet)
1	Canyon Lake	Guadalupe River	CANYON	207401	386,200
2	Medina Lake	Medina River	MEDINA	CP21	237,875
3	Calaveras Lake	Calaveras Creek	CALVER	216231	63,200
4	Coletto Creek Reservoir	Coletto Creek	COLETO	548631	35,084
5	Victor Braunig Lake	Arroyo Seco	BRAUNG	216131	26,500
6	Olmos Reservoir	Olmos Creek	R3898	P38981	14,240
7	Cooling Reservoir		R5178	517801	4,770
8	Boerne Lake	Cibolo Creek	BOERNE	114302	4,046
9	Diversion Lake	Medina River	DIVERS	CP23	3,953

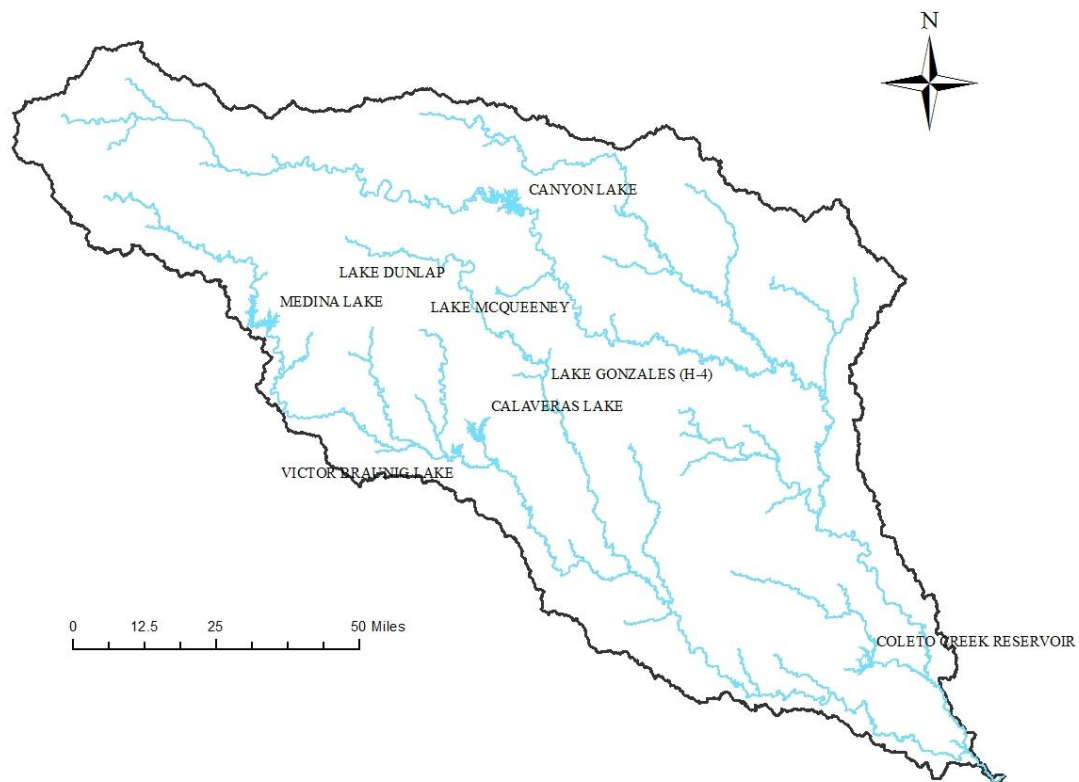


Figure 3.12 Major Reservoirs in the GSA River Basins

Water rights

There are 604 water rights having priority dates senior to August 1998 authorizing total annual diversions of 6,378,964 acre-feet/year which includes total consumptive use of 558,430 acre-feet/year in the original GSA WAM, as summarized in Table 3.13.

Flows of the Guadalupe River for generating hydroelectric energy, modeled as diversions with 100% return flows, captures 83.1 percent of the total diversions in the GSA WAM. The largest water rights in the Guadalupe River Basin and the largest water rights in the San Antonio River Basin are listed in Tables 3.14 and 3.15, respectively.

Table 3.13 Summary of Diversion Rights in the GSA WAM

Type of Use	<u>Guadalupe River Basin</u>		<u>San Antonio River Basin</u>	
	Authorized Diversions	Authorized Consumptive	Authorized Diversions	Authorized Consumptive
	(ac-ft/yr)	(ac-ft/yr)	(ac-ft/yr)	(ac-ft/yr)
Municipal	201,820	201,820	34,967	34,967
Industrial	592,324	87,862	53,436	53,436
Irrigation	89,121	89,121	88,656	88,656
Mining	153	30	4,504	600
Hydroelectric	5,303,585	0	0	0
Recreation	6,648	8	1,190	0
Other	1,600	970	0	0
Recharge	0	0	961	961
Total	6,195,250	379,810	183,714	178,620

Table 3.14 includes the 14 water rights in the Guadalupe Basin with authorized diversions of 20,000 acre-feet/year or greater. These 14 rights comprise 97.8 percent of the total authorized diversions and 74.8 percent of the total authorized consumptive use in the Guadalupe Basin. The Guadalupe Blanco River Authority (GBRA) has the first three water rights (5488, 5172, 2074) in Table 3.14 and capture 59.8 percent of the total authorized diversion in the GSA WAM.

Table 3.14 Largest Water Rights in the Guadalupe River Basin

Water Right	Owner	Diversion Rights (ac-ft/yr)	Consumptive Rights (ac-ft/yr)	Storage Rights (ac-ft)
5488	GBRA	2,603,991	0	0
5172	GBRA	1,160,431	0	0
2074	GBRA	50,000	50,000	740,900
5178	GBRA & Union Carbide	106,000	106,000	4,770
5177	GBRA & Union Carbide	51,247	51,247	0
3846	City of Gonzales	798,603	2,240	1,400
3853	Cuero Hydroelectric, Inc.	538,560	0	808
3824	New Braunfels Utilities	266,508	5,858	150
5485	Central Power and Light	209,189	0	0
3859	South Texas Electric	110,000	1,900	20
3865	Texas State University	66,217	1,217	150
3861	E.I. DuPont DeNemours	60,000	33,000	1,056
5486	Central Power and Light	20,000	12,500	35,084
5466	City of Victoria	20,000	20,000	1,000

Table 3.15 Largest Water Rights in the San Antonio River Basin

Water Right	Owner	Diversion Rights (ac-ft/yr)	Consumptive Rights (ac-ft/yr)	Storage Rights (ac-ft)
2130	Bexar-Medina-Atascosa WCID	66,750	66,750	242,374
2162	City of San Antonio	60,000	37,000	63,200
5517	City of San Antonio	12,000	12,000	26,500
4768	Leon Creek WSC	7,500	7,500	1,000
4768	Bexar Metropolitan Wat Dist	5,000	5,000	595
5211	Lone Star Growers Co.	3,000	3,000	458
5549	Bexar Metropolitan Wat Dist	2,250	2,250	148

CHAPTER IV

INFILLING MISSING NATURALIZED FLOWS USING THE SWAT MODEL

4.1 Procedure

The primary control points having monthly naturalized flow datasets are adopted from the WAM datasets of the Sabine, Neches, and GSA River Basins in order to explore the validity and effectiveness of flow synthesis methods proposed in this research. Certain sub-periods of the periods-of-analysis of the naturalized flow datasets are treated as having no flow data and called the “missing period.” Other sub-periods of the datasets are called the “recorded period” and are treated as having recorded data.

The monthly SWAT models are developed at each primary control point with GIS data such as DEM, land cover and use, soil type and the location of each primary control point. The selected parameters of these SWAT models are calibrated with the naturalized flow data for recorded period after initial simulations with inputted rainfall data and generated weather data. The calibrated SWAT models generate the flow sequences during the missing period at each primary control point. MOVE2 is applied to transfer the flow sequences to fill in gaps of missing periods based on flow sequences synthesized with the SWAT model.

The method performance is finally evaluated through comparative analysis with naturalized datasets for assumed missing periods. The comparative analysis includes statistical and flow frequency evaluations. Figure 4.1 is a flowchart summarizing the methodology for synthesizing missing naturalized flows using the SWAT model.

4.2 Data

4.2.1 Selected Naturalized Flow Datasets

In general, naturalized flow datasets in the WAM system were developed based on adjusting USGS recorded flow data using Equation 1.1. If the recorded data at a control point has some missing period, this missing period was filled by a linear transfer method

based on the data for the same period at a near control point. These WAM datasets could possibly have some statistical bias in the flow sequences. Thus, the primary control points that have naturalized flow datasets made based on only USGS recorded data without any missing periods are being selected from the three WAM datasets for more accurate evaluation of the proposed method in this research.

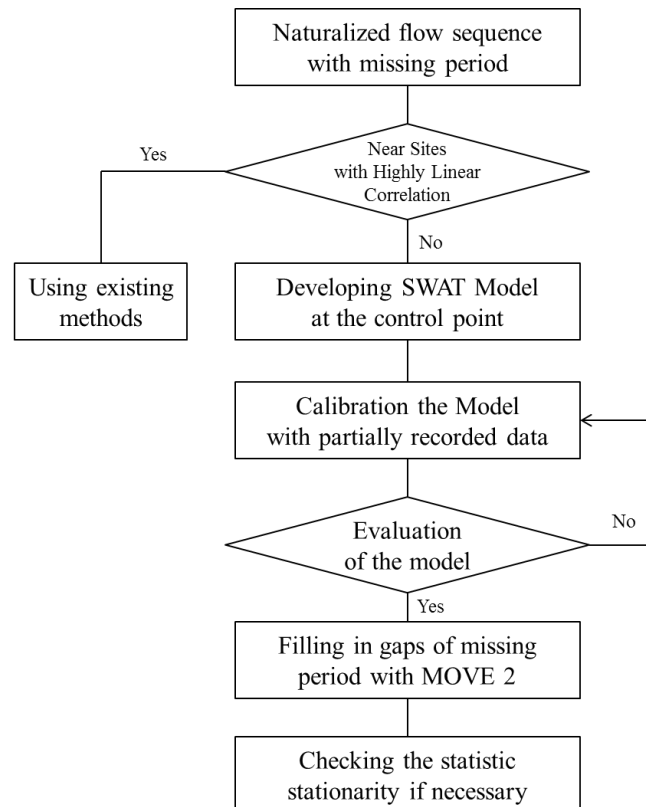


Figure 4.1 Flowchart for Infilling Missing Naturalized Flows Using the SWAT Model

Table 4.1 lists the selected primary control points from the three river basin WAM datasets, and these locations are referred to in the figures in the Chapter III. Seventeen primary control points are chosen, including 6 primary control points from the Sabine River Basin, 5 primary control points from the Neches River Basin, and 6 primary control points from the GSA River Basins, respectively.

The selected control points are well-distributed within each river basin, covering upstream and downstream. The watershed areas of these primary control points range from 130 square miles (CP05 in the GSA River Basins) as a minimum to 9,329 square miles (SRRL in the Sabine River Basin) as a maximum. The flow characteristics are very different from each other depending on the location of the control points. For example, control point CP05 has a small drainage area but relatively great base flows due to a spring water source. The periods-of-analysis of the basins are 1940-1998 for the Sabine River Basin, 1940-1996 for the Neches River basin, and 1934-1989 for the GSA River Basins, respectively, as listed in Table 4.1.

Table 4.1 Selected Primary Control Points from the Basins

Control Point	Location	Gage Number	Watershed Area (mile ²)	Period of Record
<u>Sabine River Basin (Period of Analysis: 1940 to 1998)</u>				
LFQT	Lake Fork Creek near Quitman, TX	8019000	585	7/24-4/26, 3/39 to present
BSBS	Big Sandy Creek near Big Sandy, TX	8019500	231	02/39 to present
SRGW	Sabine River near Gladewater, TX	8020000	2,791	10/32 to present
SRBE	Sabine River near Beckville, TX	8022040	3,589	10/38 to present
SRBW	Sabine River near Bon Wier, TX	8028500	8,229	10/23 to present
SRRL	Sabine River near Ruliff, TX	8030500	9,329	10/24 to present
<u>Neches River Basin (Period of Analysis: 1940 to 1996)</u>				
ATCH	Attoyac Bayou near Chireno, TX	8038000	504	2/24-9/25,08/39 to present
NEEV	Neches River at Evadale, TX	8041000	7,885	08/04 to present
NENE	Neches River near Neches, TX	8032000	1,145	02/39 to present
NERO	Neches River near Rockland, TX	8033500	3,631	07/03 to present
VIKO	Village Creek near Kountze, TX	8041500	861	01/24 to present
<u>GSA River Basins (Period of Analysis: 1934 to 1989)</u>				
CP02	Guadalupe River near Spring Branch	8167500	1,315	01/34 to present
CP04	Guadalupe River at New Braunfels	8168500	1,519	12/27 to present
CP05	Comal River at New Braunfels	8169000	130	12/27 to present
CP08	Blanco River at Wimberley	8171000	355	01/34 to present
CP32	San Antonio River near Falls City	8183500	2,108	05/25 to present
CP35	Cibolo Creek near Falls City	8186000	825	10/30 to present

4.2.2 Precipitation

It is very important to obtain as many rain gauges as possible with data for the long term period for the SWAT simulation. Hernandez *et al.* (2000) highlighted that improvements in the results of estimated streamflow could be expected by increasing the number of rain gauges. The spatial distribution of rain gauges within a basin is also important. Inadequate spatial coverage of precipitation inputs would lead to some of the poorer results in previous SWAT studies (Cao *et al.*, 2006; Conan *et al.*, 2003; Bouraoui *et al.*, 2002; Bouraoui *et al.*, 2005).

The National Climatic Data Center (NCDC) of the National Oceanic and Atmospheric Administration (NOAA) provides climatic data including precipitation on the Website: <http://www.ncdc.noaa.gov/cdo-eb/search;sessionid=113467DB762EC09682895783A8FFCDB2>. The climate data can be obtained from the web site based on the administrative, hydrologic, and climate regions. The data period can be also determined.

The measured precipitation data among various weather data such as temperature, wind speed, sun shine hours, etc. are used only in this research because the three river basins don't have snow melt issues. The SWAT model can generate other weather data that are needed for the simulation based on the weather database by its own weather generator. The ideal condition is that the periods of the collected data should cover the period-of-analysis of WAM datasets for the SWAT model, but most rain gauges don't have the desirable period of data. Although the SWAT model can fill in gaps of missing period with weather generator, the rain gauges with long term period data are collected considering spatial distribution within a basin as long as possible. Tables 4.2, 4.3, and 4.4 list the rain gauges for the three basin SWAT models in this research. The locations of the gauges are as shown in Figures 4.2, 4.3, and 4.4, respectively. These rain gauges will serve the SWAT models for the disaggregation of monthly flow to daily later in this research as well.

Table 4.2 List of Rain Gauges for the Sabine River Basin

No.	Name	Latitude	Longitude	Elevation
1	USW00013972	32.354	-95.403	165.800
2	USC00162023	31.750	-93.700	67.100
3	USC00162367	30.843	-93.287	57.900
4	USC00164288	31.375	-93.391	128.000
5	USC00165266	31.142	-93.240	8.500
6	USC00165522	31.967	-94.000	57.900
7	USC00165527	31.983	-93.950	64.000
8	USC00165892	31.577	-93.482	77.700
9	USC00410611	30.097	-94.100	6.100
10	USC00410917	30.733	-93.650	27.100
11	USC00411425	32.563	-95.877	149.400
12	USC00411500	32.162	-94.340	93.000
13	USC00411578	31.808	-94.164	99.100
14	USC00411921	33.200	-95.928	167.600
15	USC00413000	30.333	-94.083	10.100
16	USC00413546	32.746	-95.050	118.900
17	USC00413565	32.517	-94.967	85.000
18	USC00413734	33.168	-96.098	166.100
19	USC00413846	33.333	-95.233	107.000
20	USC00414020	32.578	-95.203	102.100
21	USC00414081	32.181	-94.796	128.000
22	USC00414563	30.915	-94.010	88.400
23	USC00414819	30.617	-93.917	61.000
24	USC00415094	33.035	-96.486	155.400
25	USC00415341	32.473	-94.717	100.600
26	USC00415618	32.540	-94.351	107.300
27	USC00415766	33.236	-96.642	189.600
28	USC00415956	32.717	-95.367	117.300
29	USC00416177	31.616	-94.643	132.600
30	USC00416265	31.667	-94.150	98.100
31	USC00416664	30.086	-93.742	3.000
32	USC00416722	32.267	-94.983	152.400
33	USC00417271	32.117	-94.967	113.100
34	USC00417363	32.783	-95.433	114.300
35	USC00417707	32.933	-96.465	165.500
36	USC00419068	31.175	-93.565	57.900
37	USC00419214	32.400	-95.267	149.000
38	USC00419734	31.017	-93.717	54.900
39	USC00419800	32.702	-96.015	158.500
40	USC00419836	32.889	-95.333	130.800

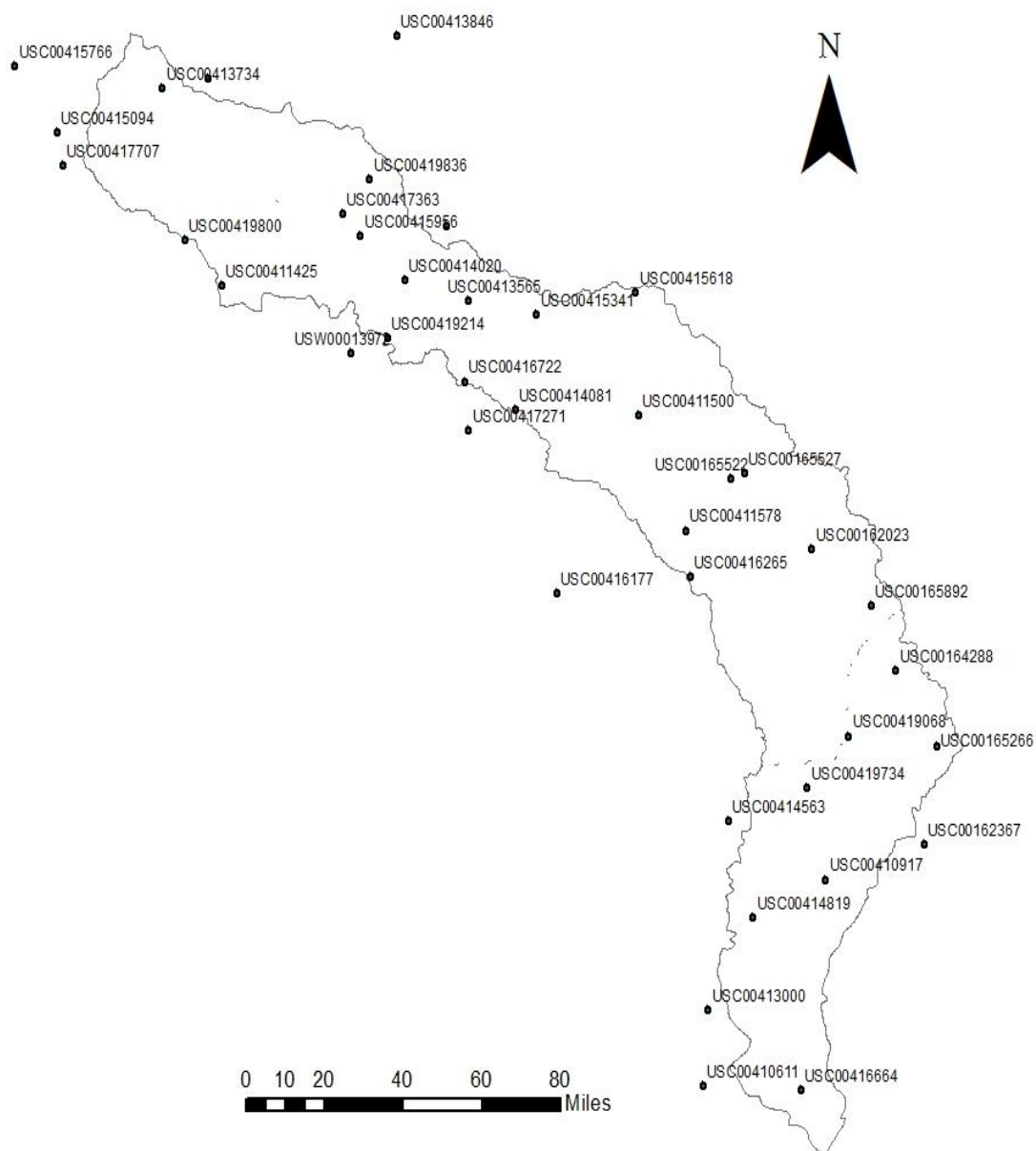


Figure 4.2 Map of Rain Gauges for the Sabine River Basin

Table 4.3 List of Rain Gauges for the Neches River Basin

No.	Name	Latitude	Longitude	Elevation
1	USW00012917	29.950	-94.017	9.100
2	USW00013972	32.367	-95.400	157.000
3	USW00093914	31.783	-95.600	141.700
4	USW00093987	31.233	-94.750	89.000
5	USC00410190	31.600	-95.150	78.900
6	USC00410404	32.217	-95.850	149.000
7	USC00410611	30.083	-94.100	6.100
8	USC00411089	31.317	-94.283	82.000
9	USC00411425	32.563	-95.877	149.400
10	USC00411578	31.808	-94.164	99.100
11	USC00412114	31.317	-95.467	107.000
12	USC00412444	31.867	-95.250	177.100
13	USC00413000	30.350	-94.083	11.900
14	USC00414081	32.150	-94.800	153.000
15	USC00414525	31.967	-95.283	159.100
16	USC00414563	30.933	-94.000	88.100
17	USC00414819	30.617	-93.917	61.000
18	USC00415196	30.050	-94.817	11.900
19	USC00415271	30.700	-94.950	57.900
20	USC00416177	31.600	-94.583	107.000
21	USC00416722	32.267	-94.983	152.400
22	USC00417040	31.250	-93.967	88.100
23	USC00417547	31.900	-94.983	100.600
24	USC00417700	31.017	-94.417	39.900
25	USC00417841	31.800	-95.150	149.000
26	USC00417936	31.067	-94.100	57.900
27	USC00417951	31.533	-94.117	100.900
28	USC00419101	30.800	-94.183	65.200
29	USC00419734	31.017	-93.717	54.900
30	USC00419754	30.550	-94.450	61.000
31	USC00419800	32.702	-96.015	158.500

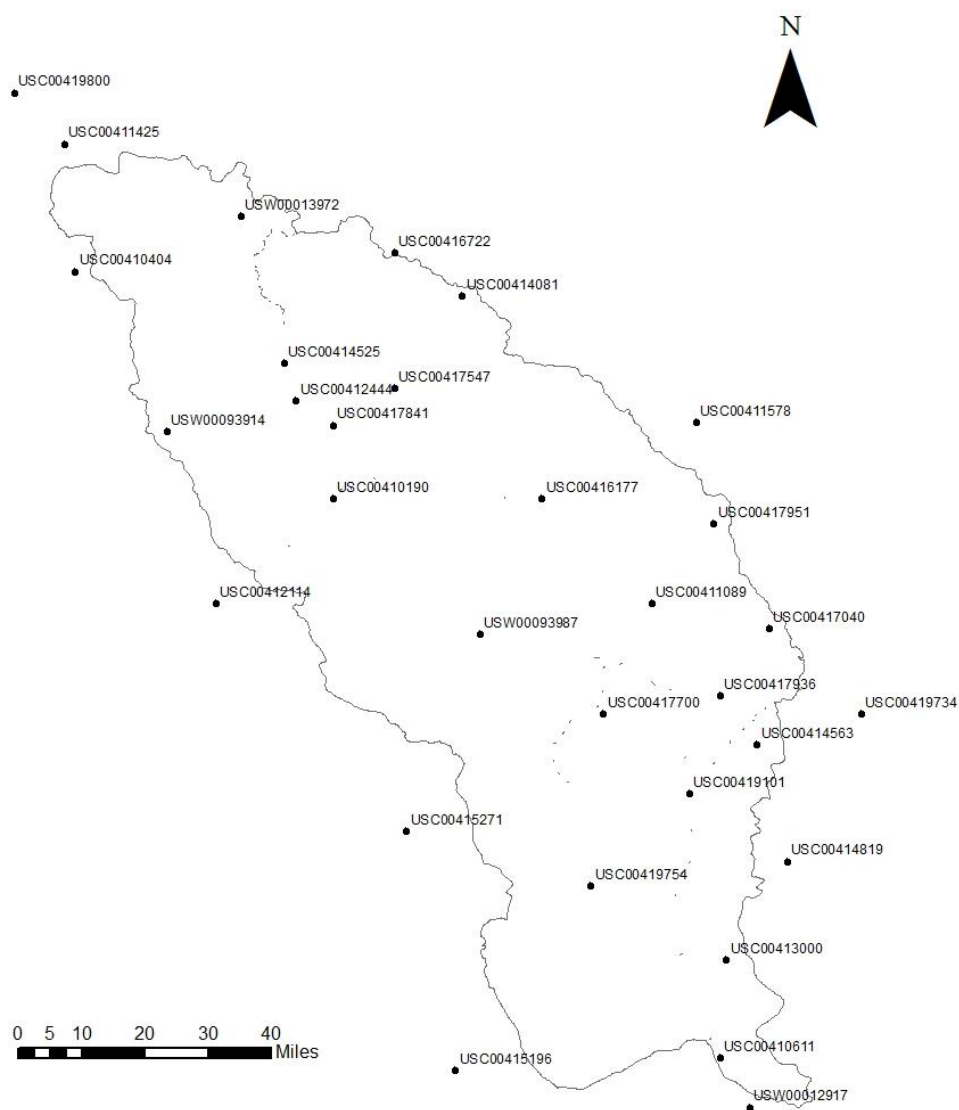


Figure 4.3 Map of Rain Gauges for the Neches River Basin

Table 4.4 List of Rain Gauges for the GSA River Basins

No.	Name	Latitude	Longitude	Elevation
1	USW00012921	29.533	-98.467	241.100
2	USC00417628	29.467	-98.867	289.900
3	USC00413065	28.950	-98.067	92.000
4	USC00417836	28.867	-97.717	95.100
5	USC00413618	28.667	-97.383	49.100
6	USC00410902	29.817	-98.750	430.100
7	USC00418845	29.650	-99.250	442.000
8	USC00419813	29.900	-99.600	694.900
9	USC00418658	29.300	-97.967	146.000
10	USC00413201	29.133	-98.150	118.900
11	USC00411215	29.750	-98.450	335.900
12	USC00417215	29.050	-98.567	189.000
13	USC00410639	28.450	-97.700	67.100
14	USC00410509	30.133	-98.817	533.400
15	USC00410832	30.100	-98.417	412.100
16	USC00411671	29.267	-97.400	75.900
17	USC00412040	30.167	-99.133	681.200
18	USC00412173	29.083	-97.250	57.900
19	USC00413156	29.983	-98.267	353.900
20	USC00413183	29.683	-97.133	131.100
21	USC00413622	29.500	-97.450	92.000
22	USC00414375	30.050	-99.517	638.600
23	USC00414575	29.817	-97.317	139.900
24	USC00414780	30.033	-99.133	502.900
25	USC00415284	29.900	-97.700	159.100
26	USC00415429	29.667	-97.633	121.900
27	USC00416276	29.700	-98.117	220.100
28	USC00416368	29.267	-97.767	121.900
29	USC00417983	29.850	-97.950	186.500
30	USC00418186	29.550	-97.967	171.000
31	USC00418544	29.867	-98.383	305.100
32	USC00419365	28.800	-97.017	9.100
33	USC00419953	28.983	-97.500	82.300

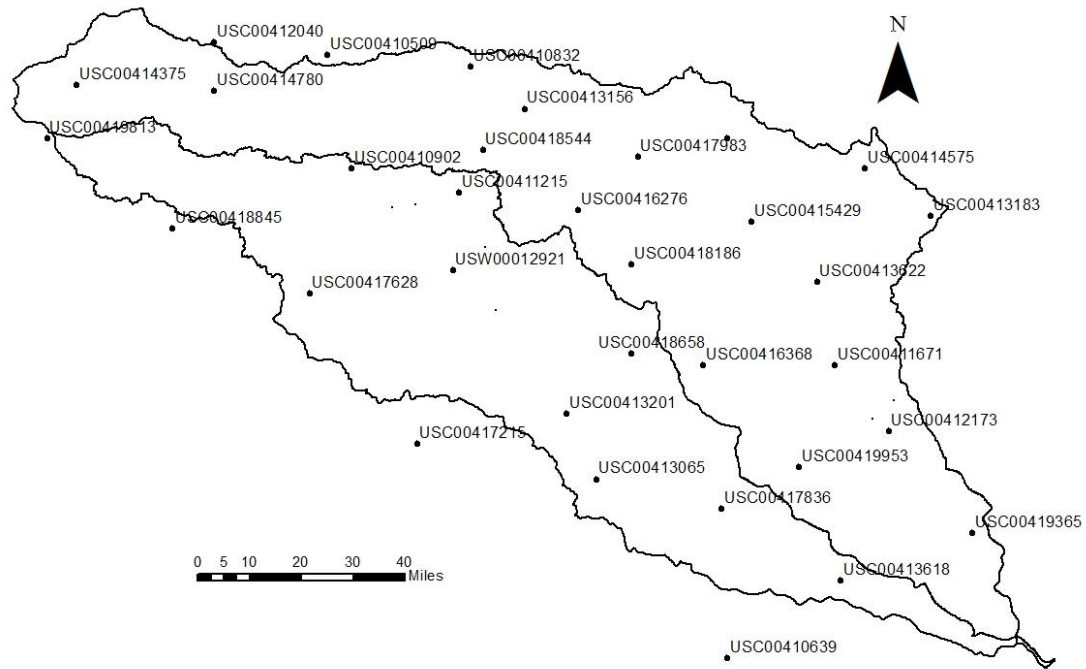


Figure 4.4 Map of Rain Gauges for GSA River Basins

4.2.3 GIS Data

The SWAT model needs GIS data such as DEM, land use, and soil map for delineating a watershed and for extracting spatially distributed hydrologic and topographic parameters. The SWAT model includes soil map (STATSGO) only in the U.S. Accordingly, DEMs with 30 m resolution and 2001 land cover map are obtained from the NRCS website for this research.

TCEQ also provides GIS data including the location of control points in WAM system through the TCEQ website. These GIS data will be used for the SWAT models for disaggregation of monthly flow to daily later like rain data in this research.

4.3 SWAT Model

4.3.1 Watershed Delineation

ArcSWAT provides a visual user interface for SWAT modeling within ArcGIS. The SWAT2012 with 2010 ArcGIS version is used in this research. Most of all, SWAT model delineates 17 watersheds for 17 primary control points with DEMs and the location data of primary control points using the watershed delineator. Next, the model determines HRUs with land use and soil data in sub-watersheds of a watershed. SWAT finally writes input tables with input rainfall data, weather generator and hydrologic and topographic parameters. Each SWAT model automatically chooses the nearest rain gauges based on the spatial locations from the input rainfall data.

The variation in HRU and/or sub-watershed delineations for watersheds generally affects streamflow prediction by SWAT model (Gassman *et al.*, 2007). In other words, the SWAT model should have a proper number of subwatersheds and HRUs in each subwatershed for more accurate streamflow prediction. Thus, all watersheds are delineated to have at least 15 subwatersheds and multiple HRUs in each subwatershed in each SWAT model. SWAT uses a daily computational time step, but daily results can be aggregated to monthly. The periods of simulation are different depending on the period-of-analysis in each basin WAM. The watershed delineations for each SWAT model are as shown in Figures 4.5 to 4.21.

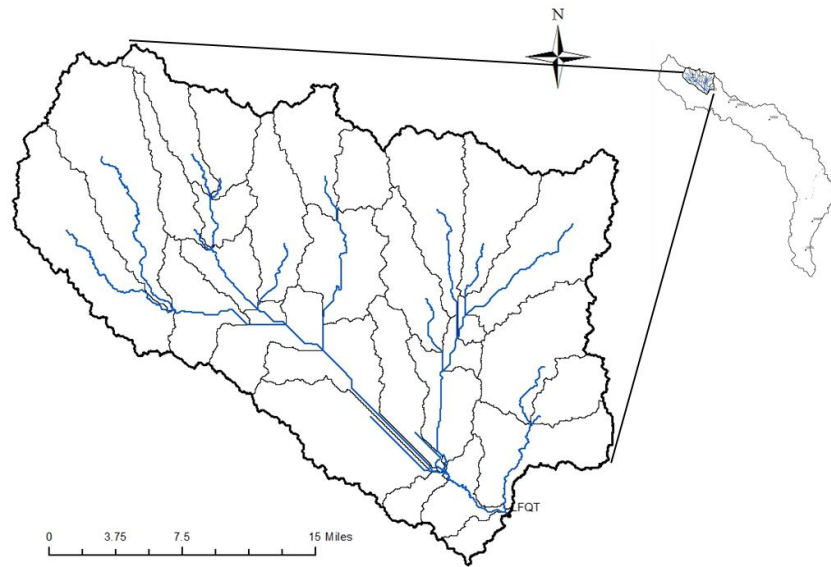


Figure 4.5 SWAT Model Watershed Delineation at LFQT in the Sabine River Basin

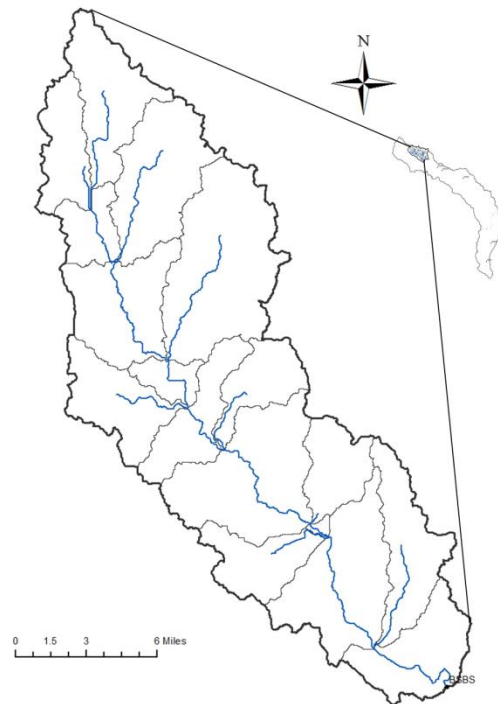


Figure 4.6 SWAT Model Watershed Delineation at BSBS in the Sabine River Basin

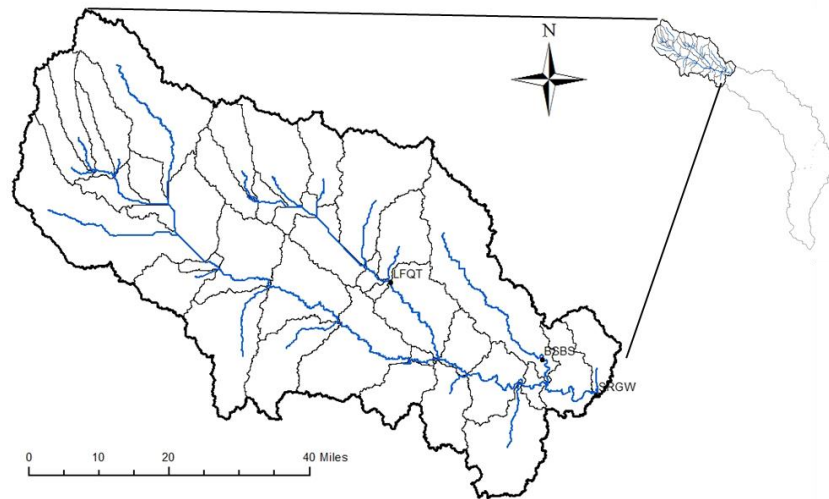


Figure 4.7 SWAT Model Watershed Delineation at SRGW in the Sabine River Basin

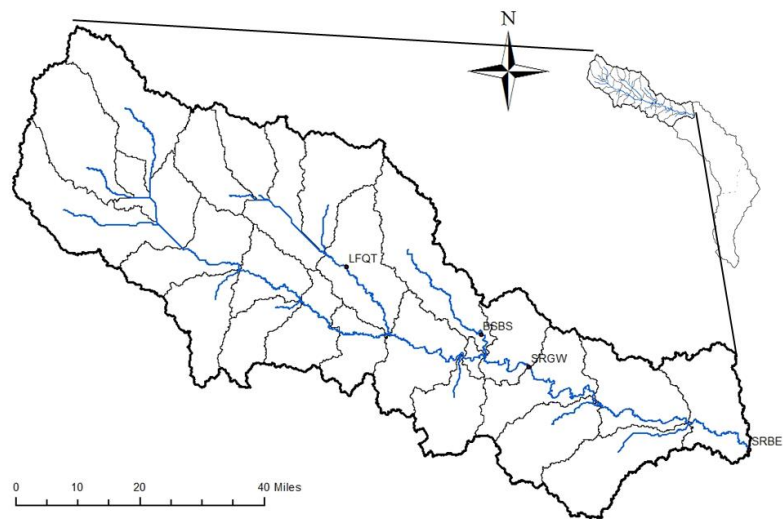


Figure 4.8 SWAT Model Watershed Delineation at SRBE in the Sabine River Basin

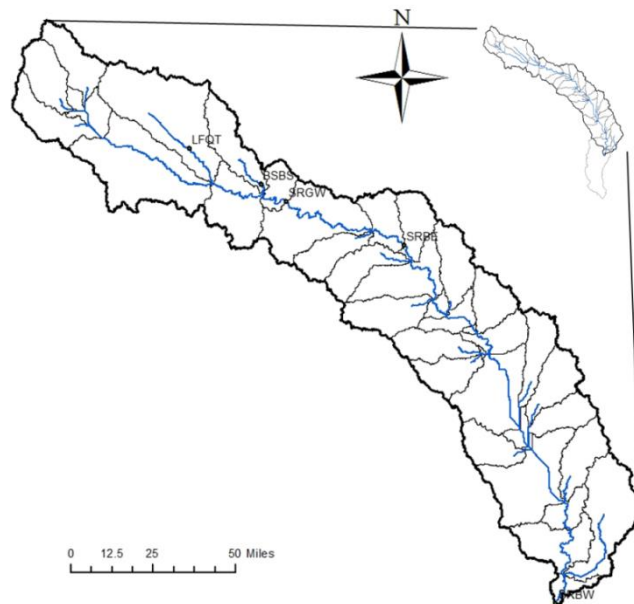


Figure 4.9 SWAT Model Watershed Delineation at SRBW in the Sabine River Basin

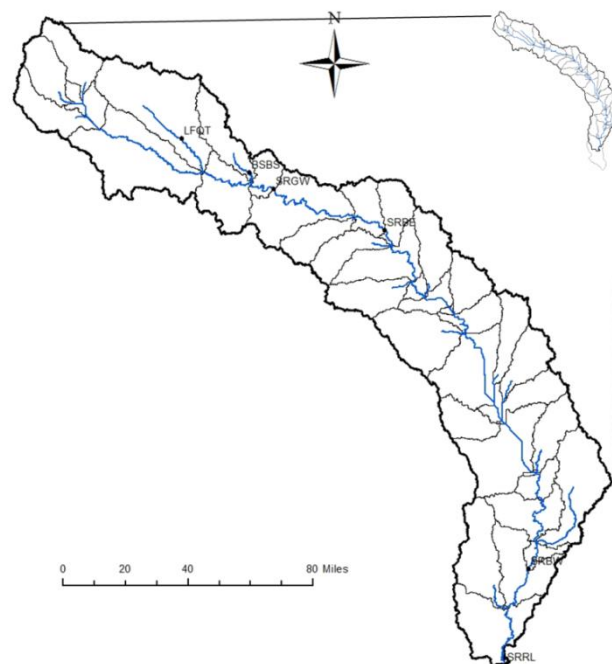


Figure 4.10 SWAT Model Watershed Delineation at SRRL in the Sabine River Basin

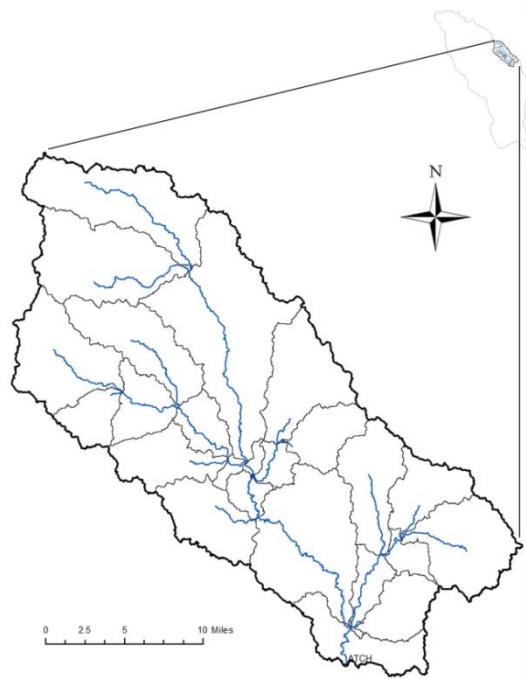


Figure 4.11 SWAT Model Watershed Delineation at ATCH in the Neches River Basin

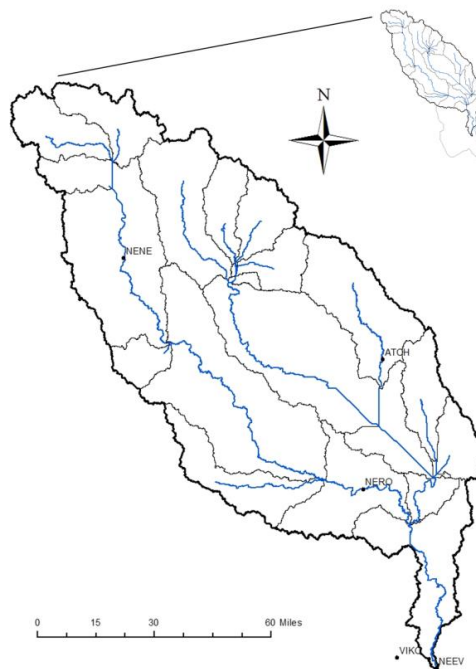


Figure 4.12 SWAT Model Watershed Delineation at NEEV in the Neches River Basin

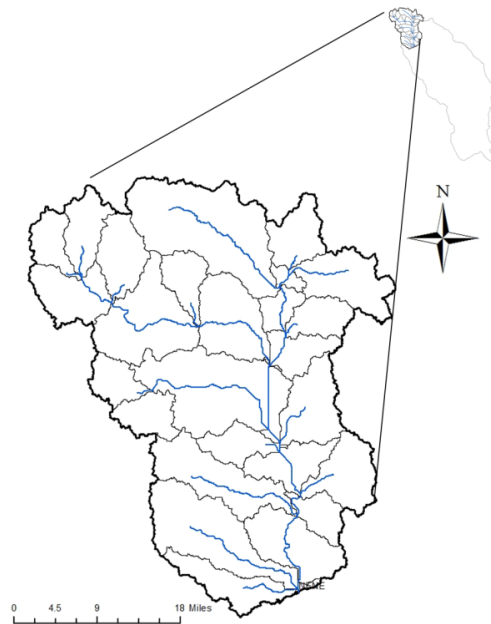


Figure 4.13 SWAT Model Watershed Delineation at NENE in the Neches River Basin



Figure 4.14 SWAT Model Watershed Delineation at NERO in the Neches River Basin

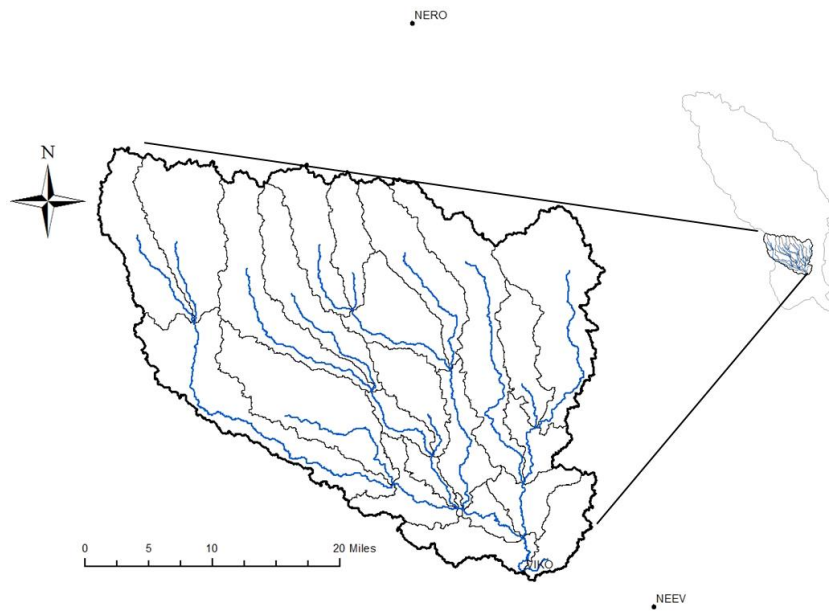


Figure 4.15 SWAT Model Watershed Delineation at VIKO in the Neches River Basin

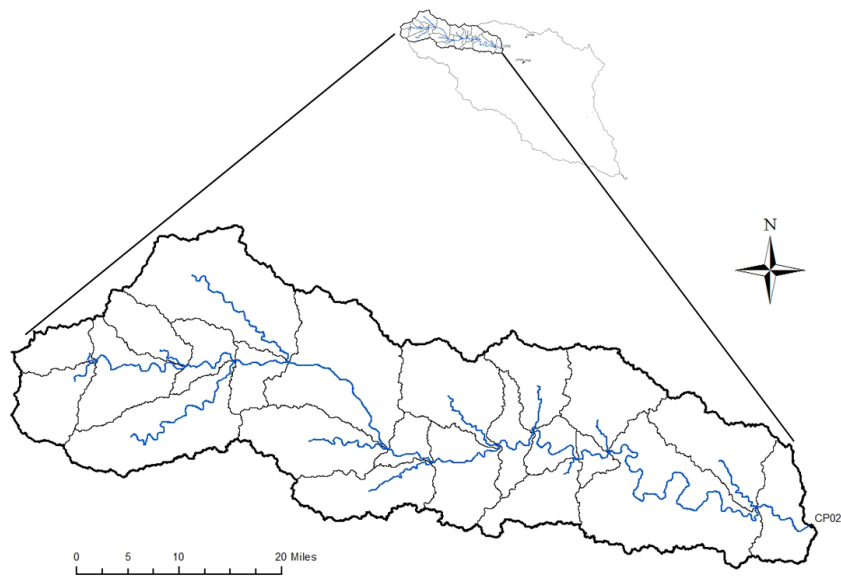


Figure 4.16 SWAT Model Watershed Delineation at CP02 in the GSA River Basins

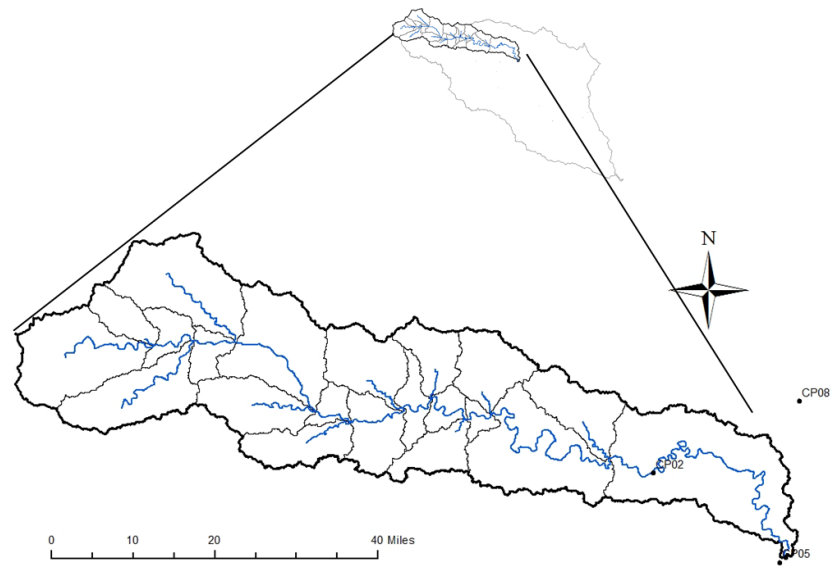


Figure 4.17 SWAT Model Watershed Delineation at CP04 in the GSA River Basins

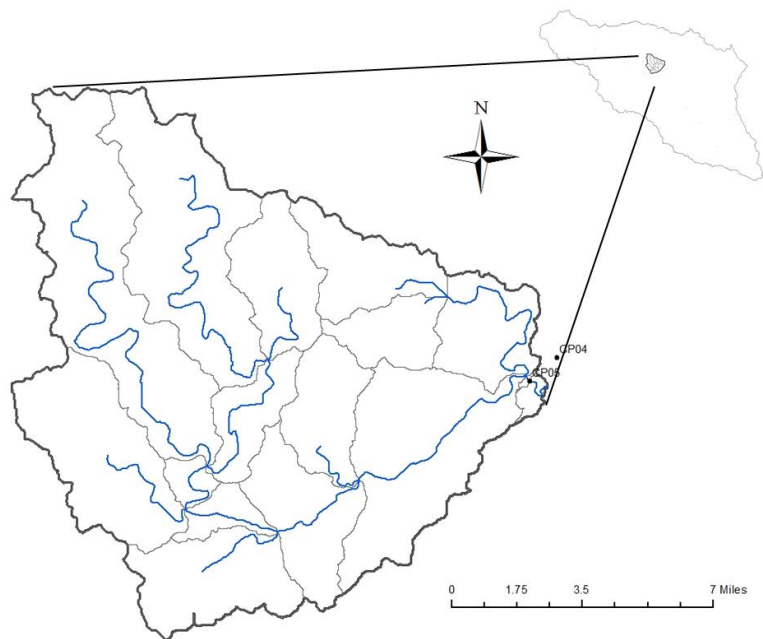


Figure 4.18 SWAT Model Watershed Delineation at CP05 in the GSA River Basins

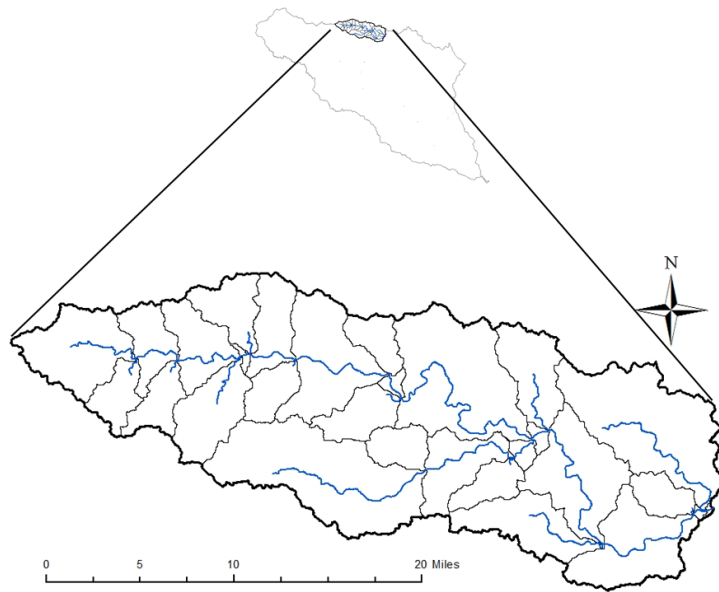


Figure 4.19 SWAT Model Watershed Delineation at CP08 in the GSA River Basins

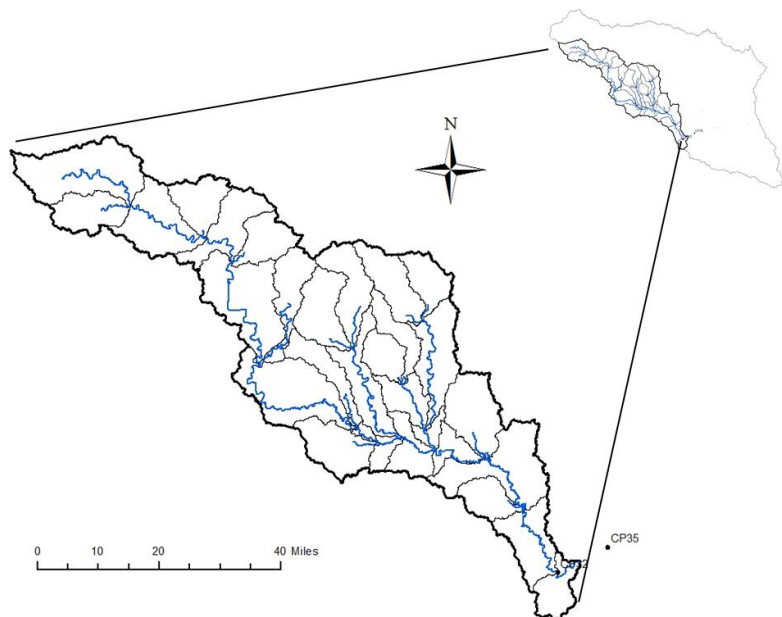


Figure 4.20 SWAT Model Watershed Delineation at CP32 in the GSA River Basins

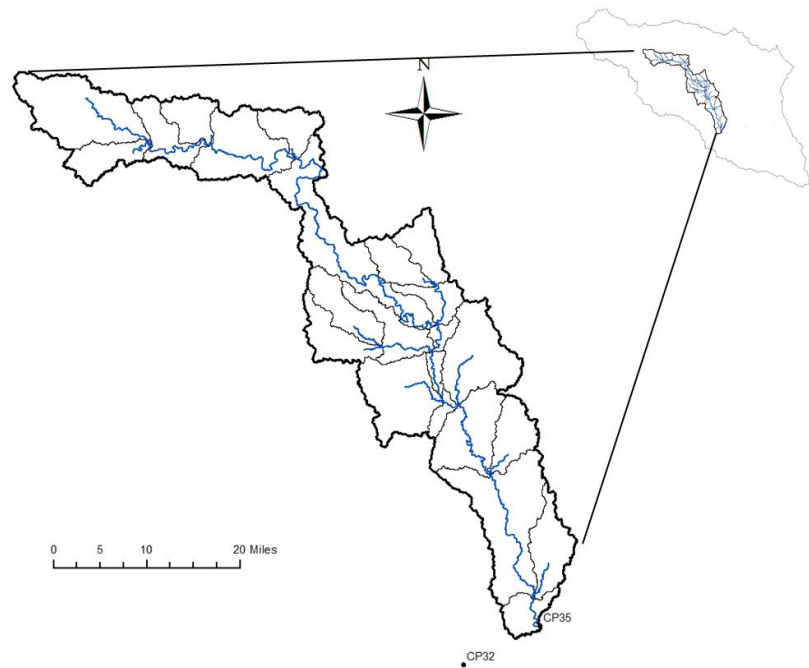


Figure 4.21 SWAT Model Watershed Delineation at CP35 in the GSA River Basins

4.3.2 Model Calibration

The recorded period and missing period are assumed for calibration of the models considering the periods-of-analysis of the three basins WAM. The periods-of-analysis are divided into warm-up, recorded and missing periods as listed in Table 4.5.

Table 4.5 Assumed Warm-up, Recorded, and Missing Periods

Basins	Period-of-Analysis	Warm-up Period	Recorded Period	Missing Period
Sabine	1940-1998 (59 yr)	1938-1949 (10 yr)	1950-1980 (31 yr)	1981-1998 (18 yr)
Neches	1940-1996 (57 yr)	1938-1949 (10 yr)	1950-1980 (31 yr)	1981-1996 (16 yr)
GSA	1934-1989 (56 yr)	1930-1949 (20 yr)	1950-1975 (26 yr)	1976-1989 (14 yr)

The parameters to be calibrated are listed in Section 2, and the calibration procedure using SWAT-CUP is as follows:

- 1) Determine calibrated parameters.
- 2) Run initial simulation and plot the simulated and recorded flow sequences.
- 3) Determine initial uncertainty range to each parameter within physically allowable ranges.
- 4) Run the SWAT-CUP 300 to 500 times and then check the result of objective function.
- 5) Determine again uncertainty range within physically allowable ranges according to suggested ranges by the Model.
- 6) Repeat the process 4 and 5 until to get maximum value of objective function.
- 7) Attaining the calibrated parameters and input the SWAT model.

Table 4.6 summarizes the calibration results of each SWAT model. The average R^2 considerably increases from 0.66 without calibration to 0.80 with calibration for the Sabine, from 0.73 to 0.87 for the Neches, and from 0.60 to 0.75 for the GSA, respectively. The R^2 at the control point CP05 in the GSA River Basins tremendously increases from 0.27 to 0.77 after calibration. The linear correlation relationships between monthly naturalized and synthesized flows by SWAT with and without calibration are plotted with linear regression equations and coefficients of determination (R^2) using Microsoft Excel as shown in Figures 4.22 to 4.38.

Table 4.6 Calibration Results of SWAT Models

Control Points	Coefficient of Determination (R^2) (Naturalized vs. SWAT Flows)	
	Without Calibration	With Calibration
<u>Sabine River Basin (Calibration Period: 1950 to 1980, 31 years)</u>		
LFQT	0.66	0.73
BSBS	0.64	0.77
SRGW	0.64	0.76
SRBE	0.63	0.83
SRBW	0.70	0.86
SRRL	0.71	0.87

Table 4.6 (Continued)

Control Points	Coefficient of Determination (R^2) (Naturalized vs. SWAT Flows)	
	Without Calibration	With Calibration
<u>Neches River Basin (Calibration Period: 1950 to 1980, 31 years)</u>		
ATCH	0.73	0.83
NEEV	0.77	0.91
NENE	0.68	0.85
NERO	0.74	0.90
VIKO	0.71	0.87
<u>GSA River Basins (Calibration Period: 1950 to 1975, 26 years)</u>		
CP02	0.64	0.73
CP04	0.62	0.71
CP05	0.27	0.77
CP08	0.75	0.78
CP32	0.64	0.70
CP35	0.70	0.81

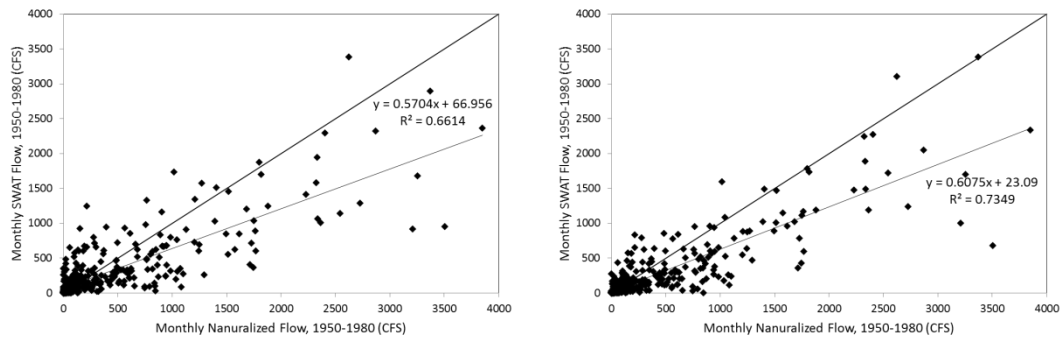


Figure 4.22 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at LFQT in the Sabine River Basin

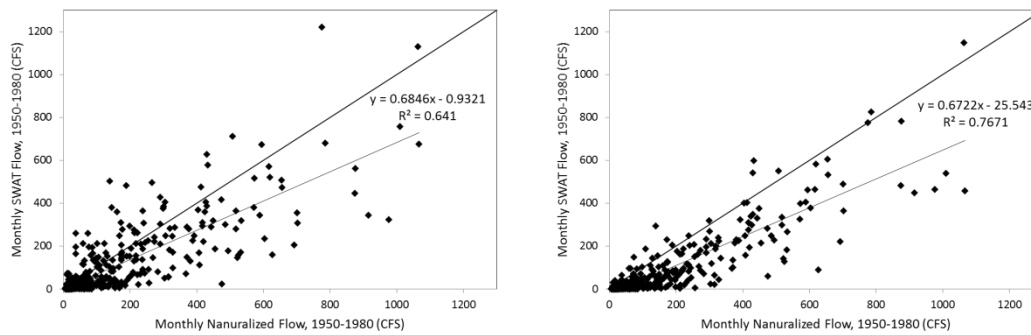


Figure 4.23 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at BSBS in the Sabine River Basin

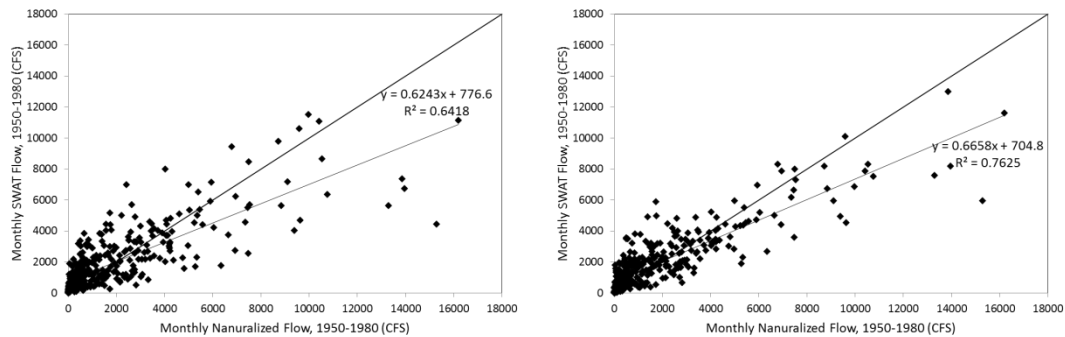


Figure 4.24 Linear Correlations between Monthly Naturalized and SWAT Flows with (below) and without (above) Calibration at SRGW in the Sabine River Basin

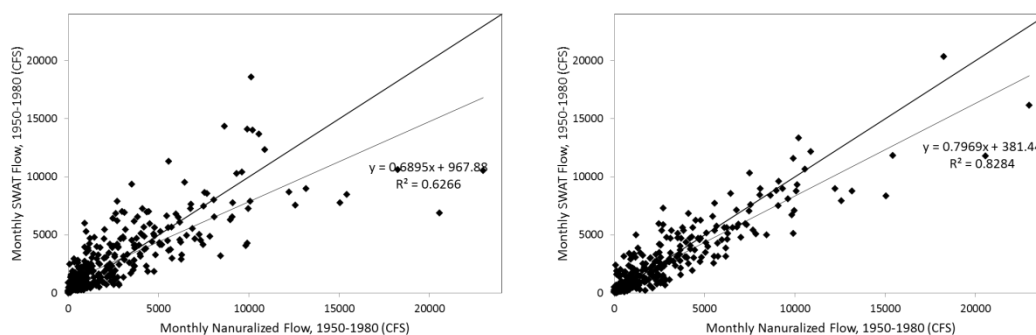


Figure 4.25 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at SRBE in the Sabine River Basin

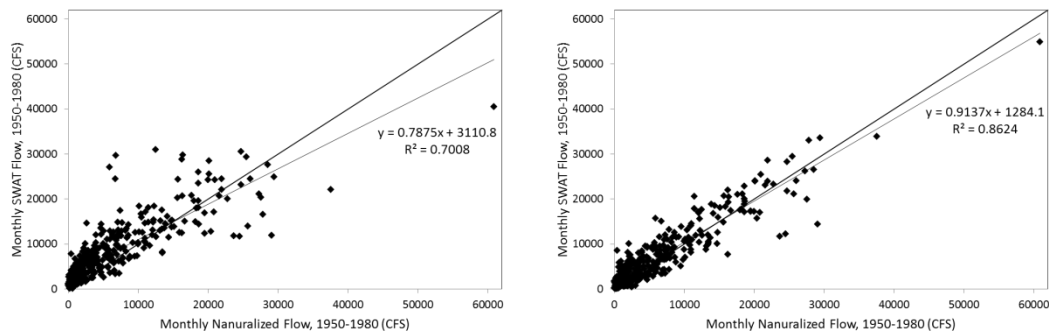


Figure 4.26 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at SRBW in the Sabine River Basin

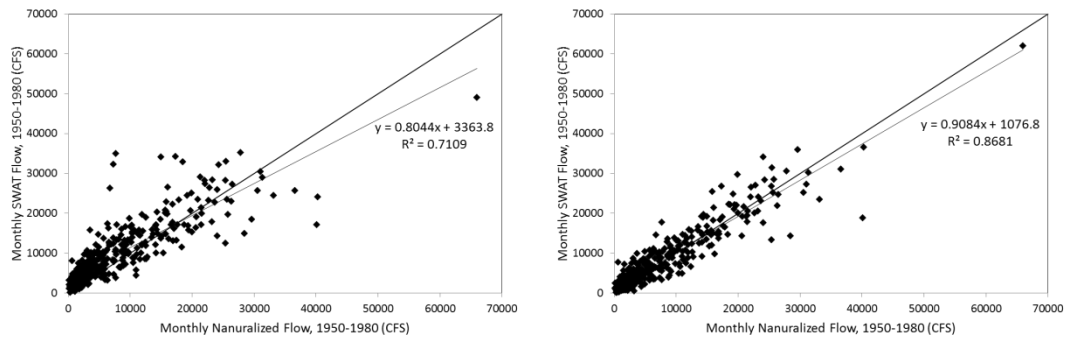


Figure 4.27 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at SRRL in the Sabine River Basin

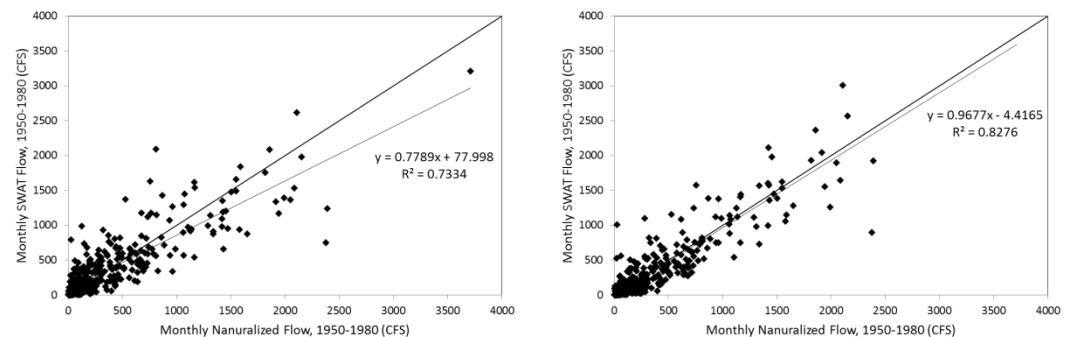


Figure 4.28 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at ATCH in the Neches River Basin

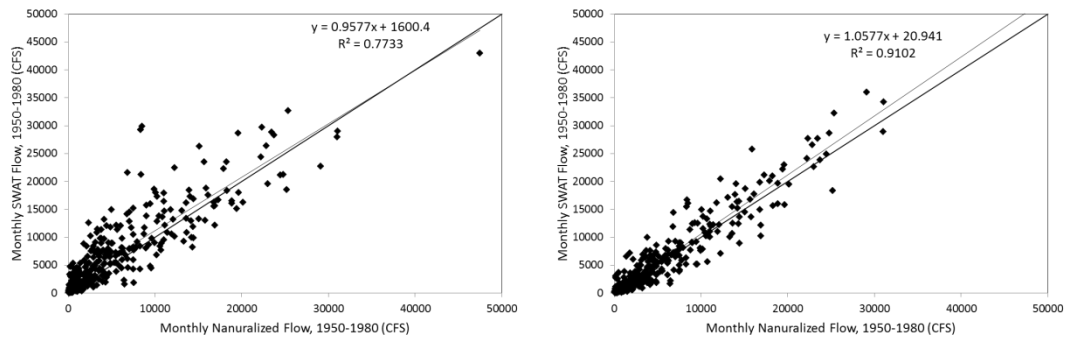


Figure 4.29 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at NEEV in the Neches River Basin

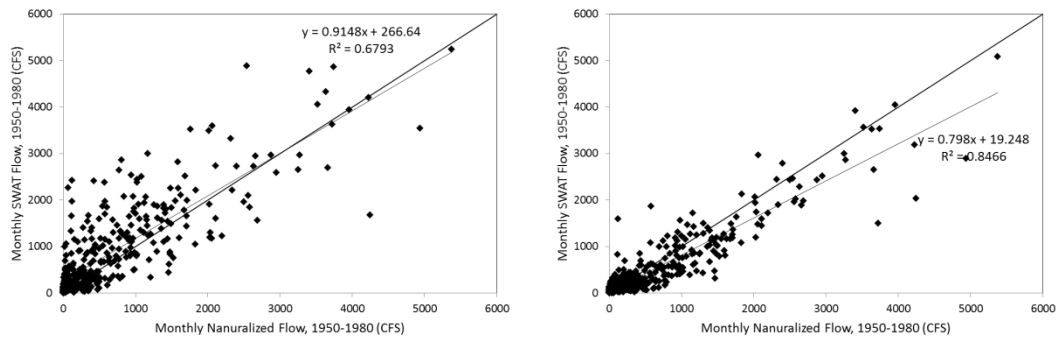


Figure 4.30 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at NENE in the Neches River Basin

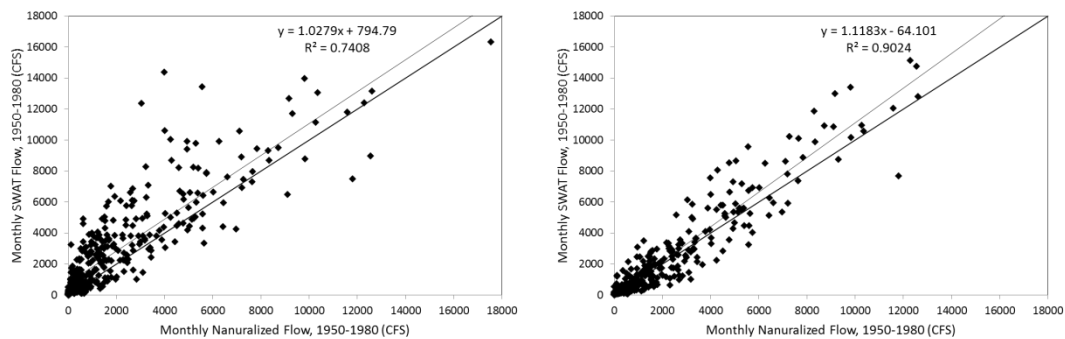


Figure 4.31 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at NERO in the Neches River Basin

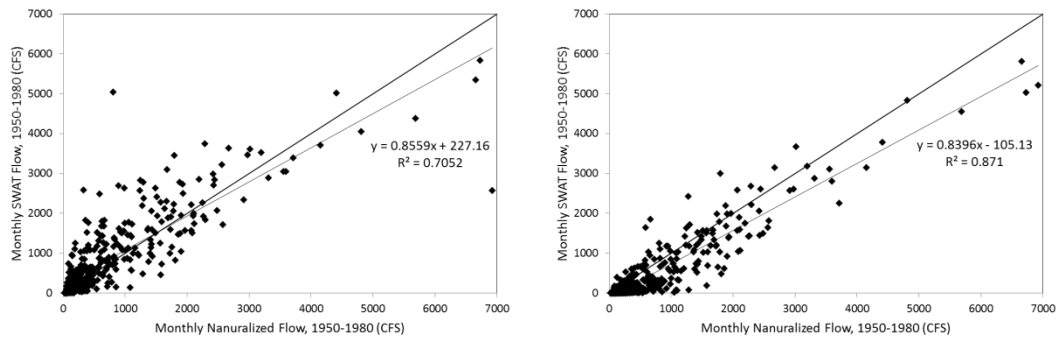


Figure 4.32 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at VIKO in the Neches River Basin

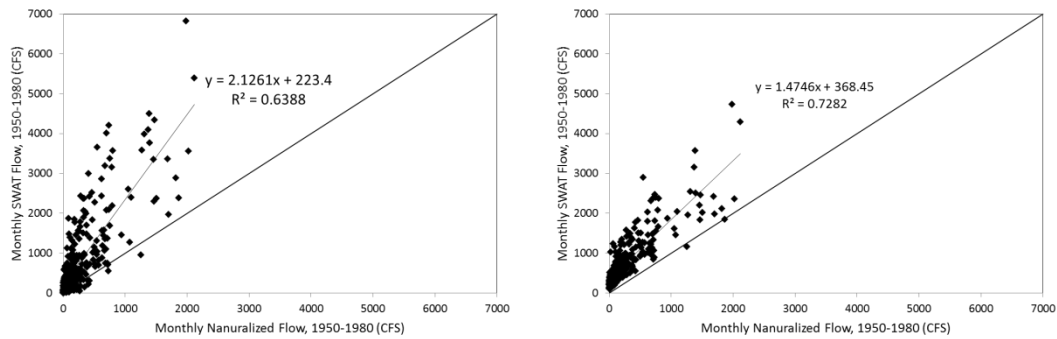


Figure 4.33 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at CP02 in the GSA River Basins

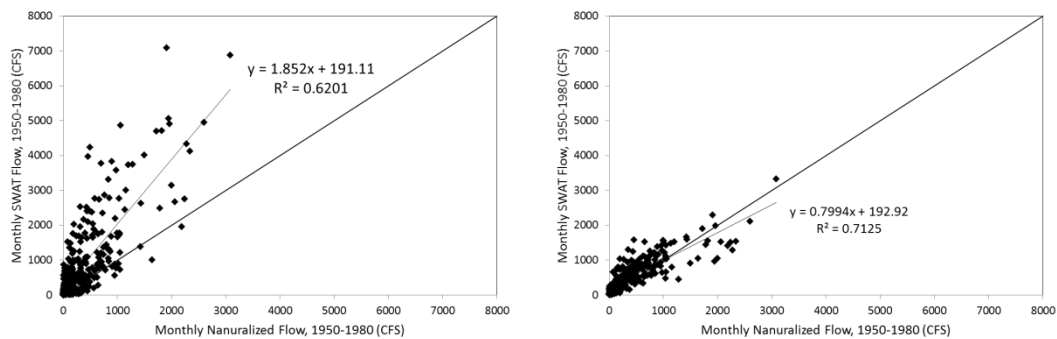


Figure 4.34 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at CP04 in the GSA River Basins

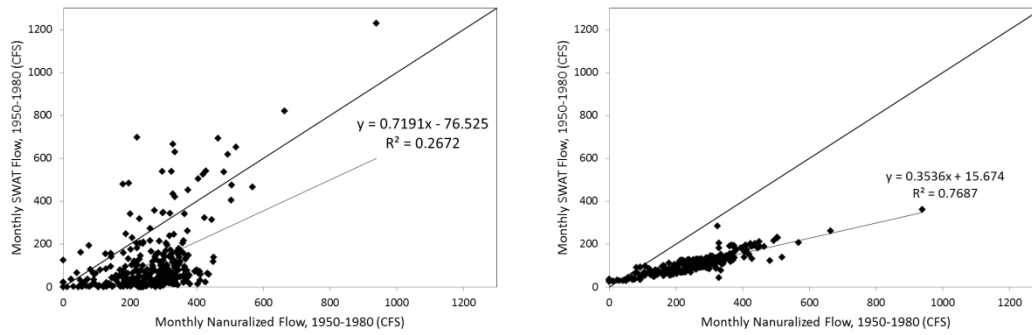


Figure 4.35 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at CP05 in the GSA River Basins

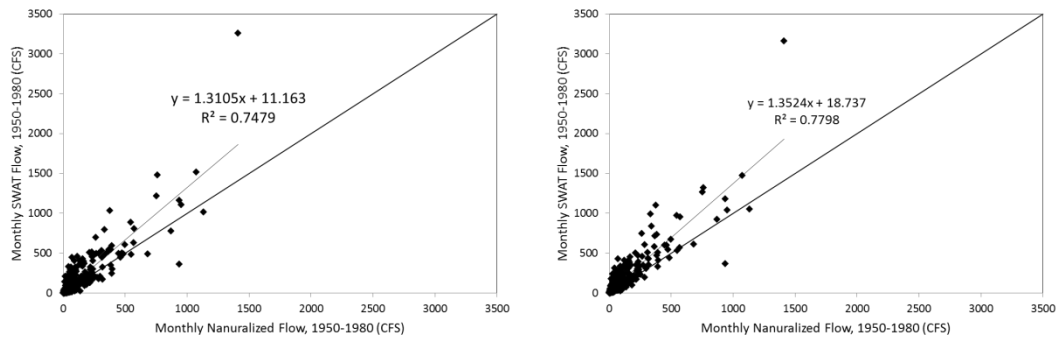


Figure 4.36 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at CP08 in the GSA River Basins

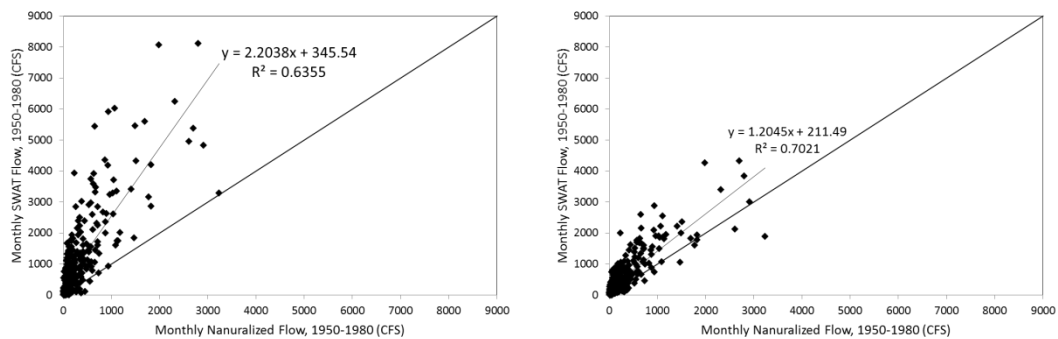


Figure 4.37 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at CP32 in the GSA River Basins

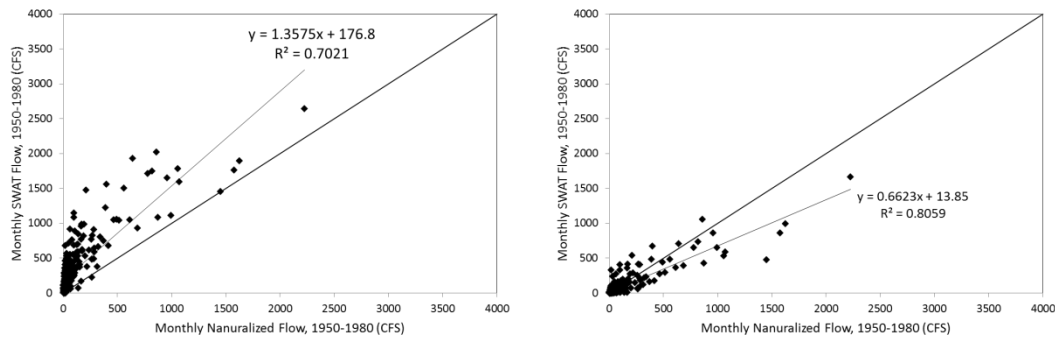


Figure 4.38 Linear Correlations between Monthly Naturalized and SWAT Flows with (right) and without (left) Calibration at CP35 in the GSA River Basins

4.4 Filling in Gaps of Missing Data with MOVE2

MOVE2 consists of two processes: (1) estimating unbiased mean and variance and (2) transferring flow sequences at a source site to a target site on the basis of estimated unbiased mean and variance. This method is expected to keep the homogeneity of flow sequences at a target site by this complexity process to estimate the unbiased mean and variance. Linear transfer methods generally tend to alter statistical characteristics of flow sequences at a target site, when filling in gaps of its missing period based on a source site.

Unbiased means and variances, called “population means and variances”, of flow sequences including the missing period at each control point for the three WAMs are calculated by Equation 2.2, 2.3, and 2.4 based on synthetic flow sequences by each SWAT model. Equation 2.2 estimates population means with linear correlation coefficient between two flow sequences for the recorded periods, numbers of data, and each mean and variance for recorded period. Equation 2.3 estimates population variances with various variables like equation 2.3 and noises by equation 2.4. Table 4.7 summarizes unbiased means and variance of flow sequences at control points in the Sabine, the Neches, and the GSA River basins, respectively.

Equation 2.1 transfers the flows sequences for filling gaps of missing data on the basis of synthesized flow sequences by SWAT. Equation 2.1 may generate negative values if the difference between flow value and mean is bigger than the estimated mean. If

negative values are generated in transferring flow values, the negative values are replaced with zero flow.

Table 4.7 Unbiased Means and Variances at Control Points in the Sabine River Basin

Control Points	Product Moment Correlation Coefficient (NF vs. SWAT)	Alpha (α)	Estimated Unbiased (1950-1998)		WAM Datasets (1950-1998)	
			Mean m(y) (CFS)	Variance S(y) (CFS)	Mean m'(y) (CFS)	Variance S'(y) (CFS)
<u>Sabine</u>						
LFQT	0.86	1.00	517.5	715.5	459.9	661.2
BSBS	0.88	1.00	175.6	193.1	181.5	201.1
SRGW	0.87	1.00	2,080.4	2,582.7	2,062.5	2,617.6
SRBE	0.91	1.00	2,895.2	3,246.0	2,454.9	3,246.1
SRBW	0.93	1.00	7,285.0	7,748.1	7,062.8	7,734.2
SRRL	0.93	1.00	8,982.7	8,653.1	8,453.2	8,525.2
<u>Neches</u>						
ATCH	0.91	1.01	446.5	528.9	435.8	542.4
NEEV	0.95	1.01	6,551.5	6,782.3	5,926.3	6,743.4
NENE	0.92	1.01	829.8	974.0	794.2	887.9
NERO	0.95	1.01	2,458.0	2,670.4	2,270.9	2,789.8
VIKO	0.93	1.01	817.9	1,023.6	845.7	1,021.0
<u>GSA</u>						
CP02	0.87	1.01	339.4	429.3	365.7	550.4
CP04	0.85	1.01	473.2	568.1	479.1	626.0
CP05	0.87	1.01	292.0	105.0	276.3	109.0
CP08	0.89	1.01	134.4	188.5	137.4	201.7
CP32	0.85	1.01	380.0	494.7	422.7	660.6
CP35	0.90	1.01	122.1	272.5	113.9	260.1

4.5 Comparative Analysis

4.5.1 Statistical Evaluation

The method performance is evaluated with two different statistical methods: the Nash-Sutcliffe efficiency (NSE) and comparison of statistic parameters. NSE is a normalized statistic to be determined by comparing the relative magnitude of the residual variance (“noise”) to the measured or recorded data variance (“information”) (Nash and Sutcliffe, 1970). NSE ranges from $-\infty$ to 1.0. If the NSE value is 1.0, that means the optimal values. NSE values between 0.0 and 1.0 are generally considered acceptable levels of performance while values of less than 0.0 indicate unacceptable performance. Moriasi *et al.* (2007) suggested that if NSE is more than 0.5, model performance can be evaluated as “satisfactory” through a thorough review of relevant literature.

Table 4.8 summarizes the performances at each control point with NSE values with and without MOVE2. The flow sequences synthesized with the calibrated SWAT models (the source flows) have NSE values ranging from -3.03 to 0.85, and the flow sequences (the infilled flows) transferred by MOVE2 based on the synthesized flow sequences by the calibrated SWAT models have a range of NSE values from 0.48 to 0.86 with the naturalized flow data for the assumed missing period at all 17 control points.

Table 4.9 also summarizes the statistical parameters of the flow sequences for missing periods at each control point for the Sabine, the Neches, and the GSA River Basins. This shows that the statistical parameters (mean and variance) of the infilled flows are closer to the parameters of the flow sequences of WAM datasets than sources for missing periods at 11 out of 17 control points.

In further detail for each basin, the infilled flows have satisfied NSE values with the naturalized flow data at the six control points in the Sabine River basin. MOVE2 makes the source flows close to the naturalized flow data at the control point BSBS, while this method spoils NSE value from 0.71 to 0.56, even satisfied, at the control point LFQT. However, MOVE2 shows obviously satisfied performances at all the 6 control points.

Table 4.8 Summary of Method Performance Evaluation
by Nash-Sutcliffe Efficiency

Control Points	Nash-Sutcliffe Efficiency (NSE) (Naturalized vs. Infilled flows)	
	SWAT Flow Only	MOVE2 Based SWAT Flow
<u>Sabine River Basin (Missing Period: 1981 to 1998, 18 years)</u>		
LFQT	0.71	0.56
BSBS	0.44	0.73
SRGW	0.70	0.71
SRBE	0.83	0.83
SRBW	0.85	0.85
SRRL	0.85	0.85
<u>Neches River Basin (Missing Period: 1981 to 1996, 16 years)</u>		
ATCH	0.78	0.79
NEEV	0.82	0.86
NENE	0.78	0.64
NERO	0.84	0.86
VIKO	0.52	0.61
<u>GSA River Basins (Missing Period: 1976 to 1989, 14 years)</u>		
CP02	0.38	0.77
CP04	0.72	0.73
CP05	-3.03	0.48
CP08	0.68	0.83
CP32	0.63	0.57
CP35	0.70	0.61

The infilled flows have satisfied NSE values with the naturalized flow data at the five control points in the Neches River basin. MOVE2 generally improves NSE values at all control points, while NSE value decreases from 0.78 to 0.64 at the control point NENE after infilling with MOVE2. However, it is also made within the level of satisfactory like the control point LFQT in the Sabine River Basin.

The NSE values increase at the control points CP02, CP04, CP05, and CP08, and slightly decrease at the control points CP32 and CP35 after infilling by MOVE2 in the

GSA River Basins. MOVE2 makes the flow characteristics tremendously close to the naturalized flow data at the control points CP02 and CP05. MOVE2 enhances NSE values from 0.38 to 0.77 for the control point CP02, and NSE values from -3.03 to 0.48 for the control point CP05, respectively.

Table 4.9 Statistical Parameters of the Flow Sequences for Missing Periods

Control Points	Infilled Flows by MOVE2 (1981-1998)		WAM Dataset (1981-1998)		Sources(SWAT) Flows (1981-1998)	
	Mean m(y) (CFS)	Variance S(y) (CFS)	Mean m'(y) (CFS)	Variance S'(y) (CFS)	Mean m''(y) (CFS)	Variance S''(y) (CFS)
<u>Sabine</u>						
LFQT	669.1	808.3	542.9	659.8	447.7	588.5
BSBS	183.1	182.9	203.8	205.9	96.8	139.4
SRGW	2,350.1	2,444.7	2,348.5	2,612.6	2,211.1	1,909.9
SRBE	3,322.0	3,179.8	3,199.0	3,181.3	2,961.5	2,781.7
SRBW	8,443.3	8,332.6	8,313.7	8,192.9	8,884.1	8,263.4
SRRL	10,281.8	9,104.6	9,451.6	8,759.4	10,241.5	8,923.6
<u>Neches</u>						
ATCH	525.5	582.2	535.1	604.8	496.7	626.1
NEEV	7,938.2	7,165.4	7,039.3	7,083.3	8,022.2	7,976.3
NENE	1,004.2	1,119.8	794.2	887.9	812.3	985.5
NERO	2,924.1	2,711.8	2,738.8	3,037.0	3,038.7	3,198.1
VIKO	875.3	1,058.0	998.5	1,031.5	617.2	953.9
<u>GSA</u>						
CP02	408.2	494.6	511.4	743.0	985.2	884.0
CP04	593.7	677.1	631.4	793.0	691.2	673.9
CP05	342.2	82.8	309.2	99.6	126.2	32.6
CP08	144.0	180.6	161.2	219.0	212.3	275.3
CP32	421.9	499.1	576.5	865.2	718.9	723.2
CP35	131.2	302.9	114.5	265.8	100.5	227.0

Linear correlation with a regression equation can intuitively demonstrate how much or well MOVE2 changes the statistic parameters of the source flows similar to the

statistic parameters of the naturalized flow data. A linear regression equation without intercept can be expressed as $Y=BX$. B is a slope of relationship between A and B. If the statistic characteristics of Ys are almost similar to Xs, the slope should be 1. Therefore, a slope of a linear regression equation without intercept represents statistic similarity of both time series. The linear relationships between naturalized flows and the source flow and the infilled flow at all control points are as shown in Figures 4.39 to 4.55.

In the Sabine River Basin, Figure 4.40 especially shows that MOVE2 changes the flow characteristics of the source flow close to the characteristics of the naturalized flow data. However, if the sources flows are originally developed close to the naturalized flow data like the control points SRBW and SRRL, the modification by MOVE2 is marginal as shown in Figures 4.43 and 4.44.

Flow characteristics of the infilled flows are slighted adjusted to the naturalized flows at all the control points in the Neches River Basin as shown in Figures 4.45 to 4.48. This is because the source flows are well developed in general. However, if there some difference between the source flow and the naturalized flow like at the control point VIKO, MOVE2 also adjust flow characteristics of the source flow similar to the naturalized flow as shown in Figure 4.49.

In the GSA River Basins, the improvement of flow characteristics by MOVE2 is more obvious than other two river basins as shown in Figures 4.50 to 4.55. The source flow has totally different flow characteristics than the naturalized flow at the control point CP05 due to a spring flow source within its watershed. However, MOVE2 surprisingly changes the characteristics of the source flow similar to the naturalized flow at the control point CP05 as shown in Figure 4.53, if both flows have highly linear correlation. Even though, this regression equation intuitionally shows the alteration of flow characteristics, it sometimes could provide wrong information due to over or under estimated maximum values as shown in Figures 4.50 and 4.54. These two figures shows that MOVE2 spoils flow characteristics of the sources flows at CP02 and CP32, but Table 4.9 indicates that this method really changes overestimated means of the sources flows close to the means of the naturalized flows at the two control points. This problem can be addressed by the flow frequency analysis in the next section.

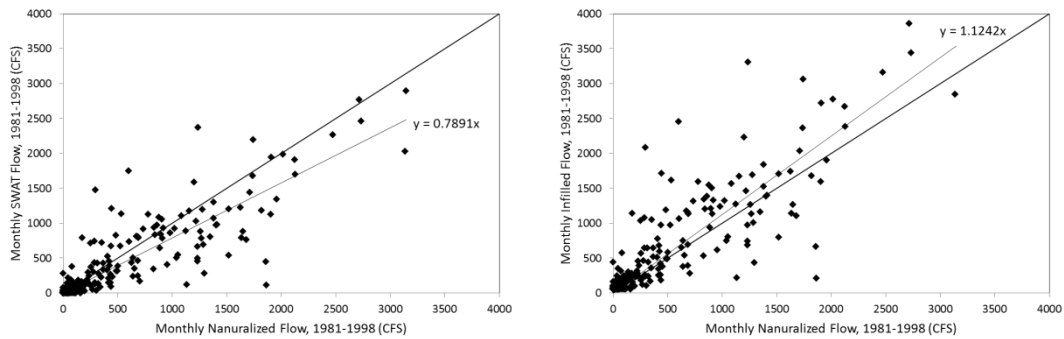


Figure 4.39 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at LFQT in the Sabine River Basin

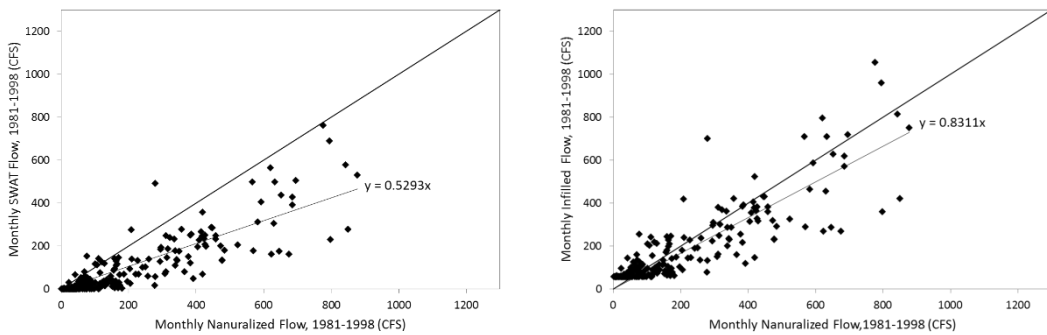


Figure 4.40 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at BSBS in the Sabine River Basin

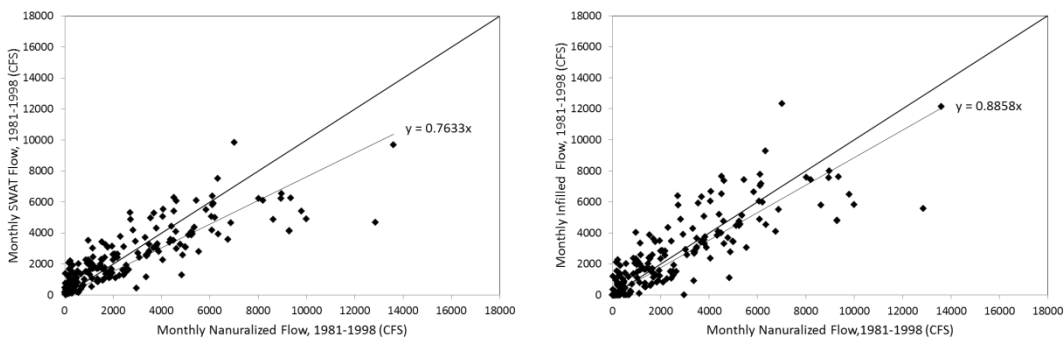


Figure 4.41 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at SRGW in the Sabine River Basin

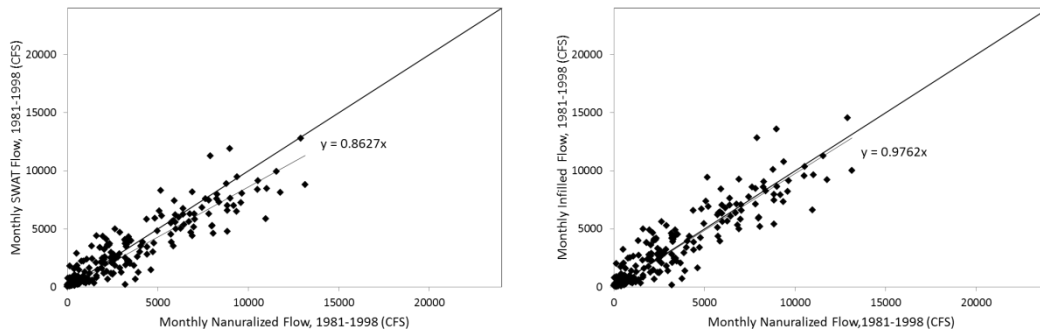


Figure 4.42 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at SRBE in the Sabine River Basin

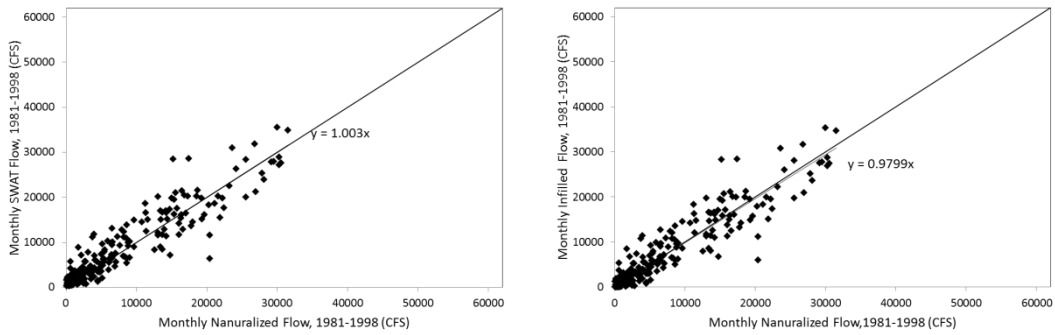


Figure 4.43 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at SRBW in the Sabine River Basin

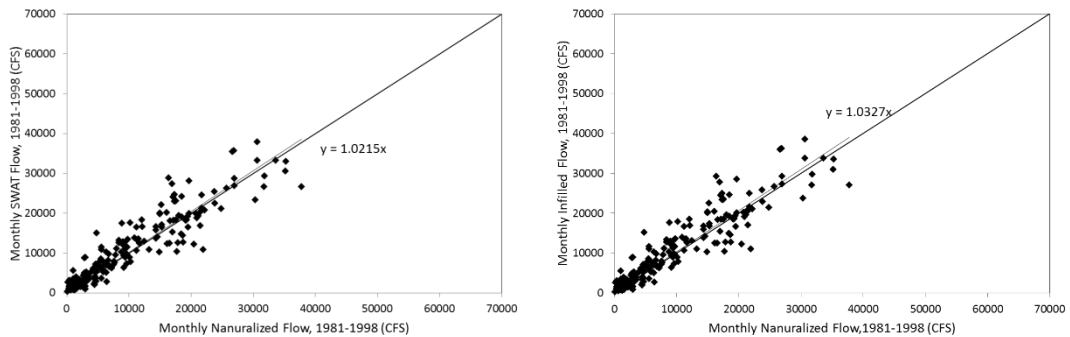


Figure 4.44 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at SRRL in the Sabine River Basin

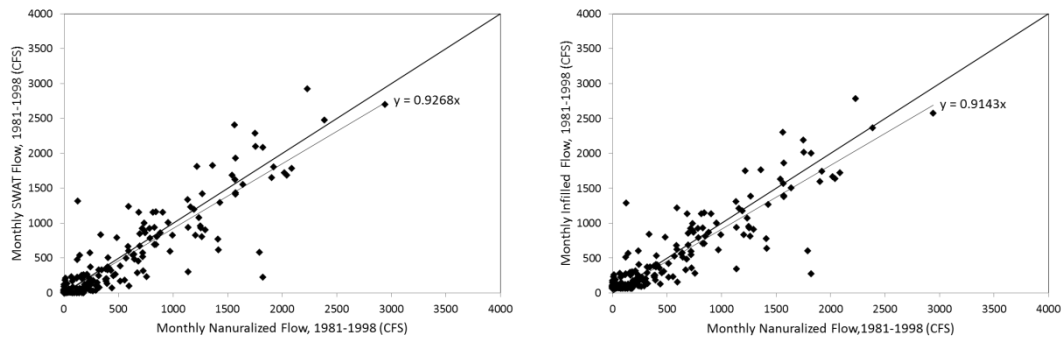


Figure 4.45 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at ATCH in the Neches River Basin

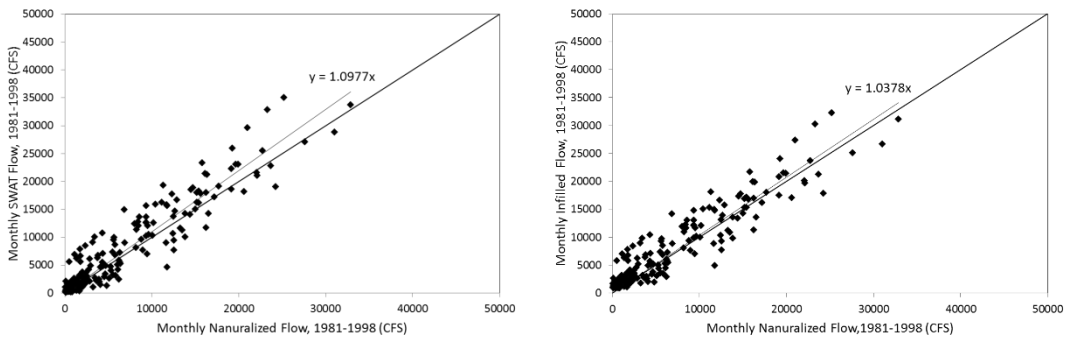


Figure 4.46 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at NEEV in the Neches River Basin

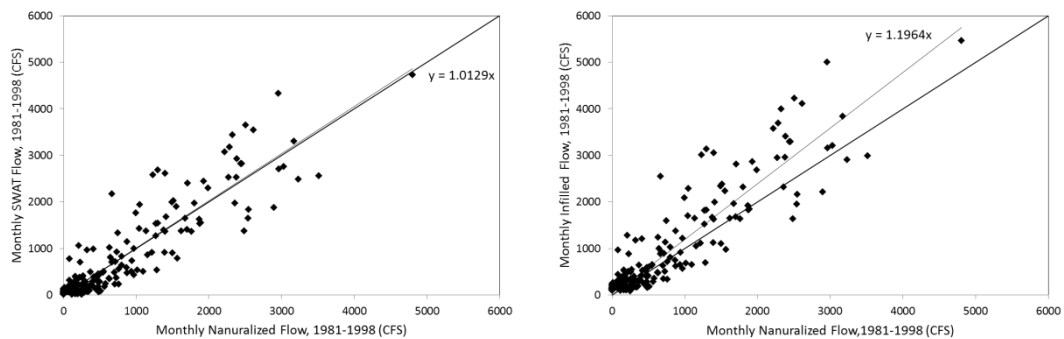


Figure 4.47 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at NENE in the Neches River Basin

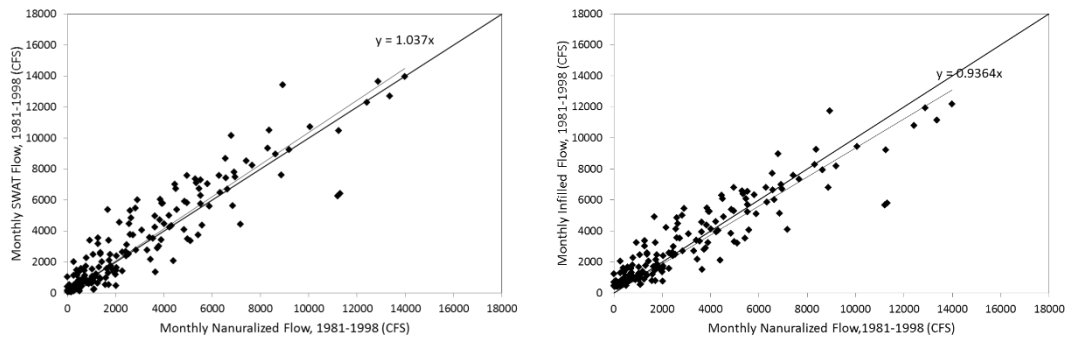


Figure 4.48 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at NERO in the Neches River Basin

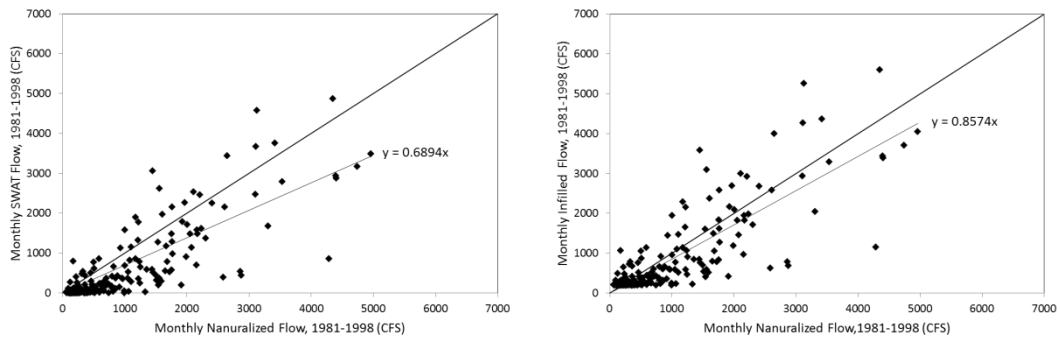


Figure 4.49 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at VIKO in the Neches River Basin

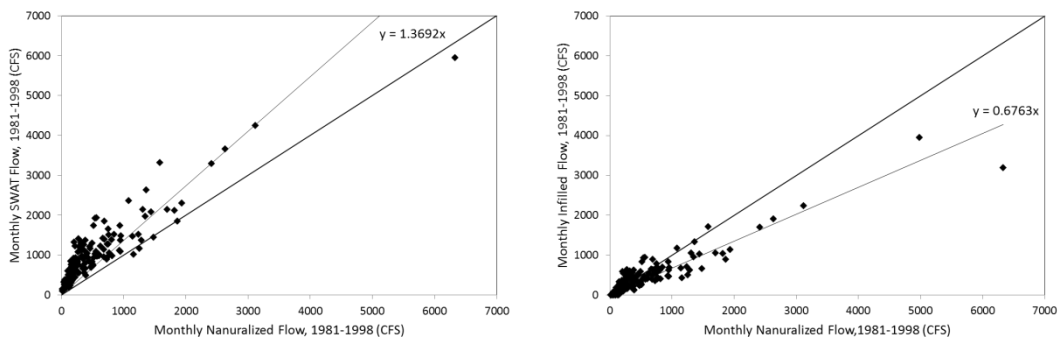


Figure 4.50 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at CP02 in the GSA River Basins

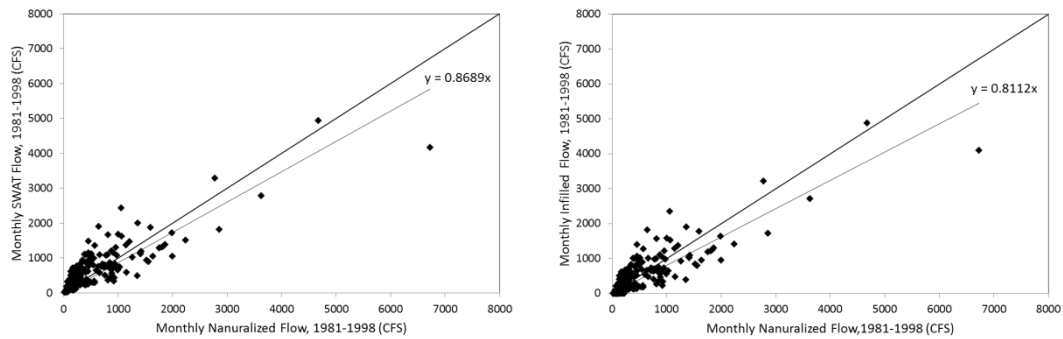


Figure 4.51 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at CP04 in the GSA River Basins

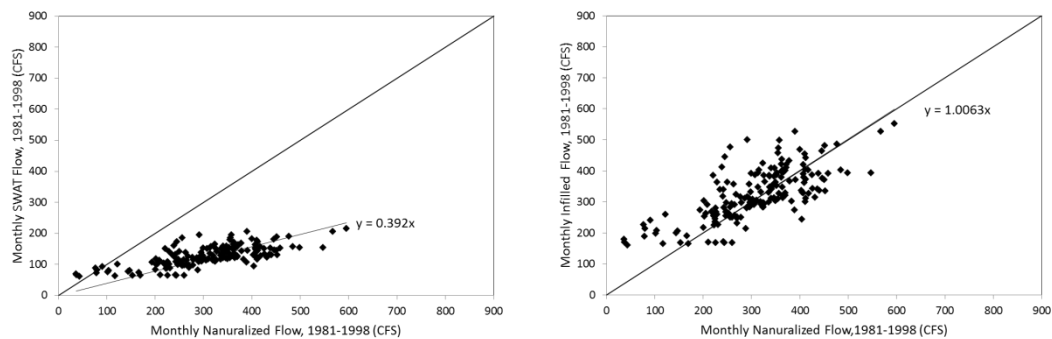


Figure 4.52 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at CP05 in the GSA River Basins

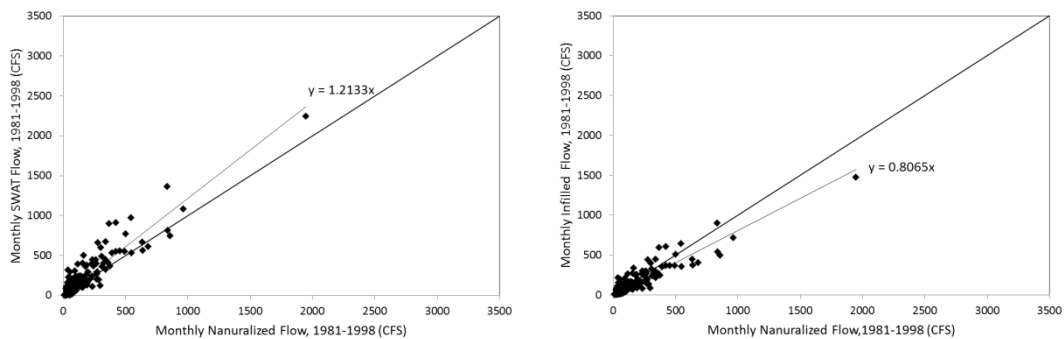


Figure 4.53 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at CP08 in the GSA River Basins

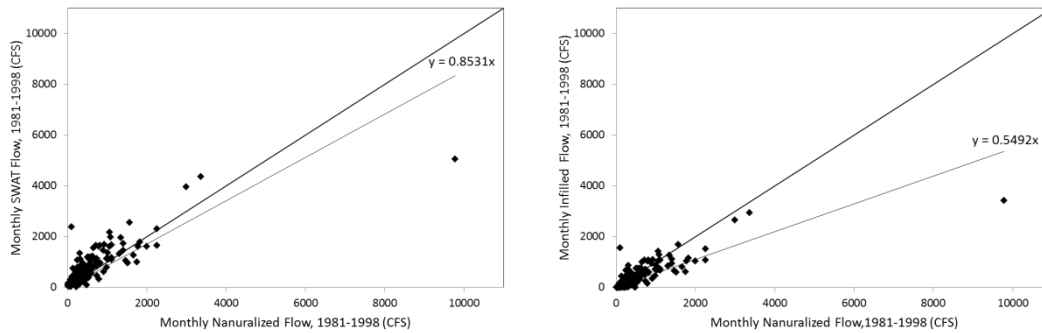


Figure 4.54 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at CP32 in the GSA River Basins

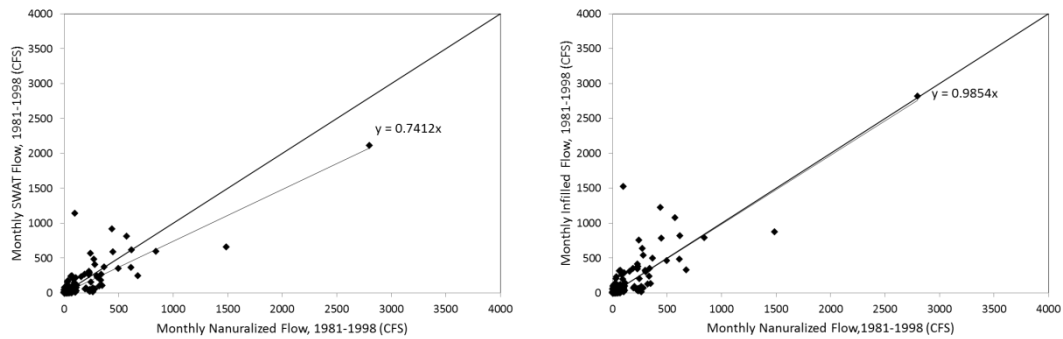


Figure 4.55 Regression Equations between Monthly Naturalized and Infilled Flows with (right) and without (left) MOVE2 at CP35 in the GSA River Basins

4.5.2 Comparison of Flow Frequencies

A method for filling gaps of missing data should conserve not only statistical stationarity but also flow characteristics. Flow duration curves and flow frequency metrics are widely used for evaluation of flow characteristics. Flow duration curves commonly provide a graphical illustration of the overall hydrologic state of flow sequences (Vogel et al, 2007).

Flow duration curves of naturalized flow, the source flow, and the infilled flow at each control point for assumed missing periods are plotted in Figures 4.56 to 4.72. These figures indicate that the infilled flows at most control points are more similar to flow characteristics of naturalized flow than the source flows.

The flow frequencies of the infilled flows are very similar to the flow frequencies of the naturalized flow data for the assumed missing period at all the six primary control points in the Sabine River Basin. MOVE2 adjusts the flow frequency relationships similar to the naturalized flows at the control points LFQT, BSBS, SRGW, and SRBE, relative upstream area, than control points SRBW and SRRL, relative downstream area.

The flow frequencies of the infilled flows are also very close to the flow frequencies of the naturalized flow data for the assumed missing period at all the five primary control points in the Neches River Basin. Flow frequency of the source flow is also changed similar to the naturalized flows at the control point VIKO by MOVE2 like other evaluation methods.

The flow frequencies of the infilled flows are very similar to the flow frequencies of the naturalized flow data for the assumed missing period at all the six primary control points in the GSA River Basins. It is more apparently that MOVE2 modifies the flow frequency similar to the naturalized flows at the control points CP02, CP05, CP08, and CP32. The flow frequency of the source flow is totally different from the naturalized flow at the control point CP05. MOVE2 dramatically changes the flow frequency nearly similar to the naturalized flow at the control point CP05. Although it could be evaluated that flow characteristics of the infilled flows are different from the naturalized flows at CP02 and CP32 through the regression equations between both flows, this flow frequency analysis proves that the infilled flows at the two points have similar flow characteristics to the naturalized flows.

These evaluations show that the suggested method is a good solution for infilling gaps of missing data, if there is no linear correlated data at near sites.

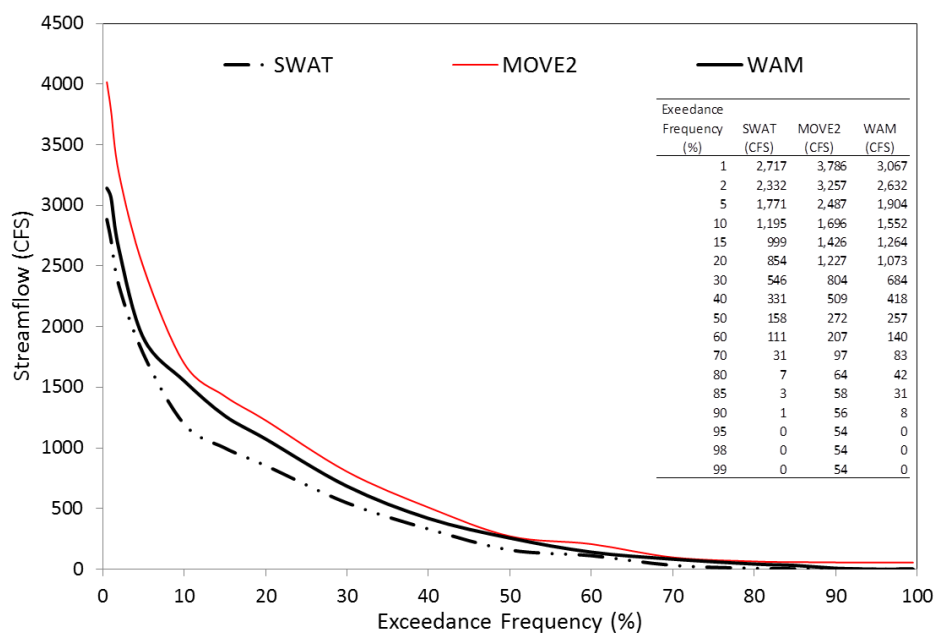


Figure 4.56 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at LFQT in the Sabine River Basin

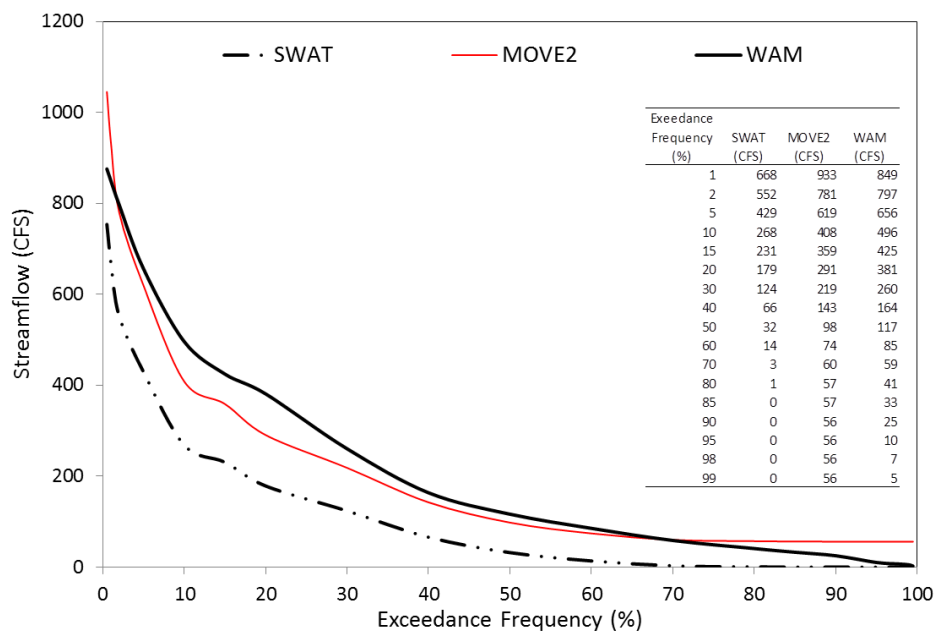


Figure 4.57 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at BSBS in the Sabine River Basin

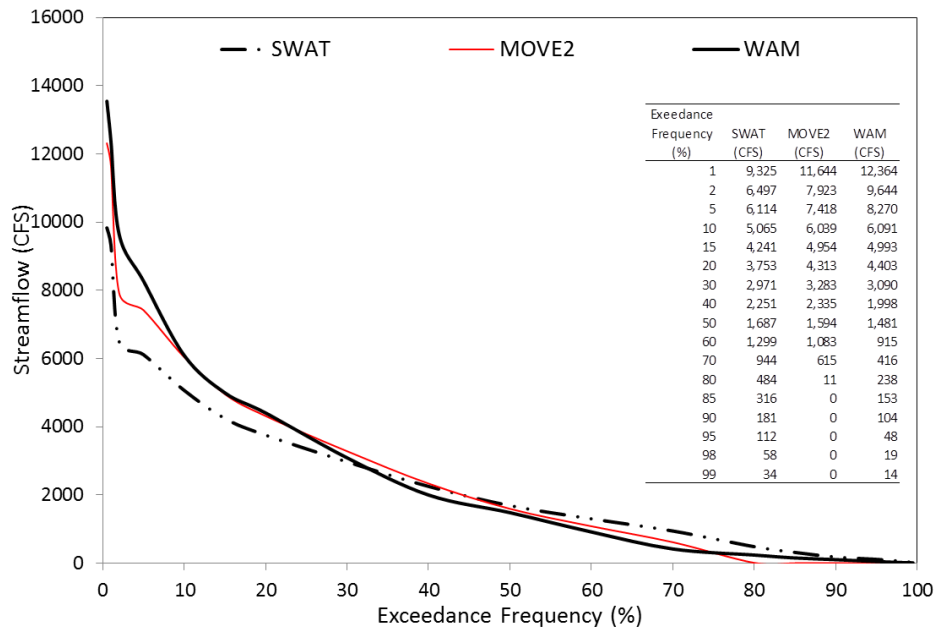


Figure 4.58 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at SRGW in the Sabine River Basin

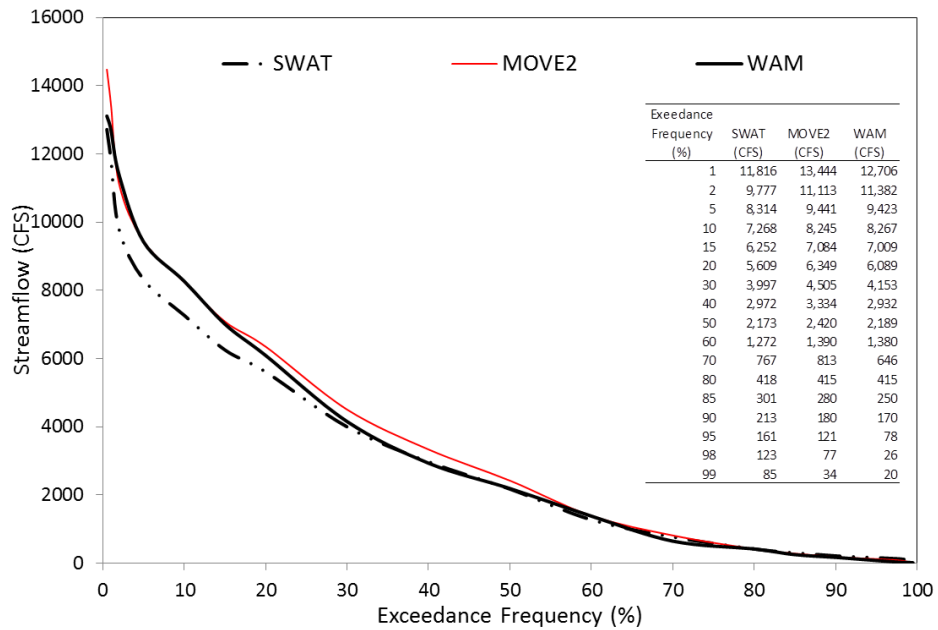


Figure 4.59 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at SRBE in the Sabine River Basin

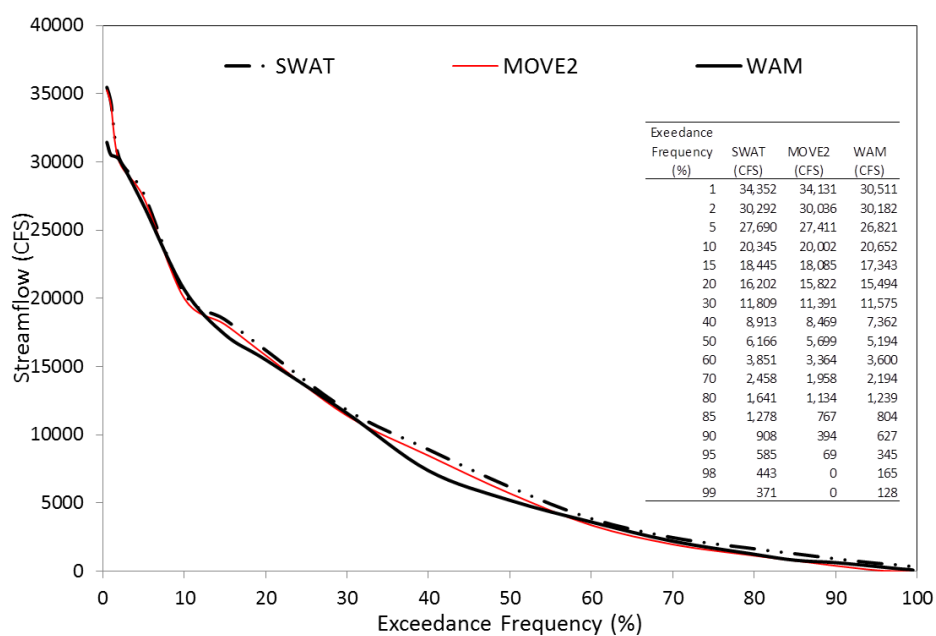


Figure 4.60 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at SRBW in the Sabine River Basin

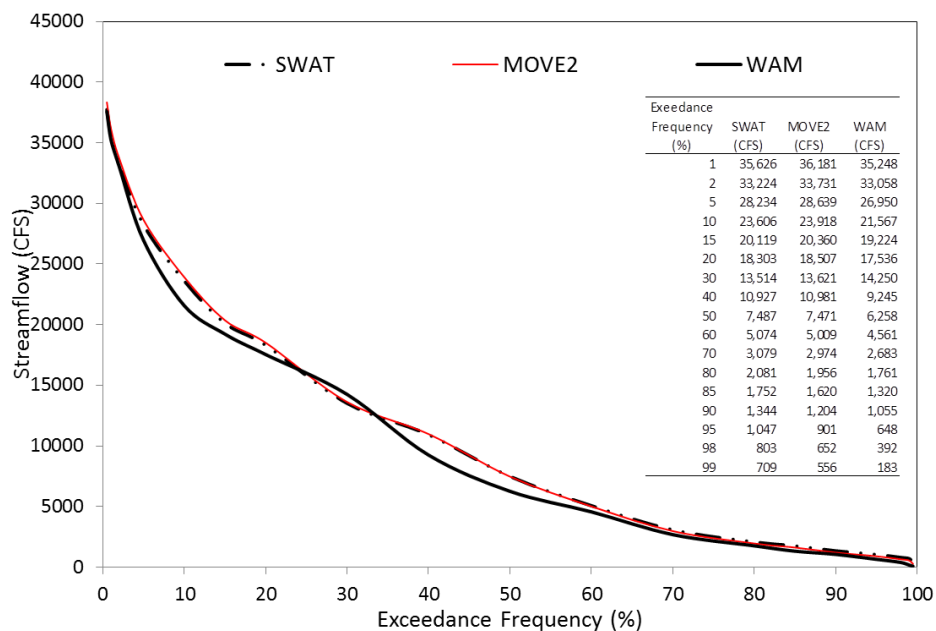


Figure 4.61 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at SRRL in the Sabine River Basin

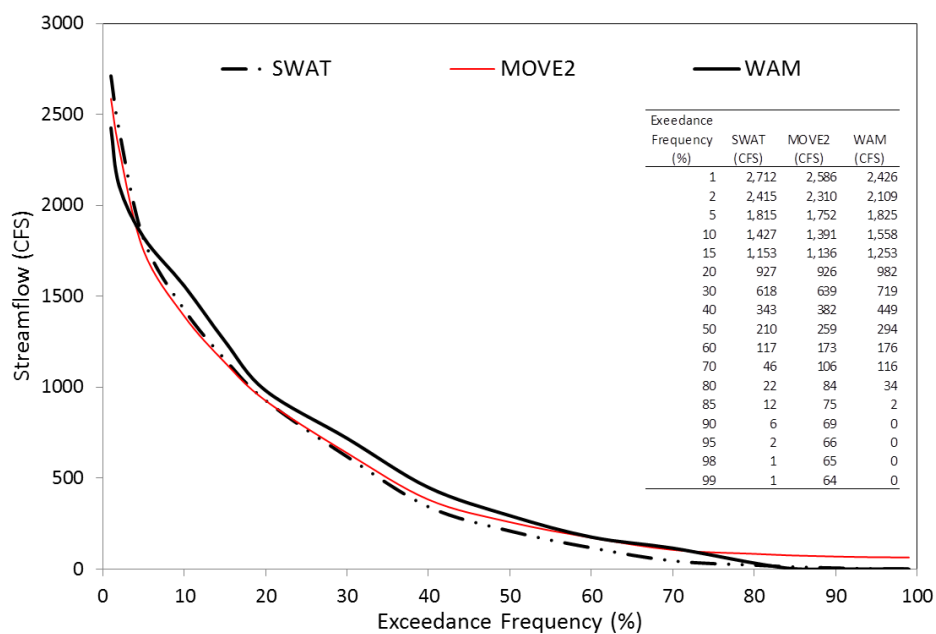


Figure 4.62 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at ATCH in the Neches River Basin

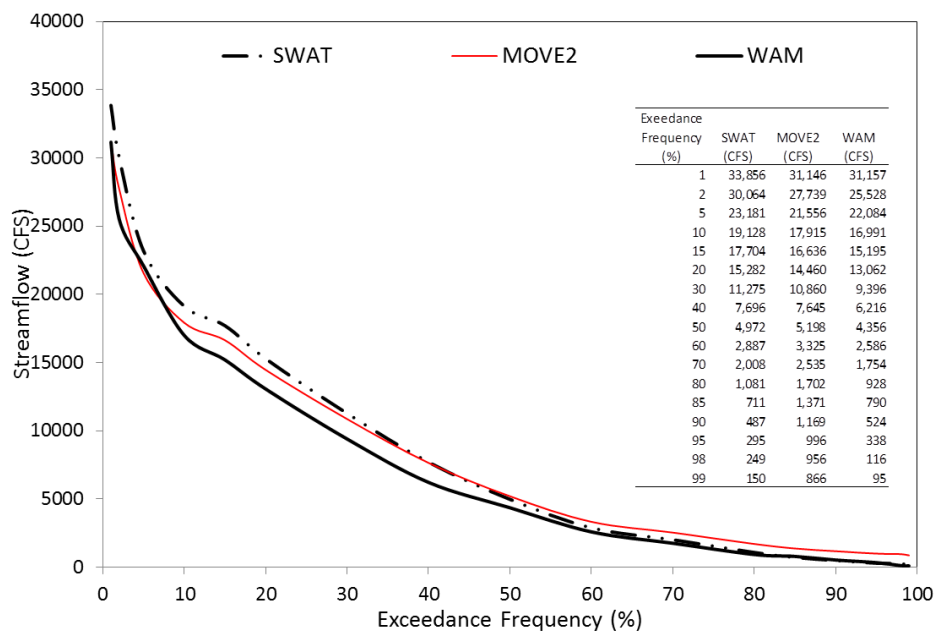


Figure 4.63 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at NEEV in the Neches River Basin

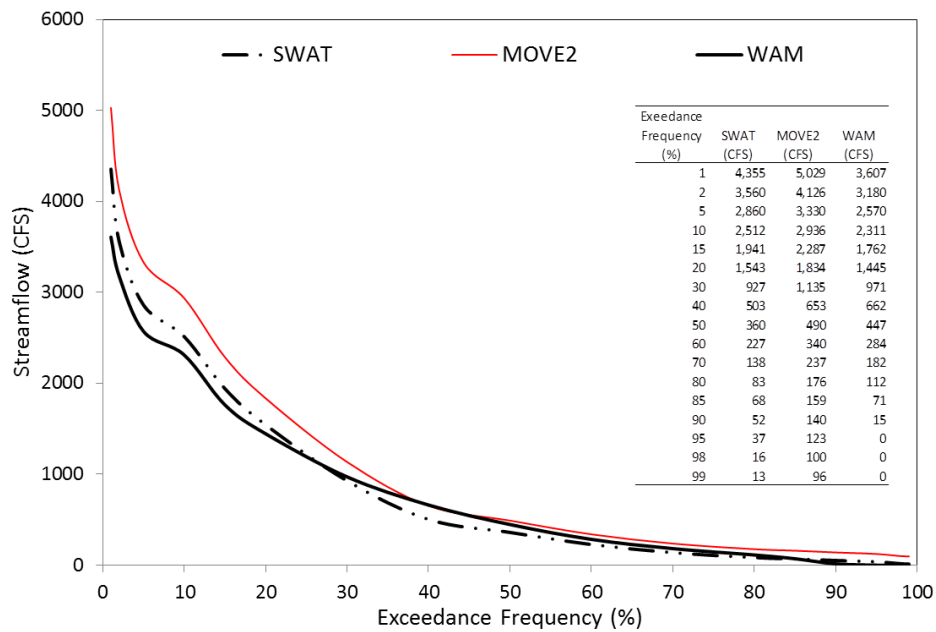


Figure 4.64 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at NENE in the Neches River Basin

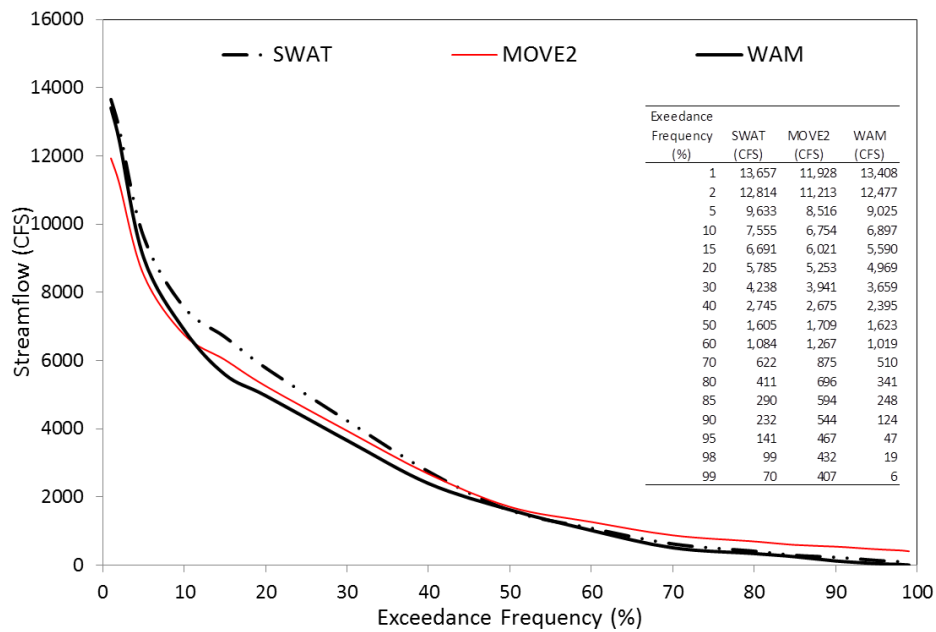


Figure 4.65 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at NERO in the Neches River Basin

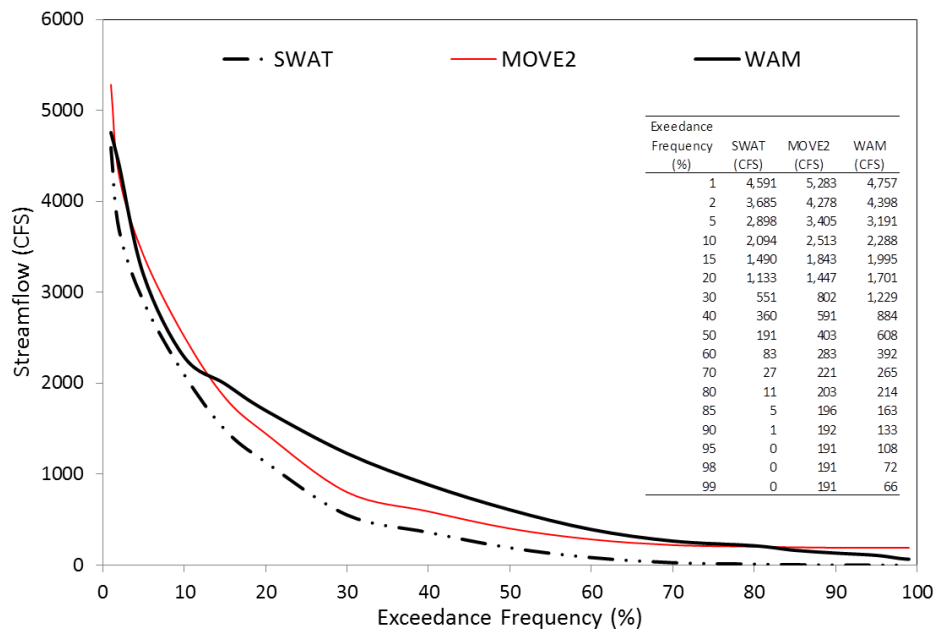


Figure 4.66 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at VIKO in the Neches River Basin

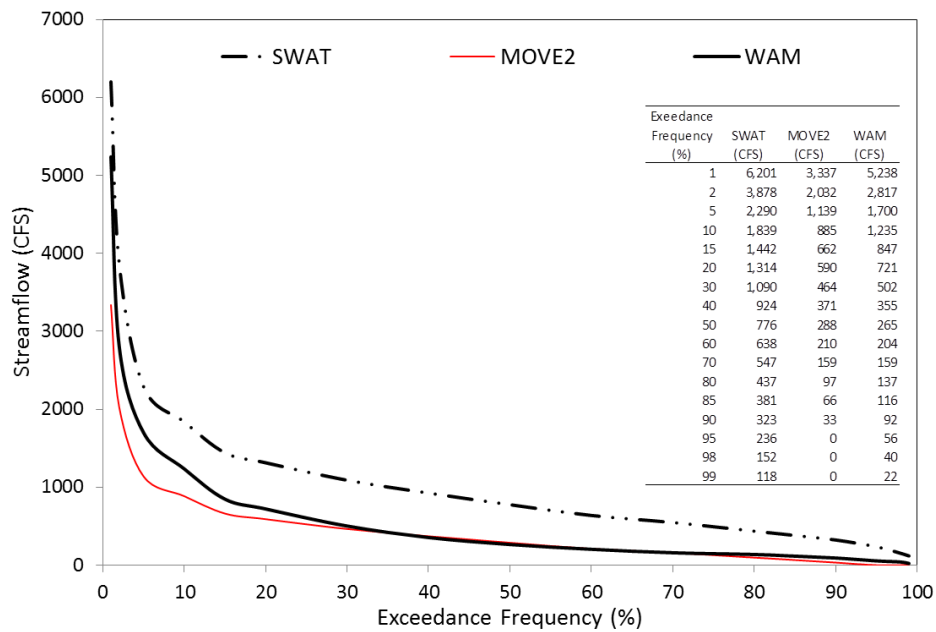


Figure 4.67 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at CP02 in the GSA River Basins

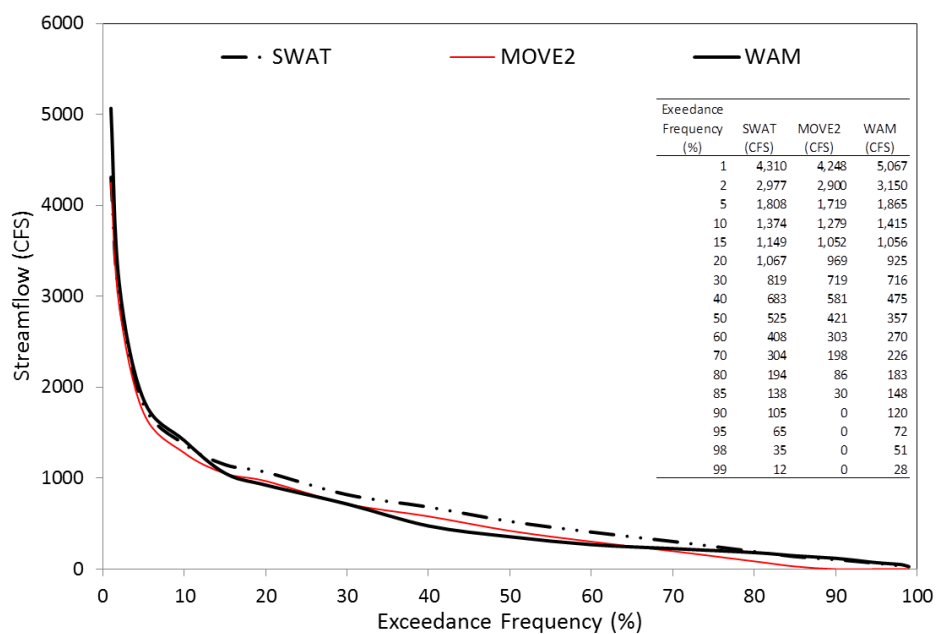


Figure 4.68 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at CP04 in the GSA River Basins

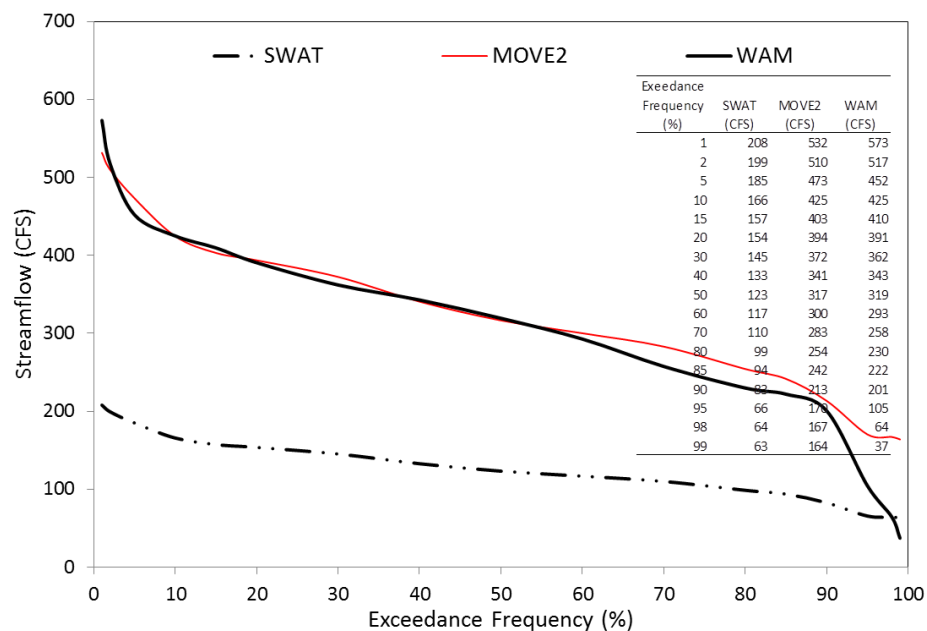


Figure 4.69 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at CP05 in the GSA River Basins

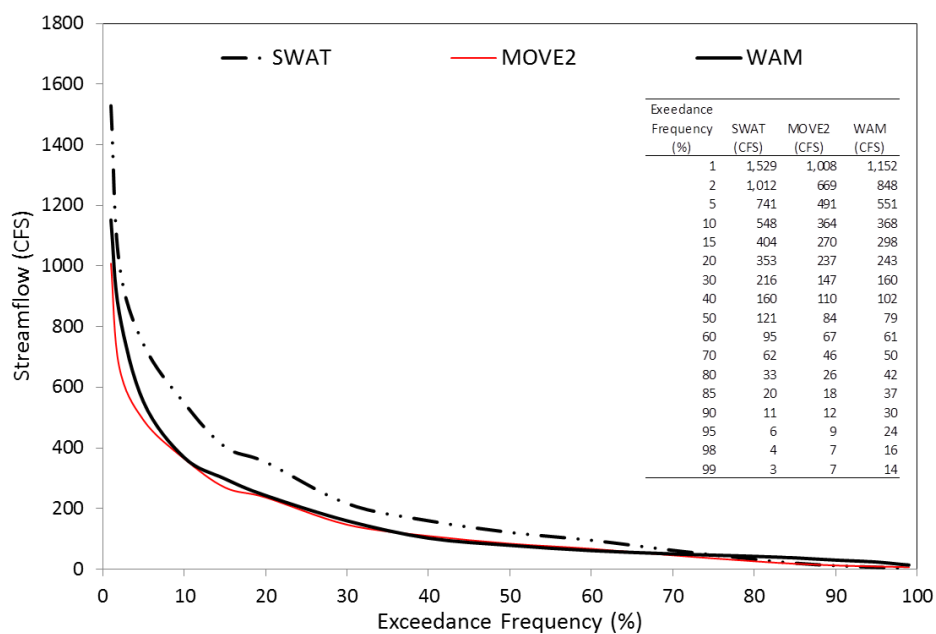


Figure 4.70 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at CP08 in the GSA River Basins

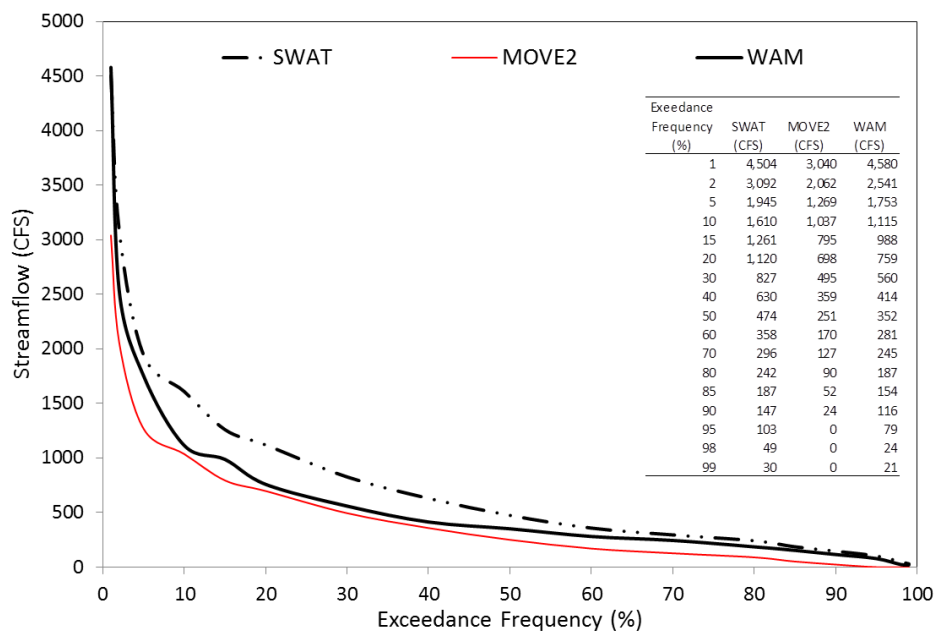


Figure 4.71 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at CP32 in the GSA River Basins

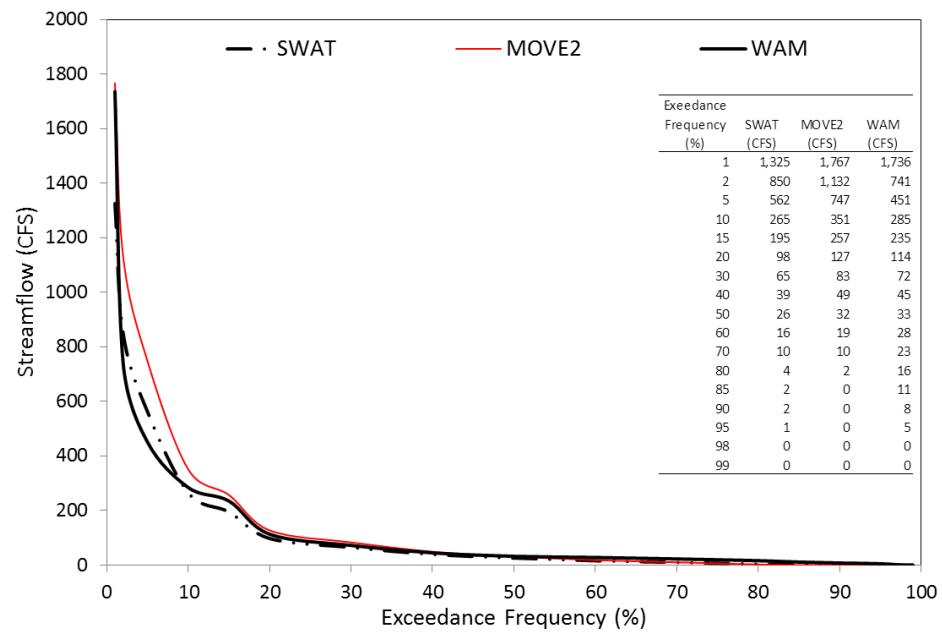


Figure 4.72 Flow Duration Curves (SWAT: SWAT flow, MOVE2: Infilled flow by MOVE2, and WAM: Naturalized flow) and Flow Frequencies at CP35 in the GSA River Basins

CHAPTER V
REFINEMENT OF METHODS FOR DEVELOPING NATURALIZED FLOWS
AT UNGAUGED SITES BASED ON FLOWS AT GAUGED SITES

5.1 Naturalized Flow Datasets

The WAM datasets of the three case study basins serve as sample data pairs in this section. In addition, the WAM dataset in the Trinity River Basin also serves as the pairs to evaluate the improvement of these suggested methods in this research. The characteristic and WAM datasets of the Trinity River Basin are referred to in *Daily Water Availability Model for the Trinity River Basin* (Hoffpauir *et al.*, 2014). The periods-of-analysis of the naturalized flows at each primary control point are 1940-1998 for the Sabine WAM, 1940-1996 for the Neches WAM, 1934-1989 for the GSA WAM, and 1940-1998 for the Trinity WAM, respectively.

In the pair of control points, one control point is assumed an ungauged site Y, and another control point is assumed a gauged site X. Its opposite pair is also available in pair sampling from WAM dataset. The pairs with high linear correlation coefficient ($r > 0.91$) are only obtained from the WAM datasets for removing biases caused by low linear correlation of both datasets when a linear transfer method is used for developing flow sequences at ungauged based on flows at gauged sites.

The WAM dataset of the Sabine River Basin (the Sabine WAM) has the monthly naturalized flows at 18 primary control points. There are 153 possible unordered pair samplings from 18 primary control points. The unordered 26 pairs have more than 0.91 correlation coefficients (r) as listed in Table 5.1. There are monthly naturalized flows at 20 primary control points in the WAM dataset of the Neches River Basin (the Neches WAM). These control points have 190 possible unordered pair samplings. The 79 unordered pairs with acceptable correlation coefficient like the Sabine River Basin are chosen, as listed in Table 5.2. The WAM datasets of the GSA River Basins (the GSA WAM) have 45 primary control points, and 990 unordered pair samplings are possible, but 58 pairs have acceptable correlation coefficients as listed in Table 5.3. The WAM

dataset of the Trinity River Basin (the Trinity WAM) has 40 primary control points. 708 unordered pair samplings are available, but 194 pairs have more than 0.9 correlation coefficients as listed in Table 5.4.

Table 5.1 Selected Pairs from the WAM dataset in the Sabine River Basin

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)
1	SRWP	SRMN	0.98
2	SRMN	SRWP	0.98
3	SRMN	LFQT	0.92
4	SRMN	SRGW	0.94
5	LFQT	SRMN	0.92
6	SRGW	SRMN	0.94
7	SRGW	SRBE	0.96
8	SRGW	SRLP	0.93
9	SRBE	SRGW	0.96
10	SRBE	SRLP	0.98
11	MCTT	MBGR	0.98
12	MBGR	MCTT	0.98
13	SRLP	SRGW	0.93
14	SRLP	SRBE	0.98
15	SRLP	SRBU	0.91
16	SRBU	SRLP	0.91
17	SRBU	SRRL	0.98
18	SRBU	SRLS	0.98
19	SRBW	SRRL	0.99
20	SRBW	SRLS	0.98
21	SRRL	SRBU	0.98
22	SRRL	SRBW	0.99
23	SRRL	SRLS	1.00
24	SRLS	SRBU	0.98
25	SRLS	SRBW	0.98
26	SRLS	SRRL	1.00

Table 5.2 Selected Pairs from the WAM dataset in the Neches River Basin

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)
1	KIBR	NEPA	0.97	28	ANAL	ANSR	0.93
2	KIBR	NENE	0.96	29	ANAL	NETB	0.92
3	NEPA	KIBR	0.97	30	ANAL	NEEV	0.91
4	NEPA	NENE	0.98	31	ANLU	NEAL	0.92
5	NEPA	NEAL	0.92	32	ANLU	NEDI	0.94
6	NENE	KIBR	0.96	33	ANLU	MUJA	0.93
7	NENE	NEPA	0.98	34	ANLU	EFACU	0.94
8	NENE	NEAL	0.95	35	ANLU	ANAL	0.99
9	NEAL	NEPA	0.92	36	ANLU	ANSR	0.95
10	NEAL	NENE	0.95	37	ANLU	NETB	0.94
11	NEAL	NEDI	0.97	38	ANLU	NEEV	0.93
12	NEDI	NEAL	0.97	39	ATCH	AYSA	0.92
13	NEDI	NERO	0.98	40	ATCH	ANSR	0.93
14	NEDI	NEBA	0.92	41	AYSA	ATCH	0.92
15	NEDI	NESL	0.92	42	AYSA	ANSR	0.93
16	NERO	NEDI	0.98	43	ANSR	NEDI	0.92
17	NERO	ANSR	0.94	44	ANSR	NERO	0.94
18	NERO	NETB	0.98	45	ANSR	ANAL	0.93
19	NERO	NEEV	0.98	46	ANSR	ANLU	0.95
20	NERO	NEBA	0.96	47	ANSR	ATCH	0.93
21	NERO	NESL	0.96	48	ANSR	AYSA	0.93
22	MUJA	ANAL	0.94	49	ANSR	NETB	0.98
23	MUJA	ANLU	0.93	50	ANSR	NEEV	0.98
24	EFACU	ANAL	0.93	51	ANSR	NEBA	0.96
25	EFACU	ANLU	0.94	52	ANSR	NESL	0.95
26	ANAL	MUJA	0.94	53	NETB	NEDI	0.95
27	ANAL	ANLU	0.99	54	NETB	NERO	0.98

Table 5.2 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)
55	NETB	ANAL	0.92
56	NETB	ANLU	0.94
57	NETB	ANSR	0.98
58	NETB	NEEV	1.00
59	NETB	NEBA	0.98
60	NETB	NESL	0.98
61	NEEV	NEDI	0.95
62	NEEV	NERO	0.98
63	NEEV	ANAL	0.91
64	NEEV	ANLU	0.93
65	NEEV	ANSR	0.98
66	NEEV	NEBA	0.99
67	NEEV	NESL	0.98
68	NEBA	NEDI	0.92
69	NEBA	NERO	0.96
70	NEBA	ANSR	0.96
71	NEBA	NETB	0.98
72	NEBA	NEEV	0.99
73	NEBA	NESL	1.00
74	NESL	NEDI	0.92
75	NESL	NERO	0.96
76	NESL	ANSR	0.95
77	NESL	NETB	0.98
78	NESL	NEEV	0.98
79	NESL	NEBA	1.00

Table 5.3 Selected Pairs from the WAM dataset in the GSA River Basins

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)
1	CP02	CP01	0.91	33	CP263	CP25	0.99
2	CP01	CP02	0.91	34	CP25	CP261	0.99
3	CP03	CP02	0.99	35	CP25	CP262	0.99
4	CP04	CP02	0.98	36	CP21	CP27	0.94
5	CP06	CP02	0.94	37	CP23	CP27	0.94
6	CP02	CP03	0.99	38	CP28	CP27	0.99
7	CP04	CP03	0.99	39	CP29	CP27	0.94
8	CP06	CP03	0.96	40	CP32	CP27	0.92
9	CP02	CP04	0.98	41	CP21	CP28	0.92
10	CP03	CP04	0.99	42	CP23	CP28	0.92
11	CP06	CP04	0.98	43	CP27	CP28	0.99
12	CP02	CP06	0.94	44	CP29	CP28	0.95
13	CP03	CP06	0.96	45	CP32	CP28	0.94
14	CP04	CP06	0.98	46	CP27	CP29	0.94
15	CP09	CP08	0.99	47	CP28	CP29	0.95
16	CP10	CP08	0.91	48	CP32	CP29	0.98
17	CP08	CP09	0.99	49	CP27	CP32	0.92
18	CP10	CP09	0.92	50	CP28	CP32	0.94
19	CP08	CP10	0.91	51	CP29	CP32	0.98
20	CP09	CP10	0.92	52	CP38	CP37	0.93
21	CP15	CP14	1.00	53	CPEST	CP37	0.92
22	CP38	CP14	0.95	54	CP14	CP38	0.95
23	CPEST	CP14	0.94	55	CP15	CP38	0.95
24	CP14	CP15	1.00	56	CP37	CP38	0.93
25	CP38	CP15	0.95	57	CP14	CPEST	0.94
26	CPEST	CP15	0.94	58	CP15	CPEST	0.94
27	CP27	CP21	0.94				
28	CP28	CP21	0.92				
29	CP27	CP23	0.94				
30	CP28	CP23	0.92				
31	CP261	CP25	0.99				
32	CP262	CP25	0.99				

Table 5.4 Selected Pairs from the WAM dataset in the Trinity River Basin

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)
1	8WTJA	8WTBO	0.93	35	8ELSA	8TRDA	0.91
2	8BSBR	8WTBO	0.93	36	8IDPP	8ELSA	0.92
3	8WTBO	8WTJA	0.93	37	8IDPP	8CLSA	0.92
4	8WTBO	8BSBR	0.93	38	8IDPP	8ELLE	0.98
5	8WTBO	8WTFW	0.97	39	8IDPP	8DNGR	0.93
6	8WTBO	8WTGP	0.95	40	8IDPP	8TRDA	0.95
7	8WTBO	8DNJU	0.94	41	8IDPP	8TRRS	0.93
8	8WTBO	8DNGR	0.92	42	8CLSA	8ELSA	0.98
9	8WTBO	8TRDA	0.92	43	8CLSA	8IDPP	0.92
10	8CTAL	8CTBE	0.97	44	8CLSA	8ELLE	0.94
11	8CTAL	8CTFW	0.97	45	8CLSA	8DNJU	0.97
12	8CTBE	8CTAL	0.97	46	8CLSA	8DNGR	0.95
13	8CTBE	8CTFW	0.99	47	8CLSA	8TRDA	0.93
14	8CTFW	8CTAL	0.97	48	8ELLE	8ELSA	0.92
15	8CTFW	8CTBE	0.99	49	8ELLE	8IDPP	0.98
16	8WTFW	8WTBO	0.97	50	8ELLE	8CLSA	0.94
17	8WTFW	8WTGP	0.99	51	8ELLE	8DNJU	0.94
18	8WTFW	8DNJU	0.94	52	8ELLE	8DNGR	0.96
19	8WTFW	8DNGR	0.94	53	8ELLE	8TRDA	0.97
20	8WTFW	8TRDA	0.96	54	8ELLE	8ETMK	0.91
21	8WTFW	8TRRS	0.93	55	8ELLE	8TRRS	0.95
22	8WTFW	8TRTR	0.92	56	8ELLE	8TRTR	0.93
23	8WTGP	8WTBO	0.95	57	8DNJU	8WTBO	0.94
24	8WTGP	8WTFW	0.99	58	8DNJU	8WTFW	0.94
25	8WTGP	8DNJU	0.92	59	8DNJU	8WTGP	0.92
26	8WTGP	8DNGR	0.93	60	8DNJU	8ELSA	0.94
27	8WTGP	8TRDA	0.97	61	8DNJU	8CLSA	0.97
28	8WTGP	8TRRS	0.95	62	8DNJU	8ELLE	0.94
29	8WTGP	8TRTR	0.94	63	8DNJU	8DNGR	0.98
30	8ELSA	8IDPP	0.92	64	8DNJU	8TRDA	0.95
31	8ELSA	8CLSA	0.98	65	8DNGR	8WTBO	0.92
32	8ELSA	8ELLE	0.92	66	8DNGR	8WTFW	0.94
33	8ELSA	8DNJU	0.94	67	8DNGR	8WTGP	0.93
34	8ELSA	8DNGR	0.93	68	8DNGR	8ELSA	0.93

Table 5.4 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)
69	8DNGR	8IDPP	0.93	103	8ETLA	8SGPR	0.96
70	8DNGR	8CLSA	0.95	104	8ETLA	8ETFO	0.98
71	8DNGR	8ELLE	0.96	105	8ETLA	8ETCR	0.98
72	8DNGR	8DNJU	0.98	106	8ETLA	8TRRS	0.92
73	8DNGR	8TRDA	0.97	107	8ETLA	8TRTR	0.92
74	8DNGR	8TRRS	0.93	108	8ETFO	8ETMK	0.96
75	8DNGR	8TRTR	0.92	109	8ETFO	8SGPR	0.96
76	8TRDA	8WTBO	0.92	110	8ETFO	8ETLA	0.98
77	8TRDA	8WTFW	0.96	111	8ETFO	8ETCR	1.00
78	8TRDA	8WTGP	0.97	112	8ETFO	8TRRS	0.93
79	8TRDA	8ELSA	0.91	113	8ETFO	8TRTR	0.93
80	8TRDA	8IDPP	0.95	114	8ETCR	8ETMK	0.96
81	8TRDA	8CLSA	0.93	115	8ETCR	8SGPR	0.96
82	8TRDA	8ELLE	0.97	116	8ETCR	8ETLA	0.98
83	8TRDA	8DNJU	0.95	117	8ETCR	8ETFO	1.00
84	8TRDA	8DNGR	0.97	118	8ETCR	8TRRS	0.93
85	8TRDA	8ETMK	0.91	119	8ETCR	8TRTR	0.93
86	8TRDA	8TRRS	0.98	120	8TRRS	8WTFW	0.93
87	8TRDA	8TRTR	0.97	121	8TRRS	8WTGP	0.95
88	8ETMK	8ELLE	0.91	122	8TRRS	8IDPP	0.93
89	8ETMK	8TRDA	0.91	123	8TRRS	8ELLE	0.95
90	8ETMK	8SGPR	0.98	124	8TRRS	8DNGR	0.93
91	8ETMK	8ETLA	0.97	125	8TRRS	8TRDA	0.98
92	8ETMK	8ETFO	0.96	126	8TRRS	8ETMK	0.94
93	8ETMK	8ETCR	0.96	127	8TRRS	8SGPR	0.93
94	8ETMK	8TRRS	0.94	128	8TRRS	8ETLA	0.92
95	8ETMK	8TRTR	0.94	129	8TRRS	8ETFO	0.93
96	8SGPR	8ETMK	0.98	130	8TRRS	8ETCR	0.93
97	8SGPR	8ETLA	0.96	131	8TRRS	8TRTR	0.99
98	8SGPR	8ETFO	0.96	132	8TRRS	8TROA	0.93
99	8SGPR	8ETCR	0.96	133	8TRTR	8WTFW	0.92
100	8SGPR	8TRRS	0.93	134	8TRTR	8WTGP	0.94
101	8SGPR	8TRTR	0.93	135	8TRTR	8ELLE	0.93
102	8ETLA	8ETMK	0.97	136	8TRTR	8DNGR	0.92

Table 5.4 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)
137	8TRTR	8TRDA	0.97	171	8TRCR	8TRMI	1.00
138	8TRTR	8ETMK	0.94	172	8TRCR	8TRRI	0.99
139	8TRTR	8SGPR	0.93	173	8TRCR	8TRRO	0.97
140	8TRTR	8ETLA	0.92	174	8TRCR	8TRGB	0.96
141	8TRTR	8ETFO	0.93	175	8TRMI	8TROA	0.99
142	8TRTR	8ETCR	0.93	176	8TRMI	8TRCR	1.00
143	8TRTR	8TRRS	0.99	177	8TRMI	8TRRI	0.99
144	8TRTR	8TROA	0.94	178	8TRMI	8TRRO	0.97
145	8CEKE	8KGKA	0.95	179	8TRMI	8TRGB	0.96
146	8CEKE	8CEMA	0.98	180	8TRRI	8TROA	0.97
147	8KGKA	8CEKE	0.95	181	8TRRI	8TRCR	0.99
148	8KGKA	8CEMA	0.99	182	8TRRI	8TRMI	0.99
149	8CEMA	8CEKE	0.98	183	8TRRI	8TRRO	0.99
150	8CEMA	8KGKA	0.99	184	8TRRI	8TRGB	0.98
151	8RIDA	8RIRI	0.98	185	8TRRO	8TROA	0.94
152	8RIDA	8RIFA	0.95	186	8TRRO	8TRCR	0.97
153	8RIRI	8RIDA	0.98	187	8TRRO	8TRMI	0.97
154	8RIRI	8RIFA	0.97	188	8TRRO	8TRRI	0.99
155	8WABA	8CHCO	0.98	189	8TRRO	8TRGB	1.00
156	8WABA	8RIFA	0.94	190	8TRGB	8TROA	0.93
157	8CHCO	8WABA	0.98	191	8TRGB	8TRCR	0.96
158	8CHCO	8RIFA	0.96	192	8TRGB	8TRMI	0.96
159	8RIFA	8RIDA	0.95	193	8TRGB	8TRRI	0.98
160	8RIFA	8RIRI	0.97	194	8TRGB	8TRRO	1.00
161	8RIFA	8WABA	0.94				
162	8RIFA	8CHCO	0.96				
163	8TROA	8TRRS	0.93				
164	8TROA	8TRTR	0.94				
165	8TROA	8TRCR	0.99				
166	8TROA	8TRMI	0.99				
167	8TROA	8TRRI	0.97				
168	8TROA	8TRRO	0.94				
169	8TROA	8TRGB	0.93				
170	8TRCR	8TROA	0.99				

5.2 Drainage Area Ratio Method

5.2.1 Optimization of Correction Factors and Exponents

The linear regression equations in the form of Equation 2.10 are derived from 358 pairs in the four river basins. These linear regression equations can be considered the best linear relationships, obtained from the relationship between two time series such as flow sequences. Thus, the goal of general linear transfer methods is to find more similar values like B in the Equation 2.10. It is assumed that a slope in the regression equation is equal to the drainage area ratio with correction factor and exponent like Equation 2.11. The representative correction factors K and exponents ϕ for the four river basins are developed by the GRG optimization algorithm with the objective function like the equation 2.12 based on the equation 2.11 using Microsoft Excel. Table 5.5 lists the slopes (B) and coefficients of determination (R^2) of the regression equations, derived from linear relationships of each pair. Table 5.6 lists the optimized correction factors (K), and exponents (ϕ) of the equation 2.12 for the four river basins.

5.2.2 Comparative Evaluation of the Method Performances

Monthly naturalized flow sequences at gauged sites treated as target ungauged sites are developed based on naturalized flows at other gauged sites treated as source sites for each period-of-analysis in the four WAMs by both methods, DAR method without correction factor and exponent (“the conventional method”) and DAR method with correction factor and exponent made by the suggested method in this research (“the suggested method”). The performance of the suggested method is evaluated with NSE between the naturalized flow and synthesized flow. The method performance is also compared with the performance of the conventional method to assess how much or how well the suggested method enhances the performance than the conventional method. Most of all, this section focuses on the performance of the suggested method when a pair DAR is less than 0.3 or more than 1.5.

Table 5.5 Slopes and Coefficients of Determination (R^2) of Linear Regression Equations from the WAM dataset in the River Basins

No.	Assumed Ungaaged Site (Y)	Assumed Gauged Site (X)	Slope (Y=BX)	Coefficient of Deter. (R^2)
Sabine				
1	SRWP	SRMN	0.577	0.95
2	SRMN	SRWP	1.681	0.95
3	SRMN	LFQT	2.071	0.84
4	SRMN	SRGW	0.500	0.89
5	LFQT	SRMN	0.432	0.85
6	SRGW	SRMN	1.851	0.88
7	SRGW	SRBE	0.775	0.93
8	SRGW	SRLP	0.579	0.87
9	SRBE	SRGW	1.232	0.92
10	SRBE	SRLP	0.754	0.96
11	MCTT	MBGR	1.130	0.97
12	MBGR	MCTT	0.867	0.97
13	SRLP	SRGW	1.582	0.85
14	SRLP	SRBE	1.297	0.96
15	SRLP	SRBU	0.582	0.83
16	SRBU	SRLP	1.547	0.82
17	SRBU	SRRL	0.748	0.95
18	SRBU	SRSL	0.708	0.95
19	SRBW	SRRL	0.850	0.97
20	SRBW	SRSL	0.805	0.97
21	SRRL	SRBU	1.303	0.95
22	SRRL	SRBW	1.160	0.97
23	SRRL	SRSL	0.947	1.00
24	SRSL	SRBU	1.371	0.94
25	SRSL	SRBW	1.222	0.97
26	SRSL	SRRL	1.054	1.00
Neches				
27	KIBR	NEPA	0.307	0.93
28	KIBR	NENE	0.208	0.93
29	NEPA	KIBR	3.119	0.93
30	NEPA	NENE	0.672	0.97

Table 5.5 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Slope (Y=BX)	Coefficient of Deter. (R ²)
31	NEPA	NEAL	0.419	0.86
32	NENE	KIBR	4.582	0.93
33	NENE	NEPA	1.458	0.97
34	NENE	NEAL	0.628	0.91
35	NEAL	NEPA	2.174	0.86
36	NEAL	NENE	1.505	0.91
37	NEAL	NEDI	0.691	0.94
38	NEDI	NEAL	1.398	0.94
39	NEDI	NERO	0.680	0.96
40	NEDI	NEBA	0.205	0.85
41	NEDI	NESL	0.199	0.84
42	NERO	NEDI	1.438	0.96
43	NERO	ANSR	0.863	0.88
44	NERO	NETB	0.435	0.96
45	NERO	NEEV	0.403	0.95
46	NERO	NEBA	0.305	0.92
47	NERO	NESL	0.295	0.91
48	MUJA	ANAL	0.292	0.89
49	MUJA	ANLU	0.223	0.86
50	EFACU	ANAL	0.125	0.87
51	EFACU	ANLU	0.097	0.88
52	ANAL	MUJA	3.196	0.89
53	ANAL	ANLU	0.767	0.99
54	ANAL	ANSR	0.323	0.86
55	ANAL	NETB	0.158	0.84
56	ANAL	NEEV	0.146	0.83
57	ANLU	NEAL	0.966	0.85
58	ANLU	NEDI	0.687	0.88
59	ANLU	MUJA	4.114	0.86
60	ANLU	EFACU	9.620	0.88
61	ANLU	ANAL	1.295	0.99
62	ANLU	ANSR	0.425	0.90
63	ANLU	NETB	0.208	0.88

Table 5.5 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Slope (Y=BX)	Coefficient of Deter. (R ²)
64	ANLU	NEEV	0.192	0.86
65	ATCH	AYSA	4.192	0.85
66	ATCH	ANSR	0.160	0.87
67	AYSA	ATCH	0.216	0.85
68	AYSA	ANSR	0.036	0.87
69	ANSR	NEDI	1.550	0.85
70	ANSR	NERO	1.077	0.88
71	ANSR	ANAL	2.837	0.86
72	ANSR	ANLU	2.211	0.90
73	ANSR	ATCH	5.761	0.87
74	ANSR	AYSA	25.317	0.87
75	ANSR	NETB	0.488	0.97
76	ANSR	NEEV	0.451	0.96
77	ANSR	NEBA	0.340	0.92
78	ANSR	NESL	0.329	0.91
79	NETB	NEDI	3.211	0.91
80	NETB	NERO	2.238	0.96
81	NETB	ANAL	5.738	0.84
82	NETB	ANLU	4.469	0.88
83	NETB	ANSR	2.012	0.97
84	NETB	NEEV	0.927	0.99
85	NETB	NEBA	0.701	0.96
86	NETB	NESL	0.679	0.96
87	NEEV	NEDI	3.448	0.90
88	NEEV	NERO	2.407	0.95
89	NEEV	ANAL	6.145	0.83
90	NEEV	ANLU	4.787	0.86
91	NEEV	ANSR	2.157	0.96
92	NEEV	NEBA	0.758	0.97
93	NEEV	NESL	0.734	0.97
94	NEBA	NEDI	4.442	0.85
95	NEBA	NERO	3.117	0.92
96	NEBA	ANSR	2.788	0.92
97	NEBA	NETB	1.394	0.96

Table 5.5 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Slope (Y=BX)	Coefficient of Deter. (R ²)
98	NEBA	NEEV	1.299	0.97
99	NEBA	NESL	0.969	1.00
100	NESL	NEDI	4.573	0.84
101	NESL	NERO	3.211	0.91
102	NESL	ANSR	2.871	0.91
103	NESL	NETB	1.436	0.96
104	NESL	NEEV	1.338	0.97
105	NESL	NEBA	1.032	1.00
GSA				
106	CP02	CP01	1.490	0.82
107	CP01	CP02	0.583	0.82
108	CP03	CP02	1.033	0.99
109	CP04	CP02	1.181	0.96
110	CP06	CP02	1.363	0.89
111	CP02	CP03	0.958	0.99
112	CP04	CP03	1.147	0.98
113	CP06	CP03	1.343	0.92
114	CP02	CP04	0.823	0.96
115	CP03	CP04	0.863	0.98
116	CP06	CP04	1.182	0.95
117	CP02	CP06	0.623	0.89
118	CP03	CP06	0.663	0.92
119	CP04	CP06	0.775	0.95
120	CP09	CP08	1.042	0.98
121	CP10	CP08	2.236	0.83
122	CP08	CP09	0.945	0.98
123	CP10	CP09	2.123	0.86
124	CP08	CP10	0.393	0.83
125	CP09	CP10	0.412	0.86
126	CP15	CP14	1.044	0.99
127	CP38	CP14	1.597	0.89
128	CPEST	CP14	1.645	0.88
129	CP14	CP15	0.955	0.99

Table 5.5 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Slope (Y=BX)	Coefficient of Deter. (R ²)
130	CP38	CP15	1.534	0.91
131	CPEST	CP15	1.581	0.89
132	CP27	CP21	1.118	0.89
133	CP28	CP21	1.198	0.84
134	CP27	CP23	1.182	0.89
135	CP28	CP23	1.265	0.84
136	CP261	CP25	0.907	0.98
137	CP262	CP25	0.423	0.98
138	CP263	CP25	0.176	0.98
139	CP25	CP261	1.087	0.98
140	CP25	CP262	2.333	0.98
141	CP21	CP27	0.818	0.89
142	CP23	CP27	0.772	0.89
143	CP28	CP27	1.084	0.98
144	CP29	CP27	1.439	0.88
145	CP32	CP27	1.583	0.84
146	CP21	CP28	0.735	0.84
147	CP23	CP28	0.693	0.84
148	CP27	CP28	0.909	0.98
149	CP29	CP28	1.337	0.91
150	CP32	CP28	1.475	0.88
151	CP27	CP29	0.630	0.88
152	CP28	CP29	0.697	0.91
153	CP32	CP29	1.105	0.97
154	CP27	CP32	0.556	0.84
155	CP28	CP32	0.617	0.88
156	CP29	CP32	0.886	0.97
157	CP38	CP37	3.302	0.86
158	CPEST	CP37	3.404	0.85
159	CP14	CP38	0.585	0.89
160	CP15	CP38	0.615	0.91
161	CP37	CP38	0.271	0.86
162	CP14	CPEST	0.563	0.88

Table 5.5 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Slope (Y=BX)	Coefficient of Deter. (R ²)
162	CP14	CPEST	0.563	0.88
163	CP15	CPEST	0.591	0.89
Trinity				
164	8WTJA	8WTBO	0.353	0.86
165	8BSBR	8WTBO	0.243	0.87
166	8WTBO	8WTJA	2.468	0.86
167	8WTBO	8BSBR	3.664	0.87
168	8WTBO	8WTFW	0.568	0.94
169	8WTBO	8WTGP	0.459	0.91
170	8WTBO	8DNJU	2.854	0.88
171	8WTBO	8DNGR	1.476	0.85
172	8WTBO	8TRDA	0.171	0.85
173	8CTAL	8CTBE	0.470	0.95
174	8CTAL	8CTFW	0.354	0.94
175	8CTBE	8CTAL	2.041	0.95
176	8CTBE	8CTFW	0.754	0.99
177	8CTFW	8CTAL	2.684	0.94
178	8CTFW	8CTBE	1.131	0.99
179	8WTFW	8WTBO	1.666	0.94
180	8WTFW	8WTGP	0.813	0.98
181	8WTFW	8DNJU	4.871	0.88
182	8WTFW	8DNGR	2.557	0.88
183	8WTFW	8TRDA	0.303	0.92
184	8WTFW	8TRRS	0.208	0.86
185	8WTFW	8TRTR	0.195	0.85
186	8WTGP	8WTBO	2.001	0.91
187	8WTGP	8WTFW	1.209	0.98
188	8WTGP	8DNJU	5.869	0.85
189	8WTGP	8DNGR	3.101	0.87
190	8WTGP	8TRDA	0.373	0.94
191	8WTGP	8TRRS	0.258	0.90
192	8WTGP	8TRTR	0.242	0.88
193	8ELSA	8IDPP	1.316	0.84

Table 5.5 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Slope (Y=BX)	Coefficient of Deter. (R ²)
194	8ELSA	8CLSA	1.791	0.96
195	8ELSA	8ELLE	0.235	0.85
196	8ELSA	8DNJU	1.542	0.89
197	8ELSA	8DNGR	0.804	0.87
198	8ELSA	8TRDA	0.091	0.83
199	8IDPP	8ELSA	0.656	0.84
200	8IDPP	8CLSA	1.198	0.84
201	8IDPP	8ELLE	0.175	0.97
202	8IDPP	8DNGR	0.565	0.87
203	8IDPP	8TRDA	0.067	0.90
204	8IDPP	8TRRS	0.046	0.86
205	8CLSA	8ELSA	0.540	0.96
206	8CLSA	8IDPP	0.724	0.84
207	8CLSA	8ELLE	0.131	0.88
208	8CLSA	8DNJU	0.865	0.94
209	8CLSA	8DNGR	0.449	0.91
210	8CLSA	8TRDA	0.051	0.87
211	8ELLE	8ELSA	3.702	0.85
212	8ELLE	8IDPP	5.543	0.97
213	8ELLE	8CLSA	6.829	0.88
214	8ELLE	8DNJU	6.061	0.88
215	8ELLE	8DNGR	3.235	0.91
216	8ELLE	8TRDA	0.381	0.94
217	8ELLE	8ETMK	6.052	0.83
218	8ELLE	8TRRS	0.265	0.90
219	8ELLE	8TRTR	0.247	0.87
220	8DNJU	8WTBO	0.315	0.88
221	8DNJU	8WTFW	0.183	0.88
222	8DNJU	8WTGP	0.149	0.85
223	8DNJU	8ELSA	0.587	0.89
224	8DNJU	8CLSA	1.094	0.94
225	8DNJU	8ELLE	0.147	0.88
226	8DNJU	8DNGR	0.515	0.96

Table 5.5 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Slope (Y=BX)	Coefficient of Deter. (R ²)
227	8DNJU	8TRDA	0.058	0.90
228	8DNGR	8WTBO	0.592	0.85
229	8DNGR	8WTFW	0.350	0.88
230	8DNGR	8WTGP	0.285	0.87
231	8DNGR	8ELSA	1.112	0.87
232	8DNGR	8IDPP	1.567	0.87
233	8DNGR	8CLSA	2.061	0.91
234	8DNGR	8ELLE	0.284	0.91
235	8DNGR	8DNJU	1.871	0.96
236	8DNGR	8TRDA	0.112	0.93
237	8DNGR	8TRRS	0.077	0.87
238	8DNGR	8TRTR	0.072	0.84
239	8TRDA	8WTBO	5.101	0.85
240	8TRDA	8WTFW	3.081	0.92
241	8TRDA	8WTGP	2.549	0.94
242	8TRDA	8ELSA	9.399	0.83
243	8TRDA	8IDPP	13.769	0.90
244	8TRDA	8CLSA	17.462	0.87
245	8TRDA	8ELLE	2.487	0.94
246	8TRDA	8DNJU	15.683	0.90
247	8TRDA	8DNGR	8.355	0.93
248	8TRDA	8ETMK	15.457	0.83
249	8TRDA	8TRRS	0.696	0.97
250	8TRDA	8TRTR	0.651	0.94
251	8ETMK	8ELLE	0.144	0.83
252	8ETMK	8TRDA	0.056	0.83
253	8ETMK	8SGPR	1.686	0.96
254	8ETMK	8ETLA	0.265	0.94
255	8ETMK	8ETFO	0.175	0.93
256	8ETMK	8ETCR	0.159	0.92
257	8ETMK	8TRRS	0.041	0.88
258	8ETMK	8TRTR	0.038	0.87
259	8SGPR	8ETMK	0.577	0.96

Table 5.5 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Slope (Y=BX)	Coefficient of Deter. (R ²)
260	8SGPR	8ETLA	0.155	0.93
261	8SGPR	8ETFO	0.102	0.93
262	8SGPR	8ETCR	0.093	0.92
263	8SGPR	8TRRS	0.024	0.87
264	8SGPR	8TRTR	0.022	0.86
265	8ETLA	8ETMK	3.592	0.94
266	8ETLA	8SGPR	6.120	0.93
267	8ETLA	8ETFO	0.653	0.97
268	8ETLA	8ETCR	0.596	0.96
269	8ETLA	8TRRS	0.148	0.85
270	8ETLA	8TRTR	0.139	0.85
271	8ETFO	8ETMK	5.402	0.93
272	8ETFO	8SGPR	9.246	0.93
273	8ETFO	8ETLA	1.492	0.97
274	8ETFO	8ETCR	0.914	0.99
275	8ETFO	8TRRS	0.225	0.87
276	8ETFO	8TRTR	0.213	0.87
277	8ETCR	8ETMK	5.874	0.92
278	8ETCR	8SGPR	10.066	0.92
279	8ETCR	8ETLA	1.622	0.96
280	8ETCR	8ETFO	1.088	0.99
281	8ETCR	8TRRS	0.245	0.87
282	8ETCR	8TRTR	0.233	0.87
283	8TRRS	8WTFW	4.240	0.86
284	8TRRS	8WTGP	3.533	0.90
285	8TRRS	8IDPP	19.118	0.86
286	8TRRS	8ELLE	3.460	0.90
287	8TRRS	8DNGR	11.436	0.87
288	8TRRS	8TRDA	1.395	0.97
289	8TRRS	8ETMK	22.391	0.88
290	8TRRS	8SGPR	38.035	0.87
291	8TRRS	8ETLA	6.007	0.85
292	8TRRS	8ETFO	3.996	0.87

Table 5.5 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Slope (Y=BX)	Coefficient of Deter. (R ²)
293	8TRRS	8ETCR	3.663	0.87
294	8TRRS	8TRTR	0.940	0.99
295	8TRRS	8TROA	0.611	0.86
296	8TRTR	8WTFW	4.447	0.85
297	8TRTR	8WTGP	3.716	0.88
298	8TRTR	8ELLE	3.609	0.87
299	8TRTR	8DNGR	11.949	0.84
300	8TRTR	8TRDA	1.460	0.94
301	8TRTR	8ETMK	23.593	0.87
302	8TRTR	8SGPR	40.149	0.86
303	8TRTR	8ETLA	6.341	0.85
304	8TRTR	8ETFO	4.233	0.87
305	8TRTR	8ETCR	3.884	0.87
306	8TRTR	8TRRS	1.052	0.99
307	8TRTR	8TROA	0.655	0.89
308	8CEKE	8KGKA	0.696	0.90
309	8CEKE	8CEMA	0.301	0.96
310	8KGKA	8CEKE	1.332	0.90
311	8KGKA	8CEMA	0.418	0.97
312	8CEMA	8CEKE	3.228	0.96
313	8CEMA	8KGKA	2.343	0.97
314	8RIDA	8RIRI	0.447	0.96
315	8RIDA	8RIFA	0.185	0.90
316	8RIRI	8RIDA	2.166	0.96
317	8RIRI	8RIFA	0.411	0.94
318	8WABA	8CHCO	0.209	0.96
319	8WABA	8RIFA	0.095	0.87
320	8CHCO	8WABA	4.647	0.96
321	8CHCO	8RIFA	0.456	0.92
322	8RIFA	8RIDA	4.979	0.90
323	8RIFA	8RIRI	2.290	0.94
324	8RIFA	8WABA	9.572	0.87
325	8RIFA	8CHCO	2.067	0.92

Table 5.5 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Slope (Y=BX)	Coefficient of Deter. (R ²)
326	8TROA	8TRRS	1.468	0.86
327	8TROA	8TRTR	1.407	0.89
328	8TROA	8TRCR	0.916	0.97
329	8TROA	8TRMI	0.876	0.97
330	8TROA	8TRRI	0.804	0.94
331	8TROA	8TRRO	0.726	0.89
332	8TROA	8TRGB	0.668	0.86
333	8TRCR	8TROA	1.068	0.97
334	8TRCR	8TRMI	0.953	1.00
335	8TRCR	8TRRI	0.881	0.99
336	8TRCR	8TRRO	0.799	0.94
337	8TRCR	8TRGB	0.736	0.92
338	8TRMI	8TROA	1.120	0.97
339	8TRMI	8TRCR	1.045	1.00
340	8TRMI	8TRRI	0.923	0.99
341	8TRMI	8TRRO	0.839	0.95
342	8TRMI	8TRGB	0.772	0.92
343	8TRRI	8TROA	1.195	0.94
344	8TRRI	8TRCR	1.124	0.99
345	8TRRI	8TRMI	1.073	0.99
346	8TRRI	8TRRO	0.913	0.97
347	8TRRI	8TRGB	0.843	0.95
348	8TRRO	8TROA	1.273	0.89
349	8TRRO	8TRCR	1.202	0.94
350	8TRRO	8TRMI	1.150	0.95
351	8TRRO	8TRRI	1.076	0.97
352	8TRRO	8TRGB	0.926	0.99
353	8TRGB	8TROA	1.359	0.86
354	8TRGB	8TRCR	1.285	0.92
355	8TRGB	8TRMI	1.228	0.92
356	8TRGB	8TRRI	1.153	0.95
357	8TRGB	8TRRO	1.074	0.99

Table 5.6 Optimized Representative Correction Factors (K) and Exponents (ϕ) for the Four WAMs

Basin	Correction factors (K)	Exponents (ϕ)
Sabine	0.958	0.974
Neches	0.961	0.983
GSA	0.922	0.890
Trinity	0.885	0.902

The selected pairs are 357 pairs in total from the four WAM datasets, and these pairs have a varying range of drainage area ratios. The Sabine WAM has 5 pairs that have DARs, less than 0.3 or more than 1.5, the Neches WAM has 43 pairs, the GSA WAM has 18 pairs, and the Trinity WAM has 113 pairs, respectively. These pairs from WAMs have 179 pairs in total that have less than 0.3 or more than 1.5 of DAR, and these account for 50.1 percent of the total selected pairs.

The evaluation of both method performances for the Sabine WAM is summarized in Table 5.7. The maximum DAR is 2.32, and the minimum DAR is 0.43 among 26 pairs in total. The average NSE is 0.92 in the conventional method, and 0.92 in the suggested method, respectively. The minimum NSE of conventional method is 0.79 from the pair, control points SRLP and SRBU, and the suggested method is 0.81 from the same pair. Both methods, however, show the same performances in the cases of the 5 pairs mentioned above.

Table 5.8 summarizes the performance of both methods for the Neches WAM. The 79 pairs in total have DAR 38.79 in maximum, and DAR 0.03 in minimum. The average NSEs of both methods are 0.88 in total 79 pairs. However, the average NSE of the conventional method is 0.84, and the average NSE of the suggested method is 0.85 in the 43 pairs mentioned above. The minimum NSE of conventional method is 0.43 from the pair ANSR and AYSA that has maximum DAR. The suggested method has 0.64 NSE from the same pair. The transfer performance is significantly improved from 0.43 to 0.64 at the same pair.

Table 5.7 Comparative Evaluation of Performances by Both Methods for the Sabine WAM

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Drainage Area Ratio (A_y/A_x)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Conventional Method	Suggested Method
1	SRWP	SRMN	0.56	0.95	0.95
2	SRMN	SRWP	1.79	0.95	0.95
3	SRMN	LFQT	2.32	0.84	0.84
4	SRMN	SRGW	0.49	0.88	0.89
5	LFQT	SRMN	0.43	0.84	0.84
6	SRGW	SRMN	2.06	0.88	0.88
7	SRGW	SRBE	0.78	0.92	0.93
8	SRGW	SRLP	0.58	0.84	0.86
9	SRBE	SRGW	1.29	0.92	0.92
10	SRBE	SRLP	0.74	0.95	0.96
11	MCTT	MBGR	1.10	0.92	0.95
12	MBGR	MCTT	0.91	0.89	0.92
13	SRLP	SRGW	1.73	0.85	0.85
14	SRLP	SRBE	1.35	0.95	0.96
15	SRLP	SRBU	0.65	0.79	0.81
16	SRBU	SRLP	1.55	0.82	0.82
17	SRBU	SRRL	0.80	0.95	0.95
18	SRBU	SRLS	0.77	0.94	0.95
19	SRBW	SRRL	0.88	0.97	0.97
20	SRBW	SRLS	0.84	0.97	0.97
21	SRRL	SRBU	1.25	0.94	0.93
22	SRRL	SRBW	1.13	0.97	0.95
23	SRRL	SRLS	0.96	1.00	0.99
24	SRLS	SRBU	1.30	0.93	0.91
25	SRLS	SRBW	1.19	0.96	0.94
26	SRLS	SRRL	1.05	0.99	0.98

Table 5.8 Comparative Evaluation of Performances by Both Methods for the Neches WAM

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Drainage Area Ratio (A_y/A_x)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Conventional Method	Suggested Method
1	KIBR	NEPA	0.28	0.92	0.91
2	KIBR	NENE	0.20	0.93	0.92
3	NEPA	KIBR	3.61	0.89	0.92
4	NEPA	NENE	0.73	0.96	0.96
5	NEPA	NEAL	0.43	0.85	0.86
6	NENE	KIBR	4.94	0.91	0.92
7	NENE	NEPA	1.37	0.96	0.95
8	NENE	NEAL	0.59	0.90	0.90
9	NEAL	NEPA	2.32	0.84	0.84
10	NEAL	NENE	1.70	0.88	0.90
11	NEAL	NEDI	0.71	0.94	0.94
12	NEDI	NEAL	1.40	0.94	0.94
13	NEDI	NERO	0.75	0.94	0.95
14	NEDI	NEBA	0.28	0.66	0.68
15	NEDI	NESL	0.27	0.63	0.66
16	NERO	NEDI	1.33	0.95	0.94
17	NERO	ANSR	1.05	0.81	0.84
18	NERO	NETB	0.48	0.94	0.95
19	NERO	NEEV	0.46	0.92	0.93
20	NERO	NEBA	0.37	0.85	0.86
21	NERO	NESL	0.36	0.83	0.85
22	MUJA	ANAL	0.30	0.89	0.89
23	MUJA	ANLU	0.23	0.86	0.86
24	EFACU	ANAL	0.12	0.87	0.87
25	EFACU	ANLU	0.10	0.88	0.88
26	ANAL	MUJA	3.39	0.88	0.89
27	ANAL	ANLU	0.80	0.99	0.99
28	ANAL	ANSR	0.37	0.82	0.83
29	ANAL	NETB	0.17	0.84	0.84
30	ANAL	NEEV	0.16	0.81	0.81
31	ANLU	NEAL	0.82	0.81	0.80
32	ANLU	NEDI	0.59	0.85	0.84
33	ANLU	MUJA	4.26	0.86	0.86
34	ANLU	EFACU	10.20	0.88	0.88

Table 5.8 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Drainage Area Ratio (A_y/A_x)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Conventional Method	Suggested Method
35	ANLU	ANAL	1.26	0.99	0.98
36	ANLU	ANSR	0.46	0.88	0.89
37	ANLU	NETB	0.21	0.88	0.88
38	ANLU	NEEV	0.20	0.86	0.86
39	ATCH	AYSA	5.66	0.65	0.74
40	ATCH	ANSR	0.15	0.86	0.86
41	AYSA	ATCH	0.18	0.80	0.80
42	AYSA	ANSR	0.03	0.75	0.76
43	ANSR	NEDI	1.27	0.80	0.78
44	ANSR	NERO	0.95	0.86	0.85
45	ANSR	ANAL	2.71	0.86	0.85
46	ANSR	ANLU	2.16	0.90	0.89
47	ANSR	ATCH	6.85	0.81	0.85
48	ANSR	AYSA	38.79	0.43	0.64
49	ANSR	NETB	0.46	0.96	0.96
50	ANSR	NEEV	0.44	0.95	0.95
51	ANSR	NEBA	0.35	0.91	0.91
52	ANSR	NESL	0.34	0.90	0.91
53	NETB	NEDI	2.78	0.88	0.85
54	NETB	NERO	2.09	0.94	0.93
55	NETB	ANAL	5.95	0.84	0.84
56	NETB	ANLU	4.73	0.87	0.88
57	NETB	ANSR	2.19	0.95	0.97
58	NETB	NEEV	0.96	0.99	0.99
59	NETB	NEBA	0.77	0.94	0.95
60	NETB	NESL	0.76	0.93	0.95
61	NEEV	NEDI	2.89	0.85	0.82
62	NEEV	NERO	2.17	0.93	0.91
63	NEEV	ANAL	6.19	0.82	0.81
64	NEEV	ANLU	4.93	0.86	0.85
65	NEEV	ANSR	2.28	0.94	0.95
66	NEEV	NEBA	0.80	0.97	0.97
67	NEEV	NESL	0.79	0.96	0.97
68	NEBA	NEDI	3.61	0.78	0.75

Table 5.8 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Drainage Area Ratio (A_y/A_x)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Conventional Method	Suggested Method
69	NEBA	NERO	2.71	0.88	0.85
70	NEBA	ANSR	2.85	0.91	0.90
71	NEBA	NETB	1.30	0.95	0.94
72	NEBA	NEEV	1.25	0.97	0.96
73	NEBA	NESL	0.98	1.00	1.00
74	NESL	NEDI	3.68	0.77	0.73
75	NESL	NERO	2.76	0.87	0.84
76	NESL	ANSR	2.90	0.90	0.90
77	NESL	NETB	1.32	0.94	0.93
78	NESL	NEEV	1.27	0.96	0.95
79	NESL	NEBA	1.02	1.00	1.00

The performance of both methods, described by NSE for the GSA WAM is tabulated in Table 5.9. The 58 pairs in total have a pair with DAR 2.59 in maximum and a pair with DAR 0.21 in minimum. The performance of the conventional method has NSE 0.83 in average, and the suggested method as NSE 0.85 in average. The average NSEs of both methods in the 18 pairs mentioned above are different from the averages in total 58 pairs. The conventional method has NSE 0.74 in average, and the suggested method has NSE 0.78 in average. The lowest performance is NSE 0.22 from the pair CP28 and CP21 by the conventional method and NSE 0.58 from the same pair by the suggested method, respectively. The transfer performance is tremendously enhanced from 0.22 to 0.58 at the same pair. In addition, the suggested method improves the performance from 0.42 to 0.68 at the pair CP28 and CP23.

Table 5.9 Comparative Evaluation of Performances by Both Methods for the GSA WAM

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Drainage Area Ratio (A_y/A_x)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Conventional Method	Suggested Method
1	CP02	CP01	1.57	0.81	0.80
2	CP01	CP02	0.64	0.81	0.82
3	CP03	CP02	1.09	0.98	0.98
4	CP04	CP02	1.16	0.96	0.94
5	CP06	CP02	1.60	0.55	0.61
6	CP02	CP03	0.92	0.98	0.97
7	CP04	CP03	1.06	0.97	0.95
8	CP06	CP03	1.47	0.70	0.71
9	CP02	CP04	0.87	0.96	0.96
10	CP03	CP04	0.94	0.97	0.98
11	CP06	CP04	1.38	0.72	0.78
12	CP02	CP06	0.63	0.78	0.78
13	CP03	CP06	0.68	0.83	0.83
14	CP04	CP06	0.72	0.86	0.85
15	CP09	CP08	1.16	0.96	0.98
16	CP10	CP08	2.36	0.77	0.76
17	CP08	CP09	0.86	0.96	0.95
18	CP10	CP09	2.04	0.76	0.71
19	CP08	CP10	0.42	0.81	0.81
20	CP09	CP10	0.49	0.78	0.78
21	CP15	CP14	1.05	0.99	0.99
22	CP38	CP14	2.05	0.77	0.88
23	CPEST	CP14	2.05	0.78	0.87
24	CP14	CP15	0.95	0.99	0.98
25	CP38	CP15	1.95	0.79	0.89
26	CPEST	CP15	1.95	0.81	0.89
27	CP27	CP21	1.52	0.73	0.84
28	CP28	CP21	2.07	0.22	0.58
29	CP27	CP23	1.48	0.81	0.87
30	CP28	CP23	2.02	0.42	0.68
31	CP261	CP25	1.03	0.96	0.98
32	CP262	CP25	0.48	0.96	0.96
33	CP263	CP25	0.21	0.95	0.88
34	CP25	CP261	0.97	0.97	0.95

Table 5.9 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Drainage Area Ratio (A_y/A_x)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Conventional Method	Suggested Method
35	CP25	CP262	2.07	0.97	0.91
36	CP21	CP27	0.66	0.84	0.82
37	CP23	CP27	0.68	0.87	0.86
38	CP28	CP27	1.36	0.89	0.96
39	CP29	CP27	1.81	0.78	0.85
40	CP32	CP27	2.19	0.63	0.78
41	CP21	CP28	0.48	0.70	0.70
42	CP23	CP28	0.50	0.75	0.75
43	CP27	CP28	0.73	0.93	0.91
44	CP29	CP28	1.33	0.90	0.88
45	CP32	CP28	1.61	0.86	0.86
46	CP27	CP29	0.55	0.86	0.85
47	CP28	CP29	0.75	0.90	0.91
48	CP32	CP29	1.21	0.95	0.97
49	CP27	CP32	0.46	0.80	0.81
50	CP28	CP32	0.62	0.88	0.88
51	CP29	CP32	0.82	0.96	0.95
52	CP38	CP37	2.59	0.76	0.65
53	CPEST	CP37	2.59	0.73	0.62
54	CP14	CP38	0.49	0.85	0.85
55	CP15	CP38	0.51	0.86	0.86
56	CP37	CP38	0.39	0.63	0.59
57	CP14	CPEST	0.49	0.85	0.85
58	CP15	CPEST	0.51	0.86	0.86

Table 5.10 summarizes the performance of both methods for the Trinity WAM. Maximum DAR is 75.56, and minimum DAR is 0.01 in total 194 pairs. The average NSE is 0.77 by the conventional method and 0.84 by the suggested method, respectively. The performance of the suggested method is apparently superior to the conventional method. The performances of both methods in the 113 pairs mentioned above show more obvious

differences. The average NSE by the conventional method is 0.70, and the average NSE by the suggested method is 0.81. The suggested method leads to acceptable NSEs from all the 113 pairs, while the conventional method results in unacceptable NSEs (negative values) in the 5 pairs out of the 113 pairs. In other words, the suggested method obviously enhances the performances from unacceptable to acceptable levels in the 5 pairs. The minimum performances are -0.49 by the conventional method, 0.40 by the suggested method, respectively. In addition, the suggested method enhances the transfer performances in 71 pairs out of the 113 pairs that have less than 0.3 or more than 1.5 of DAR.

As the results of the evaluation of performances, the conventional method leads to 24 unsatisfied (below 0.5 of NSE) performances and 5 unacceptable (negative value of NSE) performances, but the suggested method results in only 2 unsatisfied performances out of total 357 pairs.

5.3 Regional Statistical Method

5.3.1 Regional Statistical Parameters

The mean or standard deviation of annual or monthly flows is significantly related to the basin or climatic variables (Benson and Matalas, 1967). Benson and Matalas (1967) suggested the general form of the equation as:

$$Y = aA^{b_1}S^{b_2}S_t^{b_3}P^{b_4}S_n^{b_5}F^{b_6} \quad (5.1)$$

Where, Y is the dependent variable, and A, S, S_t , P, S_n , and F are the basin or climatic variables. a is the regression constant, and the b values are the regression coefficients.

The statistical parameters such as mean and variance of naturalized flow dataset should be highly related to basin area because of its homogeneity or statistic stationarity. Based upon this assumption, linear regressions equations could be derived from logarithmic relationship between statistical parameters and only basin areas.

Table 5.10 Comparative Evaluation of Performances by Both Methods for the Trinity WAM

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Drainage Area Ratio (A_y/A_x)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Conventional Method	Suggested Method
1	8WTJA	8WTBO	0.40	0.84	0.85
2	8BSBR	8WTBO	0.19	0.83	0.84
3	8WTBO	8WTJA	2.53	0.85	0.82
4	8WTBO	8BSBR	5.18	0.69	0.87
5	8WTBO	8WTFW	0.66	0.91	0.93
6	8WTBO	8WTGP	0.56	0.85	0.88
7	8WTBO	8DNJU	4.31	0.60	0.85
8	8WTBO	8DNGR	2.45	0.40	0.73
9	8WTBO	8TRDA	0.28	0.41	0.41
10	8CTAL	8CTBE	0.58	0.88	0.92
11	8CTAL	8CTFW	0.48	0.78	0.83
12	8CTBE	8CTAL	1.72	0.92	0.85
13	8CTBE	8CTFW	0.83	0.97	0.99
14	8CTFW	8CTAL	2.06	0.87	0.78
15	8CTFW	8CTBE	1.20	0.98	0.93
16	8WTFW	8WTBO	1.52	0.93	0.88
17	8WTFW	8WTGP	0.85	0.97	0.97
18	8WTFW	8DNJU	6.54	0.74	0.87
19	8WTFW	8DNGR	3.71	0.65	0.85
20	8WTFW	8TRDA	0.43	0.72	0.77
21	8WTFW	8TRRS	0.32	0.54	0.56
22	8WTFW	8TRTR	0.31	0.49	0.50
23	8WTGP	8WTBO	1.78	0.88	0.82
24	8WTGP	8WTFW	1.17	0.98	0.95
25	8WTGP	8DNJU	7.66	0.73	0.83
26	8WTGP	8DNGR	4.35	0.67	0.85
27	8WTGP	8TRDA	0.50	0.79	0.85
28	8WTGP	8TRRS	0.38	0.64	0.68
29	8WTGP	8TRTR	0.36	0.61	0.64
30	8ELSA	8IDPP	1.43	0.83	0.83
31	8ELSA	8CLSA	1.29	0.87	0.79
32	8ELSA	8ELLE	0.23	0.84	0.84
33	8ELSA	8DNJU	0.95	0.72	0.66
34	8ELSA	8DNGR	0.54	0.75	0.72

Table 5.10 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Drainage Area Ratio (A_y/A_x)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Conventional Method	Suggested Method
35	8ELSA	8TRDA	0.06	0.72	0.78
36	8IDPP	8ELSA	0.70	0.82	0.83
37	8IDPP	8CLSA	0.90	0.76	0.71
38	8IDPP	8ELLE	0.16	0.95	0.96
39	8IDPP	8DNGR	0.38	0.73	0.72
40	8IDPP	8TRDA	0.04	0.76	0.84
41	8IDPP	8TRRS	0.03	0.76	0.83
42	8CLSA	8ELSA	0.77	0.74	0.85
43	8CLSA	8IDPP	1.11	0.54	0.71
44	8CLSA	8ELLE	0.18	0.73	0.68
45	8CLSA	8DNJU	0.74	0.91	0.88
46	8CLSA	8DNGR	0.42	0.90	0.90
47	8CLSA	8TRDA	0.05	0.87	0.85
48	8ELLE	8ELSA	4.39	0.80	0.83
49	8ELLE	8IDPP	6.29	0.94	0.93
50	8ELLE	8CLSA	5.67	0.83	0.70
51	8ELLE	8DNJU	4.18	0.75	0.61
52	8ELLE	8DNGR	2.37	0.81	0.71
53	8ELLE	8TRDA	0.27	0.84	0.84
54	8ELLE	8ETMK	8.81	0.61	0.83
55	8ELLE	8TRRS	0.21	0.83	0.85
56	8ELLE	8TRTR	0.20	0.81	0.83
57	8DNJU	8WTBO	0.23	0.81	0.81
58	8DNJU	8WTFW	0.15	0.84	0.86
59	8DNJU	8WTGP	0.13	0.83	0.84
60	8DNJU	8ELSA	1.05	0.22	0.53
61	8DNJU	8CLSA	1.36	0.87	0.93
62	8DNJU	8ELLE	0.24	0.44	0.40
63	8DNJU	8DNGR	0.57	0.95	0.96
64	8DNJU	8TRDA	0.07	0.88	0.79
65	8DNGR	8WTBO	0.41	0.75	0.73
66	8DNGR	8WTFW	0.27	0.82	0.82
67	8DNGR	8WTGP	0.23	0.82	0.83
68	8DNGR	8ELSA	1.85	0.40	0.71

Table 5.10 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Drainage Area Ratio (A_y/A_x)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Conventional Method	Suggested Method
69	8DNGR	8IDPP	2.65	0.35	0.72
70	8DNGR	8CLSA	2.39	0.88	0.91
71	8DNGR	8ELLE	0.42	0.64	0.70
72	8DNGR	8DNJU	1.76	0.95	0.90
73	8DNGR	8TRDA	0.12	0.93	0.91
74	8DNGR	8TRRS	0.09	0.83	0.78
75	8DNGR	8TRTR	0.08	0.80	0.73
76	8TRDA	8WTBO	3.54	0.73	0.60
77	8TRDA	8WTFW	2.33	0.84	0.73
78	8TRDA	8WTGP	1.99	0.88	0.78
79	8TRDA	8ELSA	16.03	0.26	0.79
80	8TRDA	8IDPP	22.95	0.36	0.89
81	8TRDA	8CLSA	20.70	0.82	0.80
82	8TRDA	8ELLE	3.65	0.66	0.91
83	8TRDA	8DNJU	15.27	0.88	0.75
84	8TRDA	8DNGR	8.66	0.92	0.84
85	8TRDA	8ETMK	32.14	-0.49	0.72
86	8TRDA	8TRRS	0.75	0.95	0.96
87	8TRDA	8TRTR	0.72	0.92	0.94
88	8ETMK	8ELLE	0.11	0.78	0.81
89	8ETMK	8TRDA	0.03	0.59	0.71
90	8ETMK	8SGPR	1.68	0.96	0.93
91	8ETMK	8ETLA	0.25	0.93	0.93
92	8ETMK	8ETFO	0.17	0.92	0.92
93	8ETMK	8ETCR	0.15	0.91	0.91
94	8ETMK	8TRRS	0.02	0.66	0.79
95	8ETMK	8TRTR	0.02	0.66	0.79
96	8SGPR	8ETMK	0.59	0.96	0.96
97	8SGPR	8ETLA	0.15	0.92	0.93
98	8SGPR	8ETFO	0.10	0.92	0.91
99	8SGPR	8ETCR	0.09	0.92	0.91
100	8SGPR	8TRRS	0.01	0.65	0.81
101	8SGPR	8TRTR	0.01	0.66	0.81
102	8ETLA	8ETMK	4.07	0.91	0.92

Table 5.10 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Drainage Area Ratio (A_y/A_x)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Conventional Method	Suggested Method
103	8ETLA	8SGPR	6.84	0.91	0.89
104	8ETLA	8ETFO	0.69	0.96	0.96
105	8ETLA	8ETCR	0.62	0.95	0.95
106	8ETLA	8TRRS	0.09	0.69	0.75
107	8ETLA	8TRTR	0.09	0.69	0.75
108	8ETFO	8ETMK	5.88	0.91	0.87
109	8ETFO	8SGPR	9.89	0.92	0.85
110	8ETFO	8ETLA	1.45	0.96	0.92
111	8ETFO	8ETCR	0.89	0.99	0.97
112	8ETFO	8TRRS	0.14	0.67	0.71
113	8ETFO	8TRTR	0.13	0.68	0.72
114	8ETCR	8ETMK	6.61	0.89	0.87
115	8ETCR	8SGPR	11.12	0.90	0.85
116	8ETCR	8ETLA	1.62	0.95	0.92
117	8ETCR	8ETFO	1.12	0.99	0.98
118	8ETCR	8TRRS	0.15	0.69	0.72
119	8ETCR	8TRTR	0.15	0.69	0.73
120	8TRRS	8WTFW	3.12	0.75	0.62
121	8TRRS	8WTGP	2.66	0.80	0.68
122	8TRRS	8IDPP	30.62	0.40	0.84
123	8TRRS	8ELLE	4.87	0.67	0.88
124	8TRRS	8DNGR	11.55	0.83	0.72
125	8TRRS	8TRDA	1.33	0.96	0.92
126	8TRRS	8ETMK	42.87	-0.18	0.84
127	8TRRS	8SGPR	72.09	-0.14	0.85
128	8TRRS	8ETLA	10.54	0.15	0.78
129	8TRRS	8ETFO	7.29	0.02	0.72
130	8TRRS	8ETCR	6.49	0.12	0.74
131	8TRRS	8TRTR	0.95	0.98	0.97
132	8TRRS	8TROA	0.63	0.85	0.86
133	8TRTR	8WTFW	3.27	0.73	0.60
134	8TRTR	8WTGP	2.79	0.78	0.66
135	8TRTR	8ELLE	5.10	0.63	0.84
136	8TRTR	8DNGR	12.11	0.80	0.69

Table 5.10 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Drainage Area Ratio (A_y/A_x)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Conventional Method	Suggested Method
137	8TRTR	8TRDA	1.40	0.93	0.89
138	8TRTR	8ETMK	44.94	-0.18	0.83
139	8TRTR	8SGPR	75.56	-0.13	0.84
140	8TRTR	8ETLA	11.05	0.15	0.78
141	8TRTR	8ETFO	7.64	0.04	0.74
142	8TRTR	8ETCR	6.80	0.15	0.76
143	8TRTR	8TRRS	1.05	0.98	0.96
144	8TRTR	8TROA	0.67	0.89	0.88
145	8CEKE	8KGKA	0.81	0.87	0.90
146	8CEKE	8CEMA	0.26	0.93	0.94
147	8KGKA	8CEKE	1.23	0.89	0.85
148	8KGKA	8CEMA	0.32	0.90	0.89
149	8CEMA	8CEKE	3.88	0.91	0.95
150	8CEMA	8KGKA	3.15	0.81	0.97
151	8RIDA	8RIRI	0.45	0.96	0.96
152	8RIDA	8RIFA	0.17	0.89	0.89
153	8RIRI	8RIDA	2.20	0.96	0.93
154	8RIRI	8RIFA	0.38	0.92	0.91
155	8WABA	8CHCO	0.18	0.94	0.95
156	8WABA	8RIFA	0.09	0.88	0.87
157	8CHCO	8WABA	5.41	0.93	0.94
158	8CHCO	8RIFA	0.49	0.92	0.92
159	8RIFA	8RIDA	5.88	0.85	0.87
160	8RIFA	8RIRI	2.67	0.89	0.92
161	8RIFA	8WABA	10.99	0.85	0.83
162	8RIFA	8CHCO	2.03	0.92	0.88
163	8TROA	8TRRS	1.58	0.84	0.83
164	8TROA	8TRTR	1.50	0.88	0.87
165	8TROA	8TRCR	0.92	0.97	0.95
166	8TROA	8TRMI	0.89	0.97	0.96
167	8TROA	8TRRI	0.82	0.94	0.93
168	8TROA	8TRRO	0.75	0.88	0.88
169	8TROA	8TRGB	0.71	0.85	0.86
170	8TRCR	8TROA	1.08	0.97	0.95

Table 5.10 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Drainage Area Ratio (A_y/A_x)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Conventional Method	Suggested Method
171	8TRCR	8TRMI	0.96	0.99	0.98
172	8TRCR	8TRRI	0.89	0.98	0.97
173	8TRCR	8TRRO	0.81	0.94	0.93
174	8TRCR	8TRGB	0.78	0.91	0.91
175	8TRMI	8TROA	1.13	0.97	0.95
176	8TRMI	8TRCR	1.04	0.99	0.97
177	8TRMI	8TRRI	0.93	0.98	0.97
178	8TRMI	8TRRO	0.84	0.94	0.93
179	8TRMI	8TRGB	0.81	0.92	0.91
180	8TRRI	8TROA	1.21	0.94	0.91
181	8TRRI	8TRCR	1.12	0.98	0.96
182	8TRRI	8TRMI	1.08	0.98	0.96
183	8TRRI	8TRRO	0.91	0.97	0.95
184	8TRRI	8TRGB	0.87	0.95	0.94
185	8TRRO	8TROA	1.34	0.87	0.86
186	8TRRO	8TRCR	1.24	0.93	0.91
187	8TRRO	8TRMI	1.19	0.94	0.92
188	8TRRO	8TRRI	1.10	0.97	0.95
189	8TRRO	8TRGB	0.96	0.99	0.98
190	8TRGB	8TROA	1.40	0.84	0.82
191	8TRGB	8TRCR	1.29	0.91	0.88
192	8TRGB	8TRMI	1.24	0.91	0.89
193	8TRGB	8TRRI	1.15	0.95	0.92
194	8TRGB	8TRRO	1.04	0.99	0.96

Figure 5.1 shows the logarithmic relationships between statistical parameters of monthly naturalized flows and watershed areas at the control points in the Sabine River Basin. The coefficients of determination (R^2) are 0.9903 for means and 0.9897 for standard deviations, respectively.

The statistic parameters of monthly naturalized flows in the Neches River Basin also have logarithmic relationships with the basin area, as shown in Figure 5.2. The logarithmic relationship between mean and basin area has a R^2 value of 0.9843, and the same relationship between standard deviation and basin area has a R^2 value of 0.9805.

The GSA River Basins have highly linear logarithmic relationships between statistical parameters (mean and standard deviation) and basin area as shown in Figure 5.3. R^2 values of each relationship are 0.9413 for mean and 0.9741 for standard deviation, respectively. This shows the relationship between standard deviation and basin area is more linear than the relationship between mean and basin area for the GSA River Basins.

In the Trinity River Basin, Figure 5.4 describes the logarithmic relationships between two statistical parameters, mean and standard deviation and basin area. The linear regression equations have 0.9308 R^2 value for mean, and 0.9596 R^2 value for standard deviation, respectively.

The regression equations in Figures 5.1 to 5.4 are made based on logarithm variables. These logarithmic equations are converted into algebraic equations, and these are summarized in Table 5.11.

Table 5.11 Algebraic Regression Equations for Regional Statistical Parameters

WAM	Algebraic Regression Equation (X: Basin Area, Y: Regional Statistical Parameters)	
	Mean (Y)	Standard Deviation (y)
Sabine	$Y=10^{1.8384 X^{0.9662}}$	$y=10^{2.1044 X^{0.9111}}$
Neches	$Y=10^{1.7568 X^{0.9735}}$	$y=10^{1.9456 X^{0.9399}}$
GSA	$Y=10^{0.6973 X^{1.1628}}$	$y=10^{1.2333 X^{1.0517}}$
Trinity	$Y=10^{1.5858 X^{0.9521}}$	$y=10^{2.0403 X^{0.8837}}$

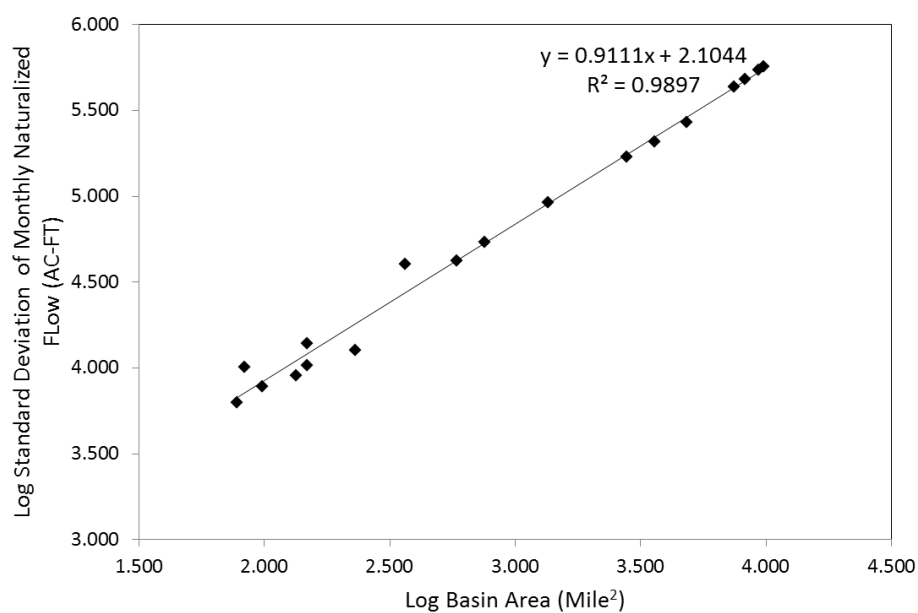
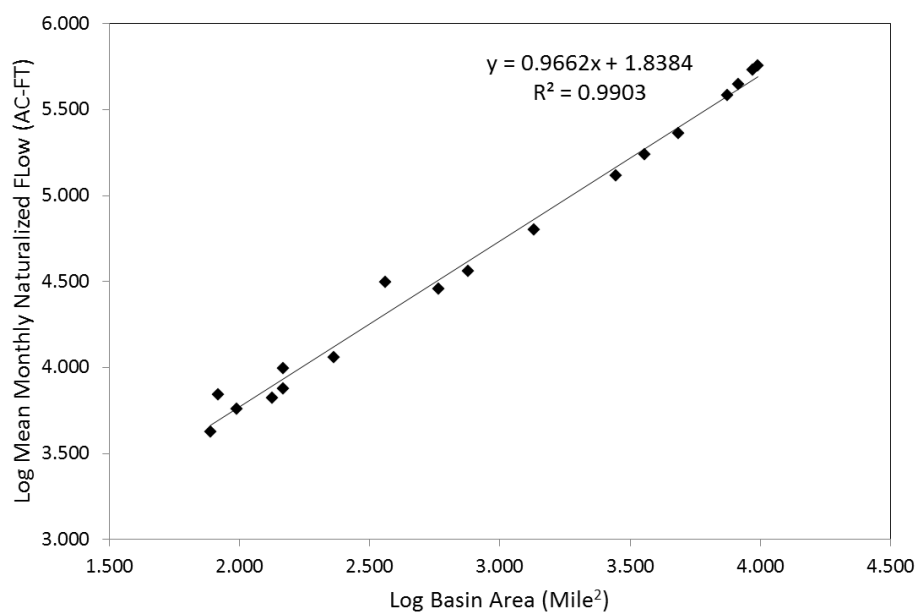


Figure 5.1 Logarithmic Linear Relationship between Mean (above) and Standard Deviation (below) of Monthly Naturalized Flows and Basin Area for the Sabine River Basin

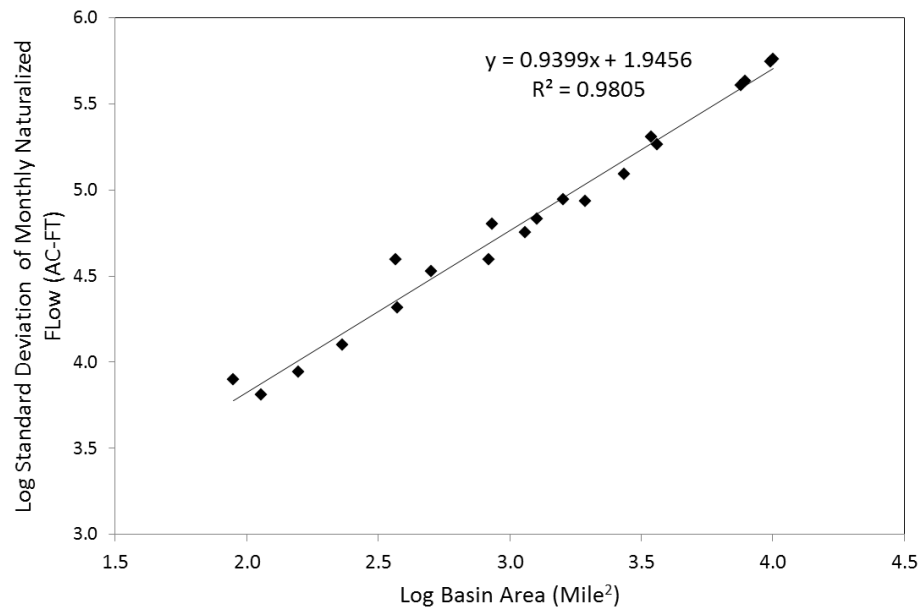
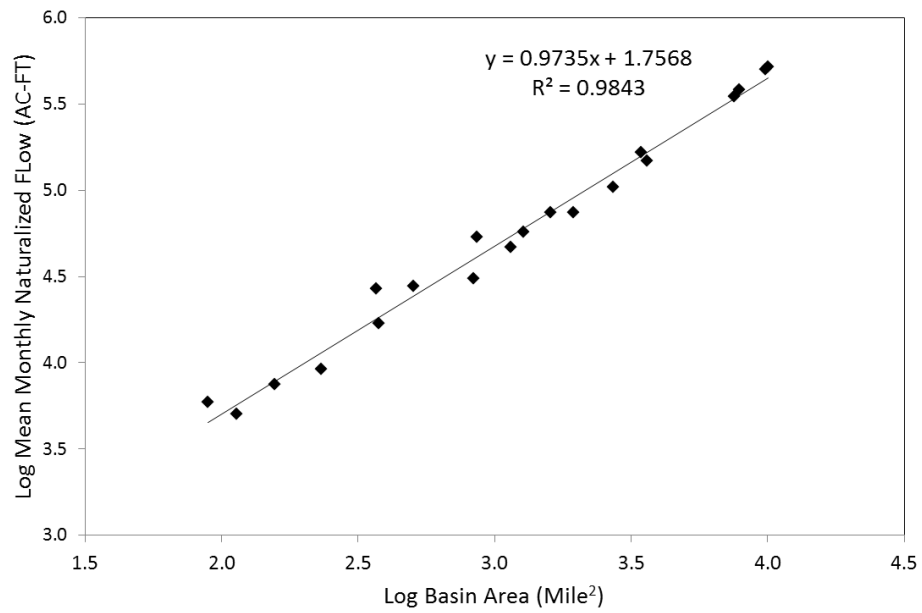


Figure 5.2 Logarithmic Linear Relationship between Mean (above) and Standard Deviation (below) of Monthly Naturalized Flows and Basin Area for the Neches River Basin

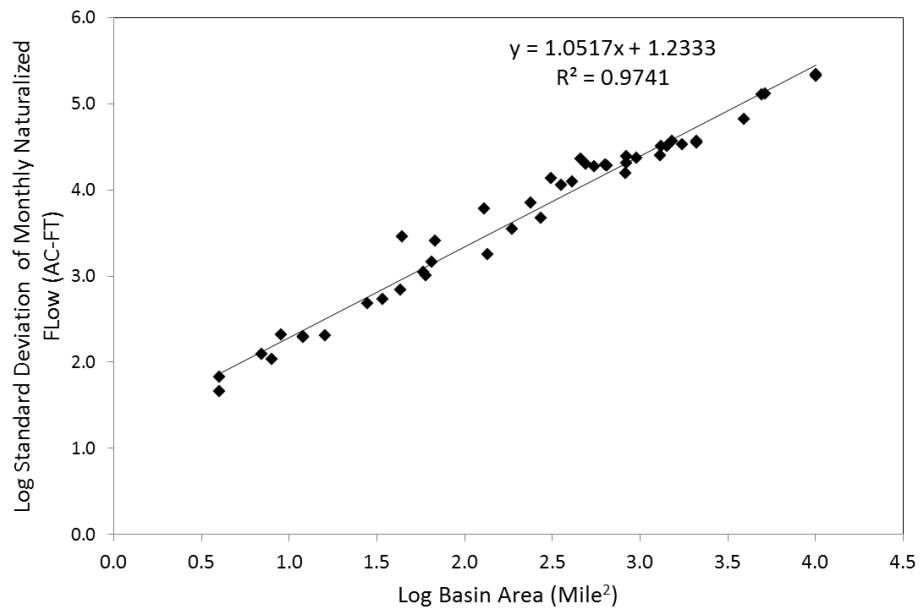
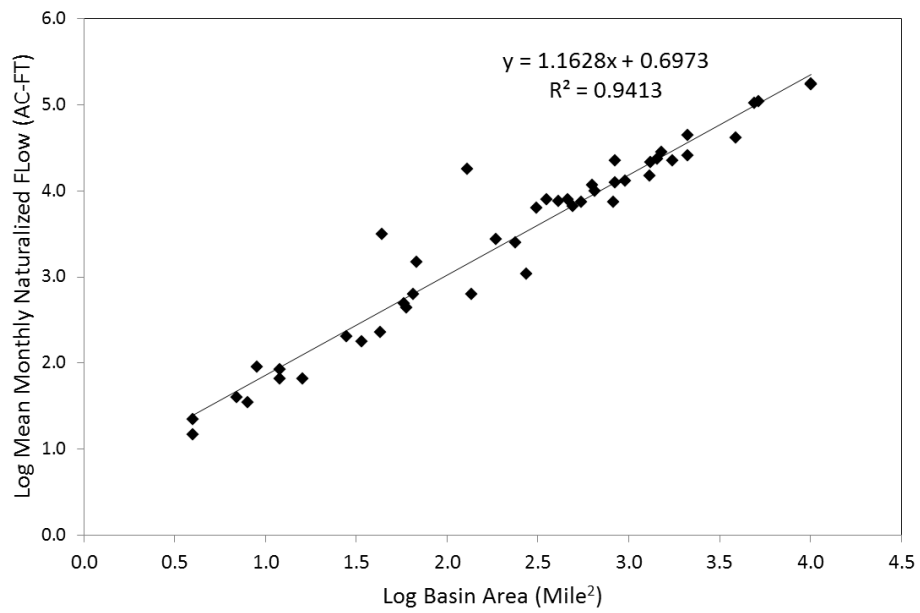


Figure 5.3 Logarithmic Linear Relationship between Mean (above) and Standard Deviation (below) of Monthly Naturalized Flows and Basin Area for the GSA River Basins

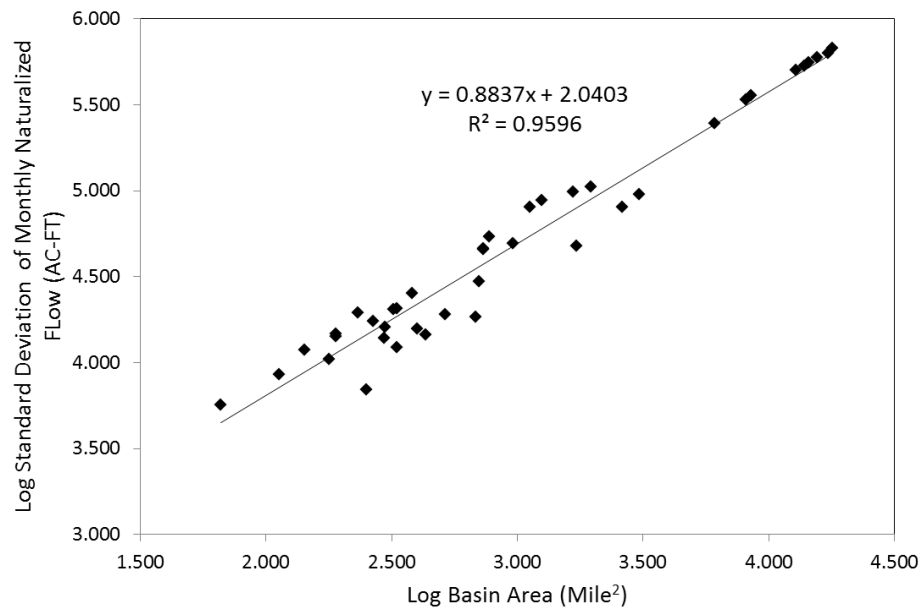
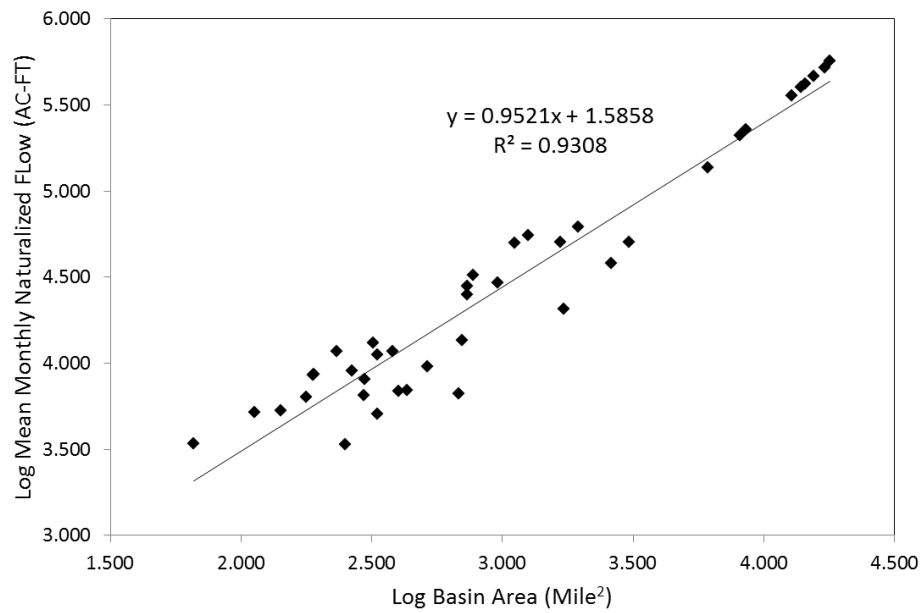


Figure 5.4 Logarithmic Linear Relationship between Mean (above) and Standard Deviation (below) of Monthly Naturalized Flows and Basin Area for the Trinity River Basin

5.3.2 Comparative Evaluation of the Method Performances

Monthly naturalized flow sequences at gauged sites treated as ungauged target sites are synthesized based on naturalized flows at other gauged sites treated as source sites for each period-of-analysis in the four WAMs using Equation 2.13. When Equation 2.13 is used, the first method (“the existing method”) uses sample mean and standard deviation from monthly naturalized flow data for $m(x)$ and $S(x)$ and estimated mean and standard deviation from a regional regression equation for $m(y)$ and $S(y)$, but the second method (“the proposed method”) uses only estimated mean and standard deviation from a regional regression equation for $m(x)$, $m(y)$, $S(y)$, and $S(y)$. The proposed method is expected to enhance the performance of Equation 2.13 because this may remove expected biases on statistic parameters mentioned in Chapter II. The performances of both methods are evaluated with NSE between the naturalized flows and synthesized flows by two different methods, and both NSEs at same pair are compared for evaluating how much or well the proposed method improves the performance than the existing method.

The performances by the two different methods for the Sabine WAM are tabulated in Table 5.12. The average NSE values are 0.91 for the existing method, and 0.92 for the proposed method. Both methods result in acceptable or satisfied values. The proposed method slightly outperforms the existing method at 21 pairs out of total 26 pairs, but the differences between the two methods are imperceptible.

Table 5.13 summarizes the performances by the two methods for the Neches WAM. The two methods have the same average NSE values, 0.88. There are no unacceptable or unsatisfied performances by the two methods in total 79 pairs like the Sabine WAM. The performances of the existing method are better than the performances of the proposed method at 42 pairs out of total 79 pairs in the Neches WAM. However, the differences between both performances are also marginal like the Sabine WAM.

Table 5.12 Comparative Evaluation of Performances by Both Methods for the Sabine WAM

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Existing Method	Proposed Method
1	SRWP	SRMN	0.98	0.95	0.95
2	SRMN	SRWP	0.98	0.94	0.95
3	SRMN	LFQT	0.92	0.83	0.84
4	SRMN	SRGW	0.94	0.88	0.89
5	LFQT	SRMN	0.92	0.83	0.84
6	SRGW	SRMN	0.94	0.87	0.88
7	SRGW	SRBE	0.96	0.92	0.93
8	SRGW	SRLP	0.93	0.85	0.86
9	SRBE	SRGW	0.96	0.92	0.92
10	SRBE	SRLP	0.98	0.95	0.96
11	MCTT	MBGR	0.98	0.93	0.97
12	MBGR	MCTT	0.98	0.90	0.97
13	SRLP	SRGW	0.93	0.84	0.85
14	SRLP	SRBE	0.98	0.95	0.96
15	SRLP	SRBU	0.91	0.80	0.79
16	SRBU	SRLP	0.91	0.82	0.82
17	SRBU	SRRL	0.98	0.96	0.94
18	SRBU	SRL	0.98	0.95	0.93
19	SRBW	SRRL	0.99	0.97	0.97
20	SRBW	SRL	0.98	0.97	0.96
21	SRRL	SRBU	0.98	0.94	0.94
22	SRRL	SRBW	0.99	0.96	0.97
23	SRRL	SRL	1.00	0.98	1.00
24	SRL	SRBU	0.98	0.93	0.93
25	SRL	SRBW	0.98	0.95	0.96
26	SRL	SRRL	1.00	0.98	1.00

Table 5.13 Comparative Evaluation of Performances by Both Methods for the Neches WAM

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Existing Method	Proposed Method
1	KIBR	NEPA	0.97	0.86	0.93
2	KIBR	NENE	0.96	0.85	0.93
3	NEPA	KIBR	0.97	0.79	0.92
4	NEPA	NENE	0.98	0.84	0.95
5	NEPA	NEAL	0.92	0.69	0.85
6	NENE	KIBR	0.96	0.87	0.93
7	NENE	NEPA	0.98	0.92	0.96
8	NENE	NEAL	0.95	0.84	0.91
9	NEAL	NEPA	0.92	0.70	0.85
10	NEAL	NENE	0.95	0.77	0.89
11	NEAL	NEDI	0.97	0.82	0.94
12	NEDI	NEAL	0.97	0.85	0.94
13	NEDI	NERO	0.98	0.88	0.94
14	NEDI	NEBA	0.92	0.74	0.55
15	NEDI	NESL	0.92	0.73	0.52
16	NERO	NEDI	0.98	0.94	0.95
17	NERO	ANSR	0.94	0.86	0.81
18	NERO	NETB	0.98	0.94	0.92
19	NERO	NEEV	0.98	0.93	0.90
20	NERO	NEBA	0.96	0.90	0.80
21	NERO	NESL	0.96	0.89	0.78
22	MUJA	ANAL	0.94	0.85	0.88
23	MUJA	ANLU	0.93	0.82	0.83
24	EFACU	ANAL	0.93	0.82	0.85
25	EFACU	ANLU	0.94	0.83	0.85
26	ANAL	MUJA	0.94	0.87	0.89
27	ANAL	ANLU	0.99	0.98	0.98
28	ANAL	ANSR	0.93	0.84	0.79
29	ANAL	NETB	0.92	0.82	0.80
30	ANAL	NEEV	0.91	0.79	0.76
31	ANLU	NEAL	0.92	0.83	0.81
32	ANLU	NEDI	0.94	0.88	0.86
33	ANLU	MUJA	0.93	0.85	0.86
34	ANLU	EFACU	0.94	0.87	0.88

Table 5.13 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Existing Method	Proposed Method
35	ANLU	ANAL	0.99	0.99	0.99
36	ANLU	ANSR	0.95	0.89	0.87
37	ANLU	NETB	0.94	0.87	0.86
38	ANLU	NEEV	0.93	0.85	0.83
39	ATCH	AYSA	0.92	0.84	0.76
40	ATCH	ANSR	0.93	0.86	0.86
41	AYSA	ATCH	0.92	0.79	0.83
42	AYSA	ANSR	0.93	0.80	0.83
43	ANSR	NEDI	0.92	0.85	0.80
44	ANSR	NERO	0.94	0.88	0.86
45	ANSR	ANAL	0.93	0.86	0.85
46	ANSR	ANLU	0.95	0.90	0.89
47	ANSR	ATCH	0.93	0.87	0.86
48	ANSR	AYSA	0.93	0.87	0.75
49	ANSR	NETB	0.98	0.96	0.97
50	ANSR	NEEV	0.98	0.95	0.95
51	ANSR	NEBA	0.96	0.91	0.90
52	ANSR	NESL	0.95	0.91	0.89
53	NETB	NEDI	0.95	0.91	0.86
54	NETB	NERO	0.98	0.96	0.94
55	NETB	ANAL	0.92	0.84	0.84
56	NETB	ANLU	0.94	0.88	0.88
57	NETB	ANSR	0.98	0.97	0.96
58	NETB	NEEV	1.00	0.99	0.99
59	NETB	NEBA	0.98	0.96	0.94
60	NETB	NESL	0.98	0.95	0.93
61	NEEV	NEDI	0.95	0.90	0.83
62	NEEV	NERO	0.98	0.95	0.92
63	NEEV	ANAL	0.91	0.82	0.81
64	NEEV	ANLU	0.93	0.86	0.86
65	NEEV	ANSR	0.98	0.95	0.95
66	NEEV	NEBA	0.99	0.97	0.96
67	NEEV	NESL	0.98	0.96	0.95
68	NEBA	NEDI	0.92	0.84	0.76

Table 5.13 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Existing Method	Proposed Method
69	NEBA	NERO	0.96	0.90	0.86
70	NEBA	ANSR	0.96	0.90	0.91
71	NEBA	NETB	0.98	0.94	0.95
72	NEBA	NEEV	0.99	0.95	0.97
73	NEBA	NESL	1.00	0.98	1.00
74	NESL	NEDI	0.92	0.83	0.74
75	NESL	NERO	0.96	0.89	0.85
76	NESL	ANSR	0.95	0.89	0.90
77	NESL	NETB	0.98	0.93	0.94
78	NESL	NEEV	0.98	0.94	0.96
79	NESL	NEBA	1.00	0.97	1.00

The performances by the two methods for the GSA WAM are tabulated in Table 5.14. The average NSE values are 0.80 for the existing method, 0.82 for the proposed method, respectively. The proposed method leads to better performances than the existing method at 32 pairs out of total 58 pairs. This indicates that the proposed method slightly enhances the performance of the regional statistical method for the GSA WAM. However, both methods generate two unsatisfied performances (NSE value < 0.5) at four different pairs. There are no differences between the performances by both methods in this point of view.

Table 5.15 lists the NSE values at each pair to show the performances by both methods for the Trinity WAM. The existing method has 0.77 NSE value in average, and the proposed method has 0.82 NSE value in average. The proposed method generates better performances than the existing method at 121 pairs out of total 194 pairs.

Table 5.14 Comparative Evaluation of Performances by Both Methods for the GSA WAM

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Existing Method	Proposed Method
1	CP02	CP01	0.91	0.81	0.80
2	CP01	CP02	0.91	0.82	0.82
3	CP03	CP02	0.99	0.97	0.98
4	CP04	CP02	0.98	0.95	0.96
5	CP06	CP02	0.94	0.52	0.55
6	CP02	CP03	0.99	0.99	0.98
7	CP04	CP03	0.99	0.98	0.98
8	CP06	CP03	0.96	0.57	0.71
9	CP02	CP04	0.98	0.96	0.96
10	CP03	CP04	0.99	0.97	0.97
11	CP06	CP04	0.98	0.62	0.71
12	CP02	CP06	0.94	0.88	0.79
13	CP03	CP06	0.96	0.90	0.84
14	CP04	CP06	0.98	0.94	0.86
15	CP09	CP08	0.99	0.91	0.96
16	CP10	CP08	0.91	0.66	0.77
17	CP08	CP09	0.99	0.83	0.96
18	CP10	CP09	0.92	0.69	0.79
19	CP08	CP10	0.91	0.71	0.82
20	CP09	CP10	0.92	0.80	0.81
21	CP15	CP14	1.00	0.99	0.99
22	CP38	CP14	0.95	0.68	0.68
23	CPEST	CP14	0.94	0.71	0.71
24	CP14	CP15	1.00	0.99	0.99
25	CP38	CP15	0.95	0.70	0.71
26	CPEST	CP15	0.94	0.73	0.74
27	CP27	CP21	0.94	0.88	0.69
28	CP28	CP21	0.92	0.64	0.04
29	CP27	CP23	0.94	0.88	0.79
30	CP28	CP23	0.92	0.64	0.30
31	CP261	CP25	0.99	0.90	0.96
32	CP262	CP25	0.99	0.94	0.97
33	CP263	CP25	0.99	0.95	0.97
34	CP25	CP261	0.99	0.97	0.97

Table 5.14 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Existing Method	Proposed Method
35	CP25	CP262	0.99	0.97	0.98
36	CP21	CP27	0.94	0.84	0.82
37	CP23	CP27	0.94	0.87	0.86
38	CP28	CP27	0.99	0.83	0.87
39	CP29	CP27	0.94	0.71	0.74
40	CP32	CP27	0.92	0.48	0.54
41	CP21	CP28	0.92	0.80	0.66
42	CP23	CP28	0.92	0.83	0.72
43	CP27	CP28	0.99	0.98	0.92
44	CP29	CP28	0.95	0.75	0.90
45	CP32	CP28	0.94	0.54	0.85
46	CP27	CP29	0.94	0.87	0.84
47	CP28	CP29	0.95	0.73	0.91
48	CP32	CP29	0.98	0.67	0.95
49	CP27	CP32	0.92	0.83	0.78
50	CP28	CP32	0.94	0.70	0.88
51	CP29	CP32	0.98	0.83	0.96
52	CP38	CP37	0.93	0.64	0.83
53	CPEST	CP37	0.92	0.68	0.81
54	CP14	CP38	0.95	0.88	0.80
55	CP15	CP38	0.95	0.90	0.82
56	CP37	CP38	0.93	0.21	0.76
57	CP14	CPEST	0.94	0.87	0.81
58	CP15	CPEST	0.94	0.88	0.82

This apparently shows that the proposed method can improve the regional statistical method for the Trinity WAM. The existing method causes 11 unsatisfied (NSE value < 0.5), and 6 unacceptable performances (NSE value < 0), respectively, while the proposed method leads to only 9 unsatisfied performances. This means that the proposed method is expected to generate more stable and accurate performances than the existing method.

Table 5.15 Comparative Evaluation of Performances by Both Methods for the Trinity WAM

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Existing Method	Proposed Method
1	8WTJA	8WTBO	0.93	-0.58	0.81
2	8BSBR	8WTBO	0.93	0.40	0.86
3	8WTBO	8WTJA	0.93	-0.01	0.85
4	8WTBO	8BSBR	0.93	0.02	0.83
5	8WTBO	8WTFW	0.97	0.13	0.89
6	8WTBO	8WTGP	0.95	0.08	0.81
7	8WTBO	8DNJU	0.94	0.03	0.77
8	8WTBO	8DNGR	0.92	-0.02	0.57
9	8WTBO	8TRDA	0.92	-0.02	0.07
10	8CTAL	8CTBE	0.97	-0.63	0.84
11	8CTAL	8CTFW	0.97	-0.65	0.68
12	8CTBE	8CTAL	0.97	0.40	0.90
13	8CTBE	8CTFW	0.99	0.47	0.97
14	8CTFW	8CTAL	0.97	0.63	0.85
15	8CTFW	8CTBE	0.99	0.70	0.97
16	8WTFW	8WTBO	0.97	0.56	0.92
17	8WTFW	8WTGP	0.99	0.63	0.97
18	8WTFW	8DNJU	0.94	0.47	0.86
19	8WTFW	8DNGR	0.94	0.47	0.79
20	8WTFW	8TRDA	0.96	0.54	0.59
21	8WTFW	8TRRS	0.93	0.45	0.29
22	8WTFW	8TRTR	0.92	0.42	0.22
23	8WTGP	8WTBO	0.95	0.62	0.87
24	8WTGP	8WTFW	0.99	0.72	0.98
25	8WTGP	8DNJU	0.92	0.53	0.84
26	8WTGP	8DNGR	0.93	0.55	0.81
27	8WTGP	8TRDA	0.97	0.66	0.70
28	8WTGP	8TRRS	0.95	0.59	0.46
29	8WTGP	8TRTR	0.94	0.58	0.41
30	8ELSA	8IDPP	0.92	0.83	0.83
31	8ELSA	8CLSA	0.98	0.94	0.86
32	8ELSA	8ELLE	0.92	0.83	0.82
33	8ELSA	8DNJU	0.94	0.87	0.73
34	8ELSA	8DNGR	0.93	0.86	0.78

Table 5.15 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Existing Method	Proposed Method
35	8ELSA	8TRDA	0.91	0.82	0.82
36	8IDPP	8ELSA	0.92	0.83	0.81
37	8IDPP	8CLSA	0.92	0.83	0.77
38	8IDPP	8ELLE	0.98	0.95	0.95
39	8IDPP	8DNGR	0.93	0.85	0.77
40	8IDPP	8TRDA	0.95	0.89	0.88
41	8IDPP	8TRRS	0.93	0.85	0.85
42	8CLSA	8ELSA	0.98	0.88	0.69
43	8CLSA	8IDPP	0.92	0.73	0.56
44	8CLSA	8ELLE	0.94	0.77	0.45
45	8CLSA	8DNJU	0.97	0.85	0.92
46	8CLSA	8DNGR	0.95	0.82	0.91
47	8CLSA	8TRDA	0.93	0.77	0.78
48	8ELLE	8ELSA	0.92	0.82	0.84
49	8ELLE	8IDPP	0.98	0.92	0.96
50	8ELLE	8CLSA	0.94	0.84	0.77
51	8ELLE	8DNJU	0.94	0.84	0.68
52	8ELLE	8DNGR	0.96	0.87	0.78
53	8ELLE	8TRDA	0.97	0.90	0.89
54	8ELLE	8ETMK	0.91	0.82	0.80
55	8ELLE	8TRRS	0.95	0.87	0.89
56	8ELLE	8TRTR	0.93	0.84	0.86
57	8DNJU	8WTBO	0.94	0.59	0.84
58	8DNJU	8WTFW	0.94	0.58	0.87
59	8DNJU	8WTGP	0.92	0.54	0.84
60	8DNJU	8ELSA	0.94	0.60	0.23
61	8DNJU	8CLSA	0.97	0.67	0.89
62	8DNJU	8ELLE	0.94	0.57	0.02
63	8DNJU	8DNGR	0.98	0.70	0.93
64	8DNJU	8TRDA	0.95	0.61	0.65
65	8DNGR	8WTBO	0.92	0.72	0.77
66	8DNGR	8WTFW	0.94	0.75	0.85
67	8DNGR	8WTGP	0.93	0.73	0.85
68	8DNGR	8ELSA	0.93	0.75	0.53

Table 5.15 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Existing Method	Proposed Method
69	8DNGR	8IDPP	0.93	0.74	0.56
70	8DNGR	8CLSA	0.95	0.80	0.90
71	8DNGR	8ELLE	0.96	0.79	0.47
72	8DNGR	8DNJU	0.98	0.86	0.94
73	8DNGR	8TRDA	0.97	0.82	0.84
74	8DNGR	8TRRS	0.93	0.74	0.66
75	8DNGR	8TRTR	0.92	0.70	0.60
76	8TRDA	8WTBO	0.92	0.84	0.68
77	8TRDA	8WTFW	0.96	0.91	0.81
78	8TRDA	8WTGP	0.97	0.93	0.85
79	8TRDA	8ELSA	0.91	0.82	0.74
80	8TRDA	8IDPP	0.95	0.89	0.84
81	8TRDA	8CLSA	0.93	0.86	0.85
82	8TRDA	8ELLE	0.97	0.93	0.83
83	8TRDA	8DNJU	0.95	0.89	0.82
84	8TRDA	8DNGR	0.97	0.92	0.90
85	8TRDA	8ETMK	0.91	0.82	0.59
86	8TRDA	8TRRS	0.98	0.96	0.95
87	8TRDA	8TRTR	0.97	0.93	0.91
88	8ETMK	8ELLE	0.91	0.78	0.81
89	8ETMK	8TRDA	0.91	0.77	0.75
90	8ETMK	8SGPR	0.98	0.88	0.96
91	8ETMK	8ETLA	0.97	0.86	0.93
92	8ETMK	8ETFO	0.96	0.85	0.88
93	8ETMK	8ETCR	0.96	0.84	0.88
94	8ETMK	8TRRS	0.94	0.81	0.84
95	8ETMK	8TRTR	0.94	0.81	0.84
96	8SGPR	8ETMK	0.98	0.90	0.95
97	8SGPR	8ETLA	0.96	0.87	0.89
98	8SGPR	8ETFO	0.96	0.87	0.83
99	8SGPR	8ETCR	0.96	0.87	0.84
100	8SGPR	8TRRS	0.93	0.82	0.83
101	8SGPR	8TRTR	0.93	0.81	0.83
102	8ETLA	8ETMK	0.97	0.84	0.94

Table 5.15 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Existing Method	Proposed Method
103	8ETLA	8SGPR	0.96	0.83	0.92
104	8ETLA	8ETFO	0.98	0.86	0.95
105	8ETLA	8ETCR	0.98	0.85	0.95
106	8ETLA	8TRRS	0.92	0.77	0.80
107	8ETLA	8TRTR	0.92	0.77	0.80
108	8ETFO	8ETMK	0.96	0.78	0.91
109	8ETFO	8SGPR	0.96	0.78	0.89
110	8ETFO	8ETLA	0.98	0.81	0.96
111	8ETFO	8ETCR	1.00	0.83	0.99
112	8ETFO	8TRRS	0.93	0.74	0.77
113	8ETFO	8TRTR	0.93	0.74	0.78
114	8ETCR	8ETMK	0.96	0.79	0.91
115	8ETCR	8SGPR	0.96	0.79	0.89
116	8ETCR	8ETLA	0.98	0.82	0.95
117	8ETCR	8ETFO	1.00	0.84	0.99
118	8ETCR	8TRRS	0.93	0.75	0.78
119	8ETCR	8TRTR	0.93	0.75	0.79
120	8TRRS	8WTFW	0.93	0.86	0.71
121	8TRRS	8WTGP	0.95	0.89	0.76
122	8TRRS	8IDPP	0.93	0.85	0.83
123	8TRRS	8ELLE	0.95	0.90	0.84
124	8TRRS	8DNGR	0.93	0.86	0.80
125	8TRRS	8TRDA	0.98	0.96	0.95
126	8TRRS	8ETMK	0.94	0.88	0.77
127	8TRRS	8SGPR	0.93	0.86	0.79
128	8TRRS	8ETLA	0.92	0.85	0.67
129	8TRRS	8ETFO	0.93	0.86	0.57
130	8TRRS	8ETCR	0.93	0.86	0.60
131	8TRRS	8TRTR	0.99	0.98	0.98
132	8TRRS	8TROA	0.93	0.86	0.85
133	8TRTR	8WTFW	0.92	0.84	0.68
134	8TRTR	8WTGP	0.94	0.88	0.74
135	8TRTR	8ELLE	0.93	0.86	0.81
136	8TRTR	8DNGR	0.92	0.84	0.77

Table 5.15 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Existing Method	Proposed Method
137	8TRTR	8TRDA	0.97	0.93	0.93
138	8TRTR	8ETMK	0.94	0.87	0.77
139	8TRTR	8SGPR	0.93	0.86	0.80
140	8TRTR	8ETLA	0.92	0.84	0.68
141	8TRTR	8ETFO	0.93	0.86	0.59
142	8TRTR	8ETCR	0.93	0.86	0.62
143	8TRTR	8TRRS	0.99	0.98	0.98
144	8TRTR	8TROA	0.94	0.89	0.89
145	8CEKE	8KGKA	0.95	0.84	0.85
146	8CEKE	8CEMA	0.98	0.89	0.96
147	8KGKA	8CEKE	0.95	0.77	0.89
148	8KGKA	8CEMA	0.99	0.82	0.94
149	8CEMA	8CEKE	0.98	0.90	0.96
150	8CEMA	8KGKA	0.99	0.91	0.92
151	8RIDA	8RIRI	0.98	0.95	0.95
152	8RIDA	8RIFA	0.95	0.89	0.88
153	8RIRI	8RIDA	0.98	0.93	0.96
154	8RIRI	8RIFA	0.97	0.90	0.92
155	8WABA	8CHCO	0.98	0.95	0.95
156	8WABA	8RIFA	0.94	0.86	0.80
157	8CHCO	8WABA	0.98	0.96	0.96
158	8CHCO	8RIFA	0.96	0.92	0.89
159	8RIFA	8RIDA	0.95	0.88	0.90
160	8RIFA	8RIRI	0.97	0.90	0.92
161	8RIFA	8WABA	0.94	0.86	0.87
162	8RIFA	8CHCO	0.96	0.90	0.92
163	8TROA	8TRRS	0.93	0.85	0.85
164	8TROA	8TRTR	0.94	0.88	0.88
165	8TROA	8TRCR	0.99	0.96	0.97
166	8TROA	8TRMI	0.99	0.96	0.97
167	8TROA	8TRRI	0.97	0.93	0.94
168	8TROA	8TRRO	0.94	0.88	0.88
169	8TROA	8TRGB	0.93	0.86	0.85
170	8TRCR	8TROA	0.99	0.95	0.97

Table 5.15 (Continued)

No.	Assumed Ungauged Site (Y)	Assumed Gauged Site (X)	Correlation Coefficient (r)	Nash-Sutcliffe Coefficient (Naturalized vs. Transferred)	
				Existing Method	Proposed Method
171	8TRCR	8TRMI	1.00	0.98	0.99
172	8TRCR	8TRRI	0.99	0.97	0.98
173	8TRCR	8TRRO	0.97	0.92	0.94
174	8TRCR	8TRGB	0.96	0.90	0.91
175	8TRMI	8TROA	0.99	0.95	0.97
176	8TRMI	8TRCR	1.00	0.97	0.99
177	8TRMI	8TRRI	0.99	0.97	0.98
178	8TRMI	8TRRO	0.97	0.93	0.94
179	8TRMI	8TRGB	0.96	0.90	0.91
180	8TRRI	8TROA	0.97	0.92	0.94
181	8TRRI	8TRCR	0.99	0.96	0.98
182	8TRRI	8TRMI	0.99	0.96	0.98
183	8TRRI	8TRRO	0.99	0.95	0.97
184	8TRRI	8TRGB	0.98	0.93	0.95
185	8TRRO	8TROA	0.94	0.86	0.87
186	8TRRO	8TRCR	0.97	0.91	0.93
187	8TRRO	8TRMI	0.97	0.92	0.94
188	8TRRO	8TRRI	0.99	0.94	0.97
189	8TRRO	8TRGB	1.00	0.96	0.99
190	8TRGB	8TROA	0.93	0.82	0.85
191	8TRGB	8TRCR	0.96	0.88	0.91
192	8TRGB	8TRMI	0.96	0.88	0.91
193	8TRGB	8TRRI	0.98	0.91	0.95
194	8TRGB	8TRRO	1.00	0.95	0.99

As a result of the comparative evaluation of both methods, the performances by both methods are very similar if statistical parameters have relatively strong linear relationship with regional variables like the Sabine and Neches WAM. However, if not, the proposed method should enhance the performance of the regional statistical method like the GSA and Trinity WAM.

CHAPTER VI

DISAGGREGATION OF MONTHLY TO DAILY NATURALIZED FLOWS USING THE SWAT MODEL

The Soil and Water Assessment Tool (SWAT) is introduced in Chapter I and applied in Chapter IV to synthesize monthly naturalized flows for periods of missing data. Methods for compiling SWAT input data and calibrating parameters are described in Chapter IV. SWAT is applied in the present Chapter VI for the purpose of disaggregating monthly naturalized flows to daily.

6.1 Watershed Delineation

The SWAT rainfall-runoff model is applied to develop sequences of daily streamflow, representing natural conditions without water resource development and use, for periods-of-analysis at control points of interest in the three river basins. These sequences of daily streamflow provide daily flow patterns for disaggregating the monthly WAM naturalized flows to daily. The SIMD simulation model converts monthly naturalized flows to daily based on these daily flow patterns hydrographs while preserving the monthly volumes.

The SWAT models for the Sabine, Neches, and GSA River Basins are developed, including all control points of interest that are distributed from headwater to downstream. In general, the SWAT automatically delineates multiple subwatersheds for a river basin, but the locations of pertinent control points are manually assigned in the delineated models with their exact coordinates from the GIS data in WAM datasets.

Measured daily rainfall data, described in Chapter IV, are adopted for the SWAT models of the three river basins, and other climate data are also generated within the SWAT model by its weather generator. The three SWAT models initially develop monthly and daily flow sequences for certain periods for the model calibrations. Each SWAT model has warm-up simulation periods, 1938-1939 (2 years) for the Sabine and Neches River Basins and 1930-1933 (4 years) for GSA River Basins, respectively.

6.1.1 Sabine River Basin

The SWAT model for the Sabine River Basin is developed, including eighteen primary control points and three secondary control points that are tabulated in Table 6.1. Two secondary control points (E4642A and E4658A) are included for the sites of dams, and another is the site of Senate Bill 3 (SB3) environmental flow standards. Environmental flow standards have been established through the SB3 process at five USGS gaging stations (Wurbs *et al.*, 2014a), listed in Table 6.2. The delineated basin has 98 subwatersheds and covers the 21 interested control points, as shown in Figure 6.1. The model has daily rainfall data covering from 1938 to 2013. The period-of-analysis of the version of the daily Sabine WAM used in this research extends from January 1, 1940 through December 31, 2013 (Wurbs *et al.*, 2014a).

Table 6.1 Secondary Control Points

Control Point	Location	Gage Number	Area (mile ²)	Period-of-Record
<u>Large Reservoirs</u>				
E4642A	Cherokee Bayou at Cherokee Dam		158.5	
E4658A	Sabine River at Toledo Bend Dam		7,199	
<u>SB-3 Environmental Flow Standards</u>				
29500	Big Cow Creek near Newton, TX	8029500	128	5/52 to present

Table 6.2 Sabine WAM Control Point Locations for SB3 Environmental Flow Standards

Control Points	USGS Gage	Location	Watershed Area (sq. miles)	Period-of-Record
BSBS	08019500	Big Sandy Creek near Big Sandy	231	1939-present
SRGW	08020000	Sabine River near Gladewater	2,791	1932-present
SRBE	08022040	Sabine River near Beckville	3,589	1938-present
29500	08029500	Big Cow Creek near Newton	128	1952-present
SRRL	08030500	Sabine River near Ruliff	9,329	1924-present

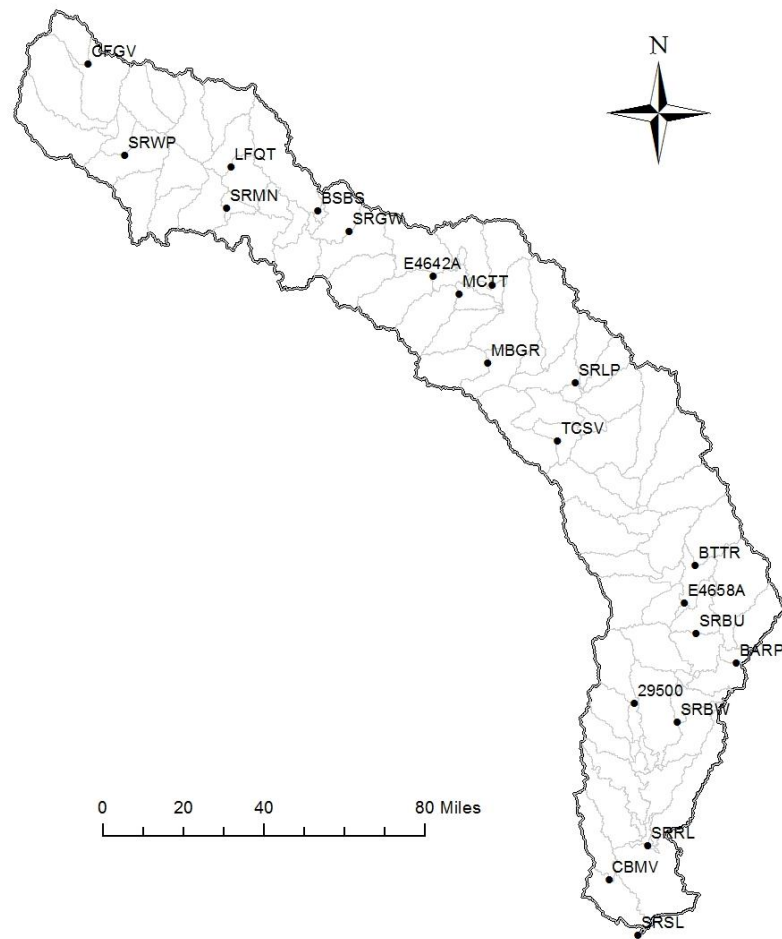


Figure 6.1 Model Delineation for the Sabine River Basin

6.1.2 Neches River Basin

The SWAT model for the Neches River Basin includes twenty primary control points. Environmental flow standards have been established through the SB3 process at five USGS gaging stations (Wurbs *et al.*, 2014b), listed in Table 6.3. The delineated basin has 123 sub-watersheds and covers the 20 pertinent control points, shown in Figure 6.2.

The model has daily rainfall data for 1938 to 2013 which covers the complete period-of-analysis of the Neches WAM that recently extended to cover from January 1, 1940 through December 31, 2013 (Wurbs *et al.*, 2014b).

Table 6.3 Neches WAM Control Point Locations for SB3 Environmental Flow Standards

Control Point	USGS Gage No.	Location	Watershed Area (square miles)	Period-of-Record
NENE	08032000	Neches River at Neches	1,145	1939-present
NERO	08033500	Neches River near Rockland	3,636	1903-present
ANAL	08036500	Angelina River near Alto	1,276	1940-present
NEEV	08041000	Neches River at Evadale	7,951	1904-present
VIKO	08041500	Village Creek near Kountze	860	1924-present



Figure 6.2 Model Delineation for the Neches River Basin

6.1.3 GSA River Basins

The SWAT model for the GSA River Basins was developed with 38 of the total of 46 primary control points and two secondary control points as listed in Table 6.4. Eight primary control points are excluded for two different reasons: CP75 (San Marcos Springs) is a spring water source, and seven others have relatively small watershed areas. Two secondary control points (C384611 and P382411) incorporating Senate Bill 3 (SB3) environmental flow standards are included. Environmental flow standards have been established through the SB3 process at fifteen USGS gaging stations in the GSA River Basins (Wurbs *et al.*, 2014c) as listed in Table 6.5. The delineated basin has 132 sub-watersheds and covers the 40 selected control points, as shown in Figure 6.3. The model has daily rainfall data cover from 1930 to 2013 for the period-of-analysis of the GSA WAM that recently extended from January 1, 1934 through December 31, 2013 (Wurbs *et al.*, 2014c).

Table 6.4 Thirty Eight Primary and Two Selected Secondary Control Points in the GSA WAM

CP	USGS Gage	Location	Drainage Area	Period-of-Record
<u>Guadalupe River Basin</u>			(sq. mile)	
CP01	08167000	Guadalupe River at Comfort	838	31 May 1939 – present
CP02	08167500	Guadalupe River near Spring Branch	1,315	1 Jan 1934 – present
CP03	08167800	Guadalupe River at Canyon Lake	1,432	1 Mar 1960 – present
CP04	08168500	Guadalupe River at New Braunfels	1,519	19 Dec 1927 – present
CP05	08169000	Comal River at New Braunfels	130	19 Dec 1927 – present
CP06	–	Guadalupe River at Lake Wood	2,103	–
CP08	08171000	Blanco River at Wimberley	355	1 Jan 1934 – present
CP09	08171300	Blanco River near Kyle	412	29 May 1956 – present
CP10	08172000	San Marcos River at Luling	839	18 Apr 1939 – present
CP11	08173000	Plum Creek near Luling	311	1 Jan 1934 – present
C384611	08173900	Guadalupe River near Gonzales	3,490	1 Oct 1996 – present
CP12	08174600	Peach Creek below Dilworth	460	1 Aug 1959 – present
CP13	08175000	Sandies Creek near Westhoff	549	1 Jan 1934 – present
CP14	08175800	Guadalupe River at Cuero	4,935	1 Jan 1964 – present
CP15	08176500	Guadalupe River at Victoria	5,196	4 Nov 1934 – present
CP16	08177400	Coleto Creek Reservoir near Victoria	493	storage 1986-2002
CP38	08188800	Guadalupe River near Tivoli	10,122	4 Aug 2000 – present
CPEST	–	Guadalupe Estuary	10,122	–

Table 6.4 (Continued)

CP	USGS Gage	Location	Drainage Area (sq. mile)	Period-of-Record
<i>San Antonio River Basin</i>				
CP17	—	Olmos Creek at Edwards	8	—
CP18	08178000	San Antonio River at San Antonio	44	1 Mar 1939 – present
CP19	08178700	Salado Creek at San Ant Upper Station	136	7 May 1997 – 5 Dec 2006
CP20	08178800	Salado Creek at San Ant Lower Station	187	1 Sep 1960 – present
P38241 1	08178880	Medina River at Bandera	328	1 Oct 1982 – present
CP21	08179500	Medina Lake	634	storage 1997-present
CP23	08180500	Medina River near Rio Medina	649	Oct 1923 – 10 Oct 2007
CP25	—	San Geronimo Creek at Edwards	58	—
CP261	—	Leon Creek at Edwards	60	—
CP262	—	Helotes Creek at Edwards	28	—
CP263	—	Government Creek at Edwards	12	—
CP27	08180800	Medina River near Somerset	962	Oct 1970 – 9 Sep 2004
CP28	08181500	Medina River at San Antonio	1,310	27 Jul 1939 – present
CP29	08181800	San Antonio River near Elmendorf	1,737	1 Oct 1962 – present
CP30	—	Braunig Lake	9	—
CP31	08182500	Calaveras Creek near Elmendorf	65	1 Oct 54 – 30 Sep 1971
CP32	08183500	San Antonio River near Falls City	2,108	1 May 1925 – present
CP33	08183900	Cibolo Creek near Boerne	68	1 Mar 1962 – present
CP34	08185000	Cibolo Creek at Selma	274	1 Apr 1946 – present
CP35	08186000	Cibolo Creek near Falls City	825	1 Oct 1930 – present
CP36	08186500	Ecletto Creek near Runge	239	1 Apr 1962 – present
CP37	08188500	San Antonio River at Goliad	3,906	1 Mar 1939 – present

Table 6.5 GSA WAM Control Point Locations for SB3 Environmental Flow Standards

No.	CP	Basin	USGS	Location	Drainage Area (sq. miles)
1	CP01	G	08167000	Guadalupe River at Comfort	838
2	CP02	G	08167500	Guadalupe River near Spring Branch	1,315
3	CP08	G	08171000	Blanco River at Wimberley	355
4	CP10	G	08172000	San Marcos River at Luling	839
5	CP11	G	08173000	Plum Creek near Luling	311
6	C384611	G	08173900	Guadalupe River at Gonzales	3,469
7	CP13	G	08175000	Sandies Creek near Westhoff	549
8	CP14	G	08175800	Guadalupe River at Cuero	4,935
9	CP15	G	08176500	Guadalupe River at Victoria	5,196
10	P382411	SA	08178880	Medina River at Bandera	328
11	CP28	SA	08181500	Medina River at San Antonio	1,310
12	CP29	SA	08181800	San Antonio River near Elmendorf	1,737
13	CP32	SA	08183500	San Antonio River near Falls City	2,108
14	CP35	SA	08186000	Cibolo Creek near Falls City	825
15	CP37	SA	08188500	San Antonio River at Goliad	3,906

Note) G: Guadalupe River Basin, and SA: San Antonio River Basin

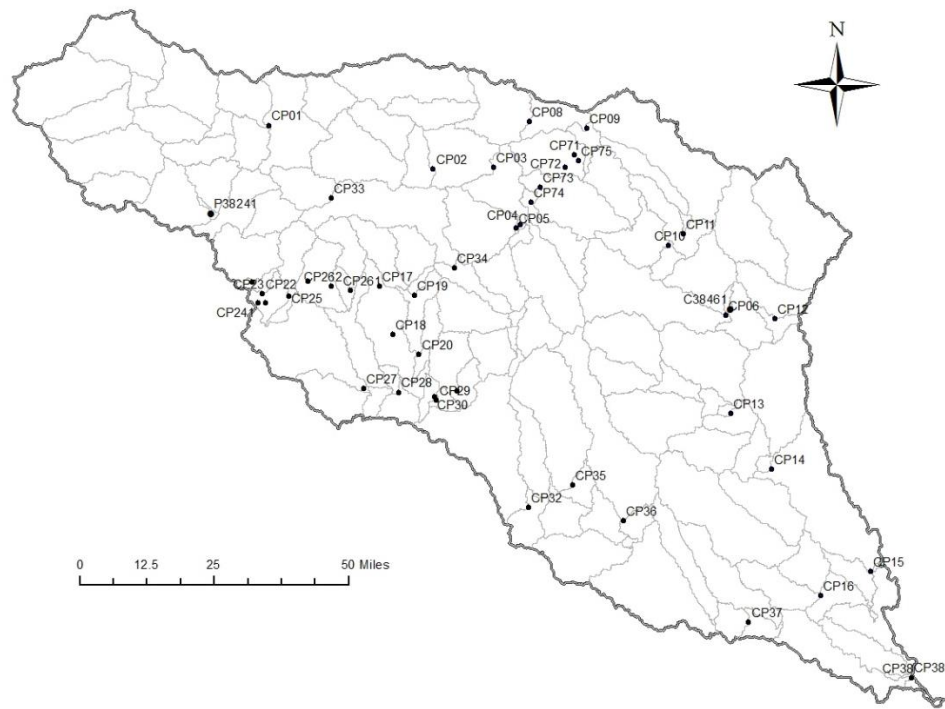


Figure 6.3 Model Delineation for the GSA River Basins

6.2 Model Calibration

This research investigates two different calibration strategies: calibrations with monthly and daily flow sequences. The calibrations are performed at multi-sites by the SWAT-CUP model to obtain the calibrated parameters with spatial consistency such as routing effects between up and downstream sites within a river basin. Coefficient of determination (R^2) is commonly selected as the objective function. This efficiency criterion has a major advantage that ensures the objective function is not governed by a single or a few badly simulated sites in multisite calibration (Arnold *et al.*, 2012b).

In both the model calibrations, the selected parameters are applied to all watersheds in the Sabine and Neches River Basins. However, the GSA River Basins consist of two major river basins, Guadalupe and San Antonio Rivers. These have also varying land cover and a major aquifer (Edward aquifer). Thus, the selected parameters are applied to 5 representative regions, divided by major land covers and Edward aquifer

area in the GSA River Basins. The five regions consist of forest, grass and pasture areas in both river basins, and urban areas in the San Antonio River Basin.

6.2.1 Calibration with Monthly Naturalized Flows

The representative control points are selected for multisite calibration. The sites for calibration are selected considering spatial distribution of control points from headwater to downstream within a river basin. The five control points for the Sabine WAM, the six control points for the Neches WAM, and thirteen control points for the GSA WAM are selected for multisite calibrations, respectively. Table 6.6 lists the control points, selected for multisite calibration for the three river basins, and their locations are referred to as shown in Figures 6.1, 6.2, and 6.3, respectively.

The calibration periods for the three river basins are the same as their original periods-of-analysis as listed in Table 6.6. The validations are not necessary because these calibrations are for developing the calibrated models to synthesize daily flow sequences at all relevant points for their periods-of-analysis.

Table 6.6 summarizes the calibration results for the three river basins. The calibrated models for synthesizing daily flow sequences at all relevant sites in the three river basins are eventually prepared through the modification of the parameters, obtained from the model calibrations on the original models.

6.2.2 Calibration with Daily USGS Recorded Flows

For developing a calibrated daily SWAT model, it is pivotal to obtain daily recorded flow data, considered under naturalized conditions without any human influences, at a relevant site. In reality, human activities such as diversion of water from a river, dam projects, and navigation structures generally impact river flow regimes, and these tend to increase over time. It is practically impossible to obtain daily pure naturalized flow records for a certain period from USGS gauges, even though the gauge has daily flow records for a long period.

Table 6.6 Monthly Calibration Results of SWAT Models

Control Points	Coefficient of Determination (R^2) (Naturalized vs. SWAT Flows)	
	Without Calibration	With Calibration
<u>Sabine River Basin (Calibration Period: 1940 to 1998, 59 years)</u>		
LFQT	0.45	0.47
SRMN	0.69	0.77
SRLP	0.62	0.72
SRBU	0.65	0.72
SRSL	0.69	0.72
<u>Neches River Basin (Calibration Period: 1940 to 1996, 57 years)</u>		
NENE	0.60	0.68
ANAL	0.63	0.74
ANSR	0.65	0.73
NERO	0.70	0.81
NEEV	0.74	0.82
NESL	0.74	0.84
<u>GSA River Basins (Calibration Period: 1934 to 1989, 56 years)</u>		
CP02	0.63	0.74
CP04	0.63	0.72
CP06	0.64	0.72
CP10	0.70	0.72
CP13	0.51	0.55
CP14	0.68	0.79
CP15	0.68	0.80
CP23	0.26	0.28
CP32	0.46	0.51
CP34	0.42	0.53
CP35	0.59	0.63
CP36	0.44	0.44
CP37	0.60	0.74

Many researchers have found that dam projects are the human influence with greatest impact on changing flow regimes of rivers. Therefore, the USGS recorded flow data prior to construction of dam projects can be considered to approximate daily naturalized flow sequences, if all other human influences are disregarded.

The Sabine River Basin has 13 major reservoirs. Toledo Bend, Lake Tawakoni, and Lake Fork are considered the reservoirs that most significantly influence the river flows. Toledo Bend was initially impounded in October, 1966, Lake Tawakoni was initially impounded in October, 1960, and Lake Fork was initially impounded in July, 1979. Thus, it can be considered that the recorded flows prior to 1960 were nearly under naturalized conditions. The six control points have the daily recorded flows from 1940 to 1960 without missing data. The four control points of the six control points in total are selected for multisite calibration considering their spatial distribution as listed in Table 6.7.

There are 11 major reservoirs in the Neches River Basin. Sam Rayburn and Palestine reservoirs considerably impact the river flow regimes. The initial impoundation years of Sam Rayburn and Palestine were 1965 and 1962, respectively. Therefore, the recorded flows prior to 1960 are considered practically daily naturalized flows. The seven control points have the daily recorded flows from 1940 to 1960 without missing data. The three control points of the seven control points in total are selected for multisite calibration considering their spatial distribution as listed in Table 6.7.

The GSA River Basins have 9 major reservoirs. Canyon Lake in the Guadalupe River Basin and Medina Lake in the San Antonio River Basin are considered the reservoirs that should remarkably change the river flow regimes. Canyon Lake was initially impounded in 1964, and located upstream of the Guadalupe River. Medina Lake, located upstream of the San Antonio River, has been operated since 1913. Thus, recorded flows prior to 1960 in the Guadalupe River can be considered daily naturalized flows, but Medina Lake has changed flow regimes along the San Antonio River since 1913. There are 9 control points that have the daily recorded flows prior to 1960 in the Guadalupe River Basin. The two control points of the nine control points in total are selected for multisite calibration, considering their spatial distribution and spring water impacts as listed in Table 6.7. The San Antonio River Basin does not have the control points that have the daily recorded flows prior to 1913. However, there are 7 control points that have the daily recorded flows prior to 1960 like the Guadalupe River Basin. Of the 7 control points, three control points are practically selected for multisite calibration as listed in Table 6.7.

Medina Lake cannot impact the flow regimes at Control point CP35, because it is located at a tributary. Even though Medina Lake should influence on the flow regimes at Control point CP32 and CP37, but these influences should be minimized at the two control points.

The SWAT model should have spatially and temporally well-distributed rainfall data covering a basin area and drought and flood years for calibration and validation periods. Calibration and validation periods are determined as listed in Table 6.7 according to two main reasons: (1) the SWAT models have recorded daily rainfall data covering the periods-of-analysis, but there are a number of rainfall gauges with missing data prior to 1950. Therefore, the calibration and validation periods should be determined after 1950, and (2) the calibration period for 1950-1955 (5 years) includes both a serious drought and regular flood years, and the validation period includes both a regular drought and flood years.

Table 6.7 summarizes the calibration and validation results for the three river basins. The Sabine and Neches River Basins show relatively better results, and the GSA River Basins show worse results than the two river basins. Although the values of R^2 for the validation periods would decrease than the calibration periods at some control points, all the values of R^2 for both the periods are acceptable as listed in Table 6.7. Therefore, the three calibrated daily models are eventually developed to generate daily flow sequences at all relevant sites, if the parameters on the original models are replaced with the calibrated parameters.

6.3 Disaggregation of Monthly to Daily Naturalized Flows

SWAT models calibrated by the two calibration strategies generate two different daily flow sequences for the periods-of-analysis for the Sabine, Neches, and GSA River Basin, respectively. In other words, each WAM eventually has the two different daily flow patterns covering the period-of-analysis, generated by calibrated SWAT models based on the monthly naturalized flows and daily USGS recorded flows.

Table 6.7 Daily Calibration Results of SWAT Models

Control Points	Coefficient of Determination (R^2) (Naturalized vs. SWAT flows)		
	Without Calibration (1950-1955)	With Calibration (1950-1955)	Validation (1956-1960)
<u>Sabine River Basin</u>			
LFQT	0.10	0.37	0.60
SRBW	0.10	0.78	0.62
SRMN	0.06	0.64	0.73
SRBE	0.10	0.64	0.77
<u>Neches River</u>			
ANLU	0.11	0.84	0.77
NERO	0.17	0.87	0.76
NEEV	0.10	0.90	0.76
<u>GSA River</u>			
CP04	0.60	0.87	0.36
CP15	0.02	0.59	0.67
CP32	0.07	0.34	0.39
CP35	0.38	0.79	0.36
CP37	0.06	0.74	0.47

For disaggregation of monthly to daily naturalized flows, monthly naturalized flows data at the relevant control points are firstly extracted from each output of monthly WRAP (SIM) model using the OI record in the HIN file as described in the WRAP Hydrology Manual (Wurbs 2013b). Table 6.8 reproduces the HIN file for the Sabine WAM as an example.

Table 6.8 OI Records in the HIN file

```

** Monthly naturalized flow records from Sabine3M OUT file.
JC  1940  73   1   7
OI   1    2  IN   1   29500   29500
OI   1    2  IN   1    CFGV   CFGV
OI   1    2  IN   1    SRWP   SRWP
OI   1    2  IN   1    SRMN   SRMN
OI   1    2  IN   1    LFQT   LFQT
OI   1    2  IN   1    BSBS   BSBS
OI   1    2  IN   1  E4642A  E4642A
OI   1    2  IN   1  E4658A  E4658A
OI   1    2  IN   1    SRBE   SRBE
OI   1    2  IN   1    MCTT   MCTT
OI   1    2  IN   1    MBGR   MBGR
OI   1    2  IN   1    SRLP   SRLP
OI   1    2  IN   1    BTTR   BTTR
OI   1    2  IN   1    SRBU   SRBU
OI   1    2  IN   1    BARP   BARP
OI   1    2  IN   1    SRBW   SRBW
OI   1    2  IN   1    SRRL   SRRL
OI   1    2  IN   1    CBMV   CBMV
OI   1    2  IN   1    SRSL   SRSL
OI   1    2  IN   1    SRGW   SRGW
OI   1    2  IN   1    TCSV   TCSV
ED

```

The OI records in the HIN file extract monthly naturalized flow sequences for 1940-2013 at 21 control points stored in the FLO file with IN record for the Sabine WAM, 1940-2013 sequences of naturalized flows at 20 primary control points stored in the FLO file with IN records for the Neches WAM, and monthly naturalized flow sequences for the period from 1934 to 2013 at 40 control points stored in the FLO file with IN record for the GSA WAM.

JOBDIS and DFLWS records in the DIN file disaggregate monthly flow sequence stored in the FLO file to daily flow sequence based on daily flow sequence stored in the DCF file as described in the WRAP Daily Manual (Wurbs and Hoffpauir, 2013b). Table 6.9 reproduces a DIN, FLO, and DCF files, respectively, as examples.

Table 6.9 Examples of DIN, FLO, and DCF files

```

JOBDIS      0      4                                2  29500
DFlOWS      1940    1    2013    12    1940    1    2013    12    2    29500    29500
END

```

a) An Example of DIN file

```

1940  1  1    17.83
1940  1  2    13.70
1940  1  3    10.88
1940  1  4     9.46
1940  1  5     8.16

```

b) An Example of DCF file

```

IN 29500    1940 27907.0 16239.1 7996.5    0.0 15978.2 2513.1 9179.7 52615.0 4962.7 3426.9 14294.8 73577.1
IN 29500    1941 41934.0 12396.3 24857.1 14335.0 11272.6 32306.8 38565.9 9194.7 9957.8 4071.6 15118.0 8769.5
IN 29500    1942 17835.1 9818.5 24258.1 24546.4 3753.5 21834.5 17842.4 10464.3 14700.4 4355.9 3832.9 4327.6
IN 29500    1943 16592.2 8898.8 10089.5 10705.9 5031.1    0.0 25255.9 8249.7 4348.3 2583.9 4487.0 8186.9
IN 29500    1944 29612.3 27949.4 27417.4 33099.7 59159.9 42274.5 7934.7 4744.4 7158.9 4880.5 6474.4 23862.8

```

c) An Example of FLO file

6.4 Comparative Evaluation of Disaggregated Daily Flows

WAM monthly naturalized flows are disaggregated based on SWAT generated daily pattern hydrographs while maintaining the monthly volumes. Even though daily flow sequences disaggregated from WAM monthly naturalized flows are not perfectly identical to the daily naturalized flow sequences generated with SWAT, they should have similar flow timing, flow regimes, and hydrological characteristics. The four different previously described methods are applied to evaluate two different disaggregated daily flow sequences: (1) disaggregated based daily flow pattern that is generated by the daily SWAT model calibrated with monthly naturalized flows (called “MDNF”), and (2) disaggregated based daily flow pattern that is generated by the daily SWAT model calibrated with USGS daily recorded flows (called “DDNF”), comparing with USGS daily recorded flows for the periods that are considered under practically naturalized conditions mentioned in the Section 6.2.2. The higher scored calibration strategy, summed up from the four different

evaluation methods, is finally selected as the strategy to calibrate the daily SWAT model for synthesizing daily flow sequences.

The daily recorded flows at 6 gauges for the period from 1940 to 1960 for the Sabine WAM, at 7 gauges for the period from 1940 to 1960 for the Neches WAM, and 12 gauges for the period from different beginning years to 1960 for the GSA WAM are selected for the comparative evaluations for the reasons discussed in Section 6.2.2.

6.4.1 Sabine River Basin

NSE values of MDNF and DDNF are computed based on the USGS daily recorded flows for 21 years at 6 control points, and the comparative scores are made through the comparison of both NSE at a control point as listed in Table 6.10. NSE values of DDNF are higher at 3 control points, and lower at 2 control points than MDNF. Both NSE values are not acceptable levels at BSBS, so both performances are considered as the same. Total comparative scores are 5 for MDNF, and 7 for DDNF.

Table 6.10 Comparative Evaluation of Nash-Sutcliffe Efficiency for the Sabine WAM

CP	Data Period	NSE		Comparative Score	
		USGS vs. MDNF	USGS vs. DDNF	USGS vs. MDNF	USGS vs. DDNF
<u>Total</u>				<u>5</u>	<u>7</u>
BSBS	01/01/1940-12/31/1960	-0.28	-0.37	1	1
LFQT	01/01/1940-12/31/1960	0.28	0.33	0	2
SRBE	01/01/1940-12/31/1960	0.76	0.75	2	0
SRGW	01/01/1940-12/31/1960	0.74	0.69	2	0
SRRL	01/01/1940-12/31/1960	0.73	0.83	0	2
SRBW	01/01/1940-12/31/1960	0.79	0.86	0	2

The three different flow frequency metrics of USGS daily recorded flows, MDNF, and DDNF at 6 control points are tabulated in Tables 6.11 and 6.12. Both mean flows of MDNF and DDNF are very similar to the mean flows of USGS daily recorded flows at 6 control points. Both median (50% exceedance frequency) values of the MDCF and DDNF are also closely similar to the median values of USGS daily recorded flows at 6 control

points. Maximum values of USGS daily recorded flows are much higher than both the value of MDNF and DDNF at 6 control points. Flow frequency metrics indicate that maximum values of MDNF are mostly higher than the value of DDNF, but minimum values of DDNF are more similar to the value of USGS daily recorded flows than MDNF.

Table 6.11 Flow Frequency Metrics for Control Points BSBS, LFQT, and SRBE

	BSBS (CFS)			LFQT (CFS)			SRBE (CFS)		
	USGS	MDNF	DDNF	USGS	MDNF	DDNF	USGS	MDNF	DDNF
Mean	203.2	202.4	202.4	480.7	480.6	480.6	2,789	2,883	2,883
Std Dev	479.4	404.2	426.6	1,921	1,335	1,197	5,430	5,040	4,814
Min	5.0	0.0	0.0	0.0	0.0	0.0	7.0	0.0	1.1
Max	17,900	7,028	8,535	70,508	26,755	21,526	120,000	60,570	51,732
0.1%	6,033	4,912	5,237	22,651	16,762	14,587	65,059	52,139	45,049
0.2%	4,560	3,742	3,812	17,258	12,342	10,491	49,125	43,774	40,609
0.5%	2,879	2,767	2,723	12,429	9,504	7,486	33,420	32,157	31,710
1%	1,870	1,939	1,990	7,723	6,908	5,879	24,528	25,922	24,869
2%	1,236	1,339	1,351	4,776	4,413	4,235	17,000	18,306	18,426
5%	684.4	769.9	813	2,140	2,120	2,213	10,300	11,105	11,091
10%	429.0	514.7	518	996.0	1,179.0	1,266	7,040	7,378	7,294
15%	309.2	361.4	348.5	584.0	794.0	851.0	5,372	5,400	5,426
20%	246.0	267.0	254.9	360.0	555.0	593.0	4,260	4,114	4,168
30%	176.0	167.2	161.8	154.0	267.0	298.0	2,554	2,732	2,809
40%	128.0	109.8	103.4	80.0	129.0	144.0	1,620	1,727	1,765
50%	87.0	73.6	68.7	40.0	58.0	69.0	984.0	1,081	1,119
60%	61.0	50.3	45.4	20.0	23.0	30.0	600.0	719.0	745.0
70%	42.0	31.6	30.3	8.0	11.0	14.0	378.0	429.0	450.0
80%	30.0	17.8	16.9	2.0	3.0	4.0	210.0	225.0	254.0
85%	24.0	12.0	12.0	1.0	1.0	2.0	149.0	148.0	179.0
90%	20.0	6.2	7.1	0.0	0.0	0.0	96.0	95.0	115.0
95%	15.0	0.2	1.8	0.0	0.0	0.0	54.0	41.0	68.0
98%	10.0	0.0	0.0	0.0	0.0	0.0	20.0	7.0	20.0
99%	8.0	0.0	0.0	0.0	0.0	0.0	14.0	0.0	10.0
99.5%	7.0	0.0	0.0	0.0	0.0	0.0	12.0	0.0	5.0
99.8%	6.0	0.0	0.0	0.0	0.0	0.0	10.0	0.0	2.0
99.9%	6.0	0.0	0.0	0.0	0.0	0.0	10.0	0.0	1.0

Table 6.12 Flow Frequency Metrics for Control Points SRBW, SRGW and SRRL

	SRBW (CFS)			SRGW (CFS)			SRRL (CFS)		
	USGS	MDNF	DDNF	USGS	MDNF	DDNF	USGS	MDNF	DDNF
Mean	7,596	7,615	7,615	2,230	2,244	2,244	9,659	9,678	9,678
Std Dev	9,818	9,897	9,456	5,242	4,435	4,158	11,617	11,843	11,186
Min	160.0	77.6	121.0	6.1	0.0	2.0	270.0	139.4	211
Max	111,012	108,730	117,262	133,000	63,054	55,313	120,000	132,360	109,083
0.1%	75,508	71,717	75,215	58,056	42,297	36,338	84,058	92,341	86,341
0.2%	68,692	68,777	64,804	48,631	36,978	32,341	77,481	86,344	78,246
0.5%	52,070	57,417	55,272	36,012	29,576	28,043	62,720	71,531	63,437
1%	43,461	49,409	45,524	22,828	22,671	21,681	52,900	57,050	53,501
2%	36,604	37,682	36,164	15,956	16,322	15,888	43,900	46,311	43,976
5%	27,243	27,029	25,707	8,588	9,308	9,290	33,100	32,497	31,126
10%	20,502	19,094	18,980	5,650	5,543	5,675	24,200	22,804	22,879
15%	16,302	14,869	14,963	4,040	3,915	4,028	19,700	18,467	18,438
20%	12,802	12,234	12,636	2,940	3,020	3,160	16,200	15,580	15,854
30%	8,661	8,397	8,636	1,730	1,926	1,995	11,500	11,021	11,381
40%	5,531	5,869	6,135	1,010	1,197	1,244	7,580	7,853	8,142
50%	3,480	3,829	4,012	610.0	710.0	758.0	4,790	5,274	5,520
60%	2,210	2,622	2,736	372.0	434.0	460.0	3,280	3,723	3,843
70%	1,480	1,703	1,831	233.0	237.0	264.0	2,340	2,614	2,804
80%	1,000	1,081	1,173	126.0	116.0	139.0	1,590	1,713	1,869
85%	760.0	804.0	880.0	90.0	78.0	92.0	1,300	1,348	1,464
90%	552.0	558.0	597.0	57.0	47.0	55.0	968.0	987.0	1,009
95%	390.0	311.0	356.0	33.0	18.0	28.0	590.0	604.0	629.0
98%	246.0	201.0	236.0	14.0	4.0	14.0	390.0	357.0	378.0
99%	213.0	167.0	188.0	11.0	0.0	9.0	332.0	281.0	320.0
99.5%	188.0	130.0	158.0	9.0	0.0	5.0	299.0	229.0	264.0
99.8%	179.0	103.0	140.0	8.0	0.0	4.0	277.0	188.0	240.0
99.9%	168.0	96.0	136.0	7.0	0.0	3.0	270.0	161.0	225.0

Table 6.13 summarizes the comparative scores, made through the qualitative comparison of both flow frequency metrics at a control point based on the USGS flows.

Table 6.13 Comparative Evaluation of Flow Frequency Metrics for the Sabine WAM

CP	Data Period	Comparative Score	
		USGS vs. MDNF	USGS vs. DDNF
<u>Total</u>		<u>6</u>	<u>6</u>
BSBS	01/01/1940-12/31/1960	1	1
LFQT	01/01/1940-12/31/1960	1	1
SRBE	01/01/1940-12/31/1960	1	1
SRGW	01/01/1940-12/31/1960	1	1
SRRL	01/01/1940-12/31/1960	1	1
SRBW	01/01/1940-12/31/1960	1	1

The DHRAM method quantitatively evaluates how much the hydrologic characteristics of MDNF and DDNF are similar to or different from USGS daily recorded flows at the 6 control points. The score ranges from 0 (almost identical) to 30 (totally different). Table 6.14 summarizes the scores of MDNF and DDNF based on the USGS daily recorded flows at the six points. Table 6.14 indicates that both MDNF and DDNF have nearly similar hydrological characteristics to USGS daily recorded flows at the 5 control points except for BSBS. At BSBS, DDNF has less impact points than MDNF. This means hydrological characteristics of DDNF are relatively more identical to USGS daily recorded flows. The detailed evaluation sheets for the six control points are tabulated in Tables 6.15 to 6.20.

Table 6.14 Impact Points by the DHRAM Method for the Sabine WAM

CP	Data Period	Impact Points	
		USGS vs. MDNF	USGS vs. DDNF
<u>Total</u>		<u>14</u>	<u>8</u>
BSBS	01/01/1940-12/31/1960	9	5
LFQT	01/01/1940-12/31/1960	2	2
SRBE	01/01/1940-12/31/1960	1	0
SRGW	01/01/1940-12/31/1960	0	0
SRRL	01/01/1940-12/31/1960	1	0
SRBW	01/01/1940-12/31/1960	1	1

Table 6.15 indicates that the daily maximum and minimum flows of NDMF and DDMF are similar to the USGS daily recorded flows according to the results of parameter group #2, but the flow timings of both MDNF and DDNF are slightly different from the USGS daily recorded flows according to the results of parameter group #3.

Table 6.15 Evaluation Sheet by DHRAM Method for Control Point BSBS

Control Point:	BSBS		Period:	1940-1960						
IHA statistics group	USGS		MDNF		Absolute Chages		DDNF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	267.9	0.7853	267.8	0.7857	0.0%	0.1%	267.8	0.7857	0.0%	0.1%
February	291.9	0.7031	291.8	0.7034	0.0%	0.0%	291.8	0.7034	0.0%	0.0%
March	324.7	1.077	324.6	1.078	0.0%	0.1%	324.6	1.078	0.0%	0.1%
April	371.3	0.8473	371.2	0.8474	0.0%	0.0%	371.2	0.8474	0.0%	0.0%
May	385.2	0.8235	385.1	0.8238	0.0%	0.0%	385.1	0.8238	0.0%	0.0%
June	213.3	1.054	204.8	0.9805	4.0%	7.0%	204.8	0.9805	4.0%	7.0%
July	76.97	0.8591	76.87	0.8608	0.1%	0.2%	76.87	0.8608	0.1%	0.2%
August	40.77	0.6854	40.67	0.6877	0.2%	0.3%	40.67	0.6876	0.2%	0.3%
September	44.86	0.6256	44.76	0.6268	0.2%	0.2%	44.75	0.6268	0.2%	0.2%
October	82.09	1.364	82	1.365	0.1%	0.1%	82	1.365	0.1%	0.1%
November	133.4	1.273	133.3	1.274	0.1%	0.1%	133.3	1.274	0.1%	0.1%
December	211.9	0.7568	211.8	0.7571	0.0%	0.0%	211.8	0.7571	0.0%	0.0%
					Average Score	0.4% 0.7%			Average Score	0.4% 0.7%
						0 0				0 0
Parameter Group #2										
1-day minimum	17.9	0.4326	1.519	1.867	91.5%	331.6%	4.886	1.419	72.7%	228.0%
3-day minimum	18.16	0.4441	2.179	1.76	88.0%	296.3%	5.43	1.387	70.1%	212.3%
7-day minimum	19.18	0.4482	4.259	1.907	77.8%	325.5%	6.416	1.268	66.5%	182.9%
30-day minimum	23.29	0.4707	16.87	0.9111	27.6%	93.6%	19.03	0.7465	18.3%	58.6%
90-day minimum	42.24	0.6288	38.91	0.6266	7.9%	0.3%	40.05	0.6484	5.2%	3.1%
1-day maximum	4100	0.9947	2652	0.6787	35.3%	31.8%	2817	0.7082	31.3%	28.8%
3-day maximum	3100	0.8816	2290	0.7188	26.1%	18.5%	2423	0.754	21.8%	14.5%
7-day maximum	1979	0.8089	1654	0.7392	16.4%	8.6%	1710	0.7533	13.6%	6.9%
30-day maximum	778.9	0.6413	744.5	0.5676	4.4%	11.5%	737.7	0.5641	5.3%	12.0%
90-day maximum	436.2	0.5453	430.9	0.5309	1.2%	2.6%	432.7	0.5312	0.8%	2.6%
					Average Score	37.6% 112.0%			Average Score	30.6% 75.0%
						0 1				0 0
Parameter Group #3										
Date of minimum	230.3	0.06167	170.8	0.2212	25.8%	258.7%	159.8	0.298	30.6%	383.2%
Date of maximum	123	0.2319	123.1	0.197	0.1%	15.0%	124	0.2546	0.8%	9.8%
					Average Score	13.0% 136.9%			Average Score	15.7% 196.5%
						1 3				1 3
Parameter Group #4										
Low pulse count	4.667	0.5175	9.952	0.4221	113.2%	18.4%	10.05	0.504	115.3%	2.6%
Low pulse duration	23.95	0.8958	9.253	0.5219	61.4%	41.7%	8.456	0.4832	64.7%	46.1%
High pulse count	4.238	0.8372	6.429	0.4656	51.7%	44.4%	6.143	0.5406	45.0%	35.4%
High pulse duration	4.547	0.4597	4.25	0.3725	6.5%	19.0%	4.301	0.3313	5.4%	27.9%
					Average Score	58.2% 30.9%			Average Score	57.6% 28.0%
						1 1				1 0
Parameter Group #5										
Rise rate	90.34	0.7494	118.4	0.689	31.1%	8.1%	135.5	0.6574	50.0%	12.3%
Fall rate	-48.86	-0.7086	-40.79	-0.562	16.5%	20.7%	-41.74	-0.6154	14.6%	13.2%
Number of reversals	75.19	0.09153	96.9	0.2916	28.9%	218.6%	87	0.1808	15.7%	97.5%
					Average Score	25.5% 82.4%			Average Score	26.8% 41.0%
						0 2				0 0
Total Point					9		Total Point		5	
Classification					3		Classification		3	
note:					Moderate risk of impact		note:		Moderate risk of impact	

Table 6.16 indicates that the amount of daily maximum and minimum flows of NDMF and DDMF are almost identical to the USGS daily recorded flows according to the results of parameter group # 2, but the flow timings of both MDNF and DDNF are also slightly different from the USGS daily recorded flows according to the results of parameter group #3.

Table 6.16 Evaluation Sheet by DHRAM Method for Control Point LFQT

Control Point:	LFQT		Period: 1940-1960							
IHA statistics group	USGS		MDNF		Absolute Chages		DDNF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	509.9	1.048	509.8	1.048	0.0%	0.0%	509.8	1.048	0.0%	0.0%
February	707.1	1.147	707	1.148	0.0%	0.1%	707	1.148	0.0%	0.1%
March	765.6	1.73	765.4	1.73	0.0%	0.0%	765.4	1.73	0.0%	0.0%
April	1059	1.155	1059	1.155	0.0%	0.0%	1059	1.155	0.0%	0.0%
May	1067	0.9239	1067	0.9239	0.0%	0.0%	1067	0.9239	0.0%	0.0%
June	541.7	1.456	541.6	1.456	0.0%	0.0%	541.6	1.456	0.0%	0.0%
July	137.6	1.717	137.5	1.717	0.1%	0.0%	137.5	1.717	0.1%	0.0%
August	23.68	1.409	23.62	1.414	0.3%	0.4%	23.62	1.414	0.3%	0.4%
September	46.4	1.234	46.34	1.235	0.1%	0.1%	46.34	1.235	0.1%	0.1%
October	193.9	2.024	193.8	2.025	0.1%	0.0%	193.8	2.025	0.1%	0.0%
November	290.5	2.025	290.3	2.026	0.1%	0.0%	290.3	2.026	0.1%	0.0%
December	449.4	1.3	449.3	1.3	0.0%	0.0%	449.3	1.3	0.0%	0.0%
					Average Score	0.1% 0			Average Score	0.1% 0
Parameter Group #2										
1-day minimum	0.3857	3.723	0.2	2.329	48.1%	37.4%	0.491	2.721	27.3%	26.9%
3-day minimum	0.419	3.653	0.2444	2.197	41.7%	39.9%	0.5848	2.384	39.6%	34.7%
7-day minimum	0.4898	3.365	0.4204	1.829	14.2%	45.6%	0.7693	2.131	57.1%	36.7%
30-day minimum	1.561	1.802	2.057	1.505	31.8%	16.5%	2.701	1.842	73.0%	2.2%
90-day minimum	29.31	1.748	28.75	1.64	1.9%	6.2%	29.92	1.528	2.1%	12.6%
1-day maximum	17550	0.8863	10310	0.6656	41.3%	24.9%	8289	0.6547	52.8%	26.1%
3-day maximum	12510	0.7942	7744	0.667	38.1%	16.0%	6682	0.7048	46.6%	11.3%
7-day maximum	7414	0.775	5671	0.6983	23.5%	9.9%	5223	0.6872	29.6%	11.3%
30-day maximum	2574	0.6791	2517	0.6513	2.2%	4.1%	2482	0.6505	3.6%	4.2%
90-day maximum	1245	0.6593	1218	0.6599	2.2%	0.1%	1215	0.66	2.4%	0.1%
					Average Score	24.5% 20.1% 0 0			Average Score	33.4% 16.6% 0 0
Parameter Group #3										
Date of minimum	226.2	0.09738	205.1	0.1589	9.3%	63.2%	222.4	0.1737	1.7%	78.4%
Date of maximum	158.7	0.3034	132.4	0.2798	16.6%	7.8%	140.9	0.313	11.2%	3.2%
					Average Score	13.0% 35.5% 1 1			Average Score	6.4% 40.8% 0 1
Parameter Group #4										
Low pulse count	3.905	0.4278	5.714	0.4635	46.3%	8.3%	5.238	0.4307	34.1%	0.7%
Low pulse duration	25.01	0.5855	17.22	0.7639	31.1%	30.5%	18.65	0.7648	25.4%	30.6%
High pulse count	4.81	0.5976	4.952	0.5511	3.0%	7.8%	4	0.6423	16.8%	7.5%
High pulse duration	3.202	0.3225	4.266	0.3714	33.2%	15.2%	6.569	0.4252	105.2%	31.8%
					Average Score	28.4% 15.4% 0 0			Average Score	45.4% 17.7% 1 0
Parameter Group #5										
Rise rate	452.2	0.6536	420.4	0.5262	7.0%	19.5%	265.5	0.5348	41.3%	18.2%
Fall rate	-211.8	-0.6301	-128.6	-0.5901	39.3%	6.3%	-93.86	-0.6017	55.7%	4.5%
Number of reversals	61.81	0.2046	73.67	0.1587	19.2%	22.4%	70.57	0.1615	14.2%	21.1%
					Average Score	21.8% 16.1% 0 0			Average Score	37.0% 14.6% 0 0
					Total Point Classification	2 2			Total Point Classification	2 2
					note: Low risk of impact				note: Low risk of impact	

The four other detailed evaluation sheets show that hydrologic characteristics of both NDMF and DDMF are almost identical to the USGS daily recorded flows as listed in Tables 6.17, 6.18, 6.19, and 6.20, respectively.

Table 6.17 Evaluation Sheet by DHRAM Method for Control Point SRBE

Control Point:	SRBE	Period:	1940-1960							
IHA statistics group	USGS		MDNF		Absolute Chages		DDMF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	3241	0.8776	3327	0.8777	2.7%	0.0%	3327	0.8777	2.7%	0.0%
February	3649	0.885	3744	0.8862	2.6%	0.1%	3744	0.8862	2.6%	0.1%
March	3863	0.8817	3971	0.8796	2.8%	0.2%	3971	0.8796	2.8%	0.2%
April	4653	1.296	4786	1.294	2.9%	0.2%	4786	1.294	2.9%	0.2%
May	6831	0.9596	7017	0.958	2.7%	0.2%	7017	0.958	2.7%	0.2%
June	4149	1.127	4277	1.121	3.1%	0.5%	4277	1.121	3.1%	0.5%
July	1074	1.33	1114	1.313	3.7%	1.3%	1114	1.313	3.7%	1.3%
August	455.8	1.224	486.4	1.168	6.7%	4.6%	486.4	1.168	6.7%	4.6%
September	432.9	0.8049	463.1	0.7802	7.0%	3.1%	463.1	0.7802	7.0%	3.1%
October	776.8	1.312	814.9	1.286	4.9%	2.0%	814.9	1.286	4.9%	2.0%
November	1894	1.556	1955	1.546	3.2%	0.6%	1955	1.546	3.2%	0.6%
December	2527	1.075	2714	1.16	7.4%	7.9%	2714	1.16	7.4%	7.9%
				Average Score	4.1%	1.7%		Average Score	4.1%	1.7%
					0	0			0	0
Parameter Group #2										
1-day minimum	80.32	0.6394	24.1	1.226	70.0%	91.7%	57.24	0.8438	28.7%	32.0%
3-day minimum	83.82	0.6599	27.77	1.173	66.9%	77.8%	61.56	0.8239	26.6%	24.9%
7-day minimum	90.89	0.6885	38.99	1.032	57.1%	49.9%	73.94	0.7998	18.6%	16.2%
30-day minimum	149.2	0.8329	136.2	0.7864	8.7%	5.6%	157.7	0.7562	5.7%	9.2%
90-day minimum	379.9	0.8972	396.6	0.8344	4.4%	7.0%	403.3	0.8229	6.2%	8.3%
1-day maximum	29820	0.9388	21970	0.6859	26.3%	26.9%	19980	0.6657	33.0%	29.1%
3-day maximum	28130	0.9338	21440	0.6989	23.8%	25.2%	19600	0.6776	30.3%	27.4%
7-day maximum	23270	0.8652	20000	0.7246	14.1%	16.3%	18420	0.7068	20.8%	18.3%
30-day maximum	11890	0.6502	11810	0.6512	0.7%	0.2%	11680	0.6527	1.8%	0.4%
90-day maximum	6529	0.6023	6692	0.5988	2.5%	0.6%	6671	0.6002	2.2%	0.3%
				Average Score	27.4%	30.1%		Average Score	17.4%	16.6%
					0	0			0	0
Parameter Group #3										
Date of minimum	258.1	0.07018	243.5	0.08581	5.7%	22.3%	260.4	0.07874	0.9%	12.2%
Date of maximum	152.7	0.2417	135.3	0.2111	11.4%	12.7%	147.7	0.2345	3.3%	3.0%
				Average Score	8.5%	17.5%		Average Score	2.1%	7.6%
					1	0			0	0
Parameter Group #4										
Low pulse count	3.762	0.436	4.619	0.3717	22.8%	14.7%	4	0.3708	6.3%	15.0%
Low pulse duration	27.8	0.8351	22.54	0.8268	18.9%	1.0%	25.47	0.7979	8.4%	4.5%
High pulse count	2.381	0.7569	2.857	0.7112	20.0%	6.0%	2.619	0.7961	10.0%	5.2%
High pulse duration	11.78	0.5354	12.96	0.6537	10.0%	22.1%	14.28	0.6199	21.2%	15.8%
				Average Score	17.9%	11.0%		Average Score	11.5%	10.1%
					0	0			0	0
Parameter Group #5										
Rise rate	463.9	0.6815	432.2	0.6254	6.8%	8.2%	369.3	0.5944	20.4%	12.8%
Fall rate	-323.2	-0.6393	-263.1	-0.5724	18.6%	10.5%	-239.7	-0.581	25.8%	9.1%
Number of reversals	61.81	0.1338	58.33	0.1114	5.6%	16.7%	51.9	0.1496	16.0%	11.8%
				Average Score	10.4%	11.8%		Average Score	20.8%	11.2%
					0	0			0	0
				Total Point Classification	1			Total Point Classification	0	
					2				1	
				note:	Low risk of impact			note:	Un-impacted condition	

Table 6.18 Evaluation Sheet by DHRAM Method for Control Point SRBW

Control Point:	SRBW	Data:	1940-1960							
IHA statistics group	USGS		MDNF		Absolute Chages		DDNF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	10830	0.8497	10830	0.8491	0.0%	0.1%	10830	0.8491	0.0%	0.1%
February	12020	0.6104	12040	0.6083	0.2%	0.3%	12040	0.6083	0.2%	0.3%
March	11640	0.5854	11650	0.5834	0.1%	0.3%	11650	0.5834	0.1%	0.3%
April	11380	0.8222	11400	0.8192	0.2%	0.4%	11400	0.8192	0.2%	0.4%
May	15470	0.9463	15460	0.9454	0.1%	0.1%	15460	0.9454	0.1%	0.1%
June	9311	0.9261	9322	0.9239	0.1%	0.2%	9322	0.9239	0.1%	0.2%
July	3691	0.9961	3696	0.9937	0.1%	0.2%	3696	0.9937	0.1%	0.2%
August	1973	0.9887	1973	0.9866	0.0%	0.2%	1973	0.9866	0.0%	0.2%
September	1687	1.295	1711	1.297	1.4%	0.2%	1711	1.297	1.4%	0.2%
October	2031	1.146	2038	1.136	0.3%	0.9%	2038	1.136	0.3%	0.9%
November	4200	1.247	4212	1.24	0.3%	0.6%	4212	1.24	0.3%	0.6%
December	7173	1.184	7298	1.18	1.7%	0.3%	7298	1.18	1.7%	0.3%
				Average Score	0.4%	0.3%		Average Score	0.4%	0.3%
					0	0			0	0
Parameter Group #2										
1-day minimum	521.9	0.4928	333.9	0.7249	36.0%	47.1%	420.2	0.6697	19.5%	35.9%
3-day minimum	533.8	0.5054	346.7	0.6963	35.1%	37.8%	434.1	0.6617	18.7%	30.9%
7-day minimum	556.6	0.5163	378.3	0.6807	32.0%	31.8%	465.2	0.6383	16.4%	23.6%
30-day minimum	743.6	0.6263	639.5	0.5798	14.0%	7.4%	708.8	0.5832	4.7%	6.9%
90-day minimum	1271	0.7443	1293	0.7258	1.7%	2.5%	1327	0.721	4.4%	3.1%
1-day maximum	39520	0.5873	46540	0.5991	17.8%	2.0%	38870	0.655	1.6%	11.5%
3-day maximum	38320	0.5811	41230	0.5422	7.6%	6.7%	36710	0.6024	4.2%	3.7%
7-day maximum	35390	0.5661	36330	0.5403	2.7%	4.6%	33740	0.5792	4.7%	2.3%
30-day maximum	25360	0.5646	25290	0.5841	0.3%	3.5%	25030	0.5904	1.3%	4.6%
90-day maximum	16410	0.5032	16120	0.5021	1.8%	0.2%	16090	0.5008	2.0%	0.5%
				Average Score	14.9%	14.4%		Average Score	7.7%	12.3%
					0	0			0	0
Parameter Group #3										
Date of minimum	274.5	0.07505	270	0.09137	1.6%	21.7%	275.4	0.0914	0.3%	21.8%
Date of maximum	133.3	0.2469	124	0.257	7.0%	4.1%	116.2	0.1866	12.8%	24.4%
				Average Score	4.3%	12.9%		Average Score	6.6%	23.1%
					0	0			0	0
Parameter Group #4										
Low pulse count	3.905	0.4429	4.238	0.4772	8.5%	7.7%	3.238	0.5066	17.1%	14.4%
Low pulse duration	23.44	0.6546	22.19	0.4934	5.3%	24.6%	33.23	0.729	41.8%	11.4%
High pulse count	3.476	0.7199	4.048	0.7986	16.5%	10.9%	2.81	0.7685	19.2%	6.8%
High pulse duration	13.26	0.5882	9.765	0.4143	26.4%	29.6%	14.74	0.674	11.2%	14.6%
				Average Score	14.2%	18.2%		Average Score	22.3%	11.8%
					0	0			0	0
Parameter Group #5										
Rise rate	1016	0.3919	1252	0.5498	23.2%	40.3%	756.2	0.5587	25.6%	42.6%
Fall rate	-588.4	-0.4064	-667.7	-0.5301	13.5%	30.4%	-461.3	-0.5457	21.6%	34.3%
Number of reversals	65.95	0.1252	71.57	0.1379	8.5%	10.1%	58.05	0.1348	12.0%	7.7%
				Average Score	15.1%	27.0%		Average Score	19.7%	28.2%
					0	0			0	0
Total Point					0		Total Point		0	
Classification					1		Classification		1	
note:					Un-impacted condition		note:		Un-impacted condition	

Table 6.19 Evaluation Sheet by DHRAM Method for Control Point SRGW

Control Point:	SRGW		Period:	1940-1960						
IHA statistics group	USGS		MDNF		Absolute Chages		DDNF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	2380	0.8903	2381	0.8897	0.0%	0.1%	2381	0.8897	0.0%	0.1%
February	2891	1.063	2895	1.062	0.1%	0.1%	2895	1.062	0.1%	0.1%
March	2856	0.9502	2859	0.9487	0.1%	0.2%	2859	0.9487	0.1%	0.2%
April	4250	1.283	4253	1.281	0.1%	0.2%	4253	1.281	0.1%	0.2%
May	5273	0.9674	5277	0.9665	0.1%	0.1%	5277	0.9665	0.1%	0.1%
June	3585	1.213	3590	1.211	0.1%	0.2%	3590	1.211	0.1%	0.2%
July	716	1.268	722.1	1.256	0.9%	0.9%	722.1	1.256	0.9%	0.9%
August	298.6	1.278	305.4	1.249	2.3%	2.3%	305.4	1.249	2.3%	2.3%
September	314.3	0.8391	319.1	0.8242	1.5%	1.8%	319.1	0.8242	1.5%	1.8%
October	671.7	1.381	676.4	1.37	0.7%	0.8%	676.4	1.37	0.7%	0.8%
November	1512	1.698	1520	1.687	0.5%	0.6%	1520	1.687	0.5%	0.6%
December	2095	1.101	2211	1.176	5.5%	6.8%	2211	1.176	5.5%	6.8%
					Average	1.0%			Average	1.0%
					Score	0			Score	0
Parameter Group #2										
1-day minimum	48.85	0.6603	10.13	1.66	79.3%	151.4%	26.52	0.992	45.7%	50.2%
3-day minimum	51.19	0.6683	11.84	1.596	76.9%	138.8%	28.38	0.962	44.6%	43.9%
7-day minimum	55.66	0.665	16.87	1.312	69.7%	97.3%	33.18	0.9194	40.4%	38.3%
30-day minimum	98.69	0.8721	77.66	0.8928	21.3%	2.4%	90.02	0.8494	8.8%	2.6%
90-day minimum	266.7	0.9217	258.8	0.8888	3.0%	3.6%	266.7	0.8869	0.0%	3.8%
1-day maximum	31860	0.9299	22020	0.6617	30.9%	28.8%	19530	0.6721	38.7%	27.7%
3-day maximum	29460	0.8738	20770	0.6898	29.5%	21.1%	18990	0.6703	35.5%	23.3%
7-day maximum	23880	0.8	19020	0.6938	20.4%	13.3%	17430	0.6666	27.0%	16.7%
30-day maximum	10450	0.6428	10160	0.6199	2.8%	3.6%	10050	0.6162	3.8%	4.1%
90-day maximum	5369	0.6237	5333	0.6185	0.7%	0.8%	5312	0.6192	1.1%	0.7%
					Average	33.4%			Average	24.6%
					Score	0			Score	0
Parameter Group #3										
Date of minimum	247.5	0.06392	230.1	0.1154	7.0%	80.5%	259.7	0.0948	4.9%	48.3%
Date of maximum	156.4	0.2839	165.7	0.2988	5.9%	5.2%	160	0.2685	2.3%	5.4%
					Average	6.5%			Average	3.6%
					Score	0			Score	0
Parameter Group #4										
Low pulse count	4.143	0.4337	4.81	0.3195	16.1%	26.3%	4.619	0.3317	11.5%	23.5%
Low pulse duration	23.29	0.7089	18.68	0.6144	19.8%	13.3%	20.69	0.7459	11.2%	5.2%
High pulse count	2.333	0.7706	2.619	0.7586	12.3%	1.6%	2.286	0.6937	2.0%	10.0%
High pulse duration	10.11	0.2896	12.32	0.5168	21.9%	78.5%	15.33	0.5277	51.6%	82.2%
					Average	17.5%			Average	19.1%
					Score	0			Score	0
Parameter Group #5										
Rise rate	469.8	0.7136	453.1	0.595	3.6%	16.6%	353.5	0.5526	24.8%	22.6%
Fall rate	-360.3	-0.7137	-265.2	-0.5784	26.4%	19.0%	-218.1	-0.5868	39.5%	17.8%
Number of reversals	60.38	0.1572	60	0.2178	0.6%	38.5%	51.86	0.1587	14.1%	1.0%
					Average	10.2%			Average	26.1%
					Score	0			Score	0
Total Point					1	Total Point				
Classification					2	Classification				1
note:					Low risk of impact	note:				Un-impacted condition

Table 6.20 Evaluation Sheet by DHRAM Method for Control Point SRRL

Control Point:	SRRL		Period:	1940-1960						
IHA statistics group	USGS		MDNF		Absolute Chages		DDNF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	13620	0.8126	13620	0.8121	0.0%	0.1%	13620	0.8121	0.0%	0.1%
February	15590	0.5655	15610	0.564	0.1%	0.3%	15610	0.564	0.1%	0.3%
March	14500	0.5446	14520	0.5431	0.1%	0.3%	14520	0.5431	0.1%	0.3%
April	14200	0.7736	14220	0.7714	0.1%	0.3%	14220	0.7714	0.1%	0.3%
May	17770	0.8819	17760	0.8812	0.1%	0.1%	17760	0.8812	0.1%	0.1%
June	11990	0.9279	12000	0.9263	0.1%	0.2%	12000	0.9263	0.1%	0.2%
July	5539	0.868	5544	0.8667	0.1%	0.1%	5544	0.8667	0.1%	0.1%
August	3138	1.034	3138	1.034	0.0%	0.0%	3138	1.034	0.0%	0.0%
September	2474	1.07	2498	1.073	1.0%	0.3%	2498	1.073	1.0%	0.3%
October	2713	1.108	2720	1.1	0.3%	0.7%	2720	1.1	0.3%	0.7%
November	5248	1.166	5261	1.16	0.2%	0.5%	5261	1.16	0.2%	0.5%
December	9488	1.108	9614	1.102	1.3%	0.5%	9614	1.102	1.3%	0.5%
				Average Score	0.3%	0.3%		Average Score	0.3%	0.3%
Parameter Group #2										
1-day minimum	879.5	0.5008	672.9	0.6058	23.5%	21.0%	774.3	0.5588	12.0%	11.6%
3-day minimum	893.6	0.5095	691.6	0.5983	22.6%	17.4%	793.1	0.5536	11.2%	8.7%
7-day minimum	924.2	0.5145	740.2	0.5806	19.9%	12.8%	843.8	0.5493	8.7%	6.8%
30-day minimum	1151	0.5563	1091	0.5113	5.2%	8.1%	1177	0.5195	2.3%	6.6%
90-day minimum	1885	0.6383	1920	0.6344	1.9%	0.6%	1958	0.6298	3.9%	1.3%
1-day maximum	47590	0.5207	58870	0.579	23.7%	11.2%	47830	0.5851	0.5%	12.4%
3-day maximum	46460	0.5236	52620	0.5517	13.3%	5.4%	45550	0.583	2.0%	11.3%
7-day maximum	43090	0.529	44760	0.5298	3.9%	0.2%	41130	0.5543	4.5%	4.8%
30-day maximum	30540	0.5255	30250	0.5355	0.9%	1.9%	29850	0.5401	2.3%	2.8%
90-day maximum	20090	0.4642	19750	0.4611	1.7%	0.7%	19710	0.4606	1.9%	0.8%
				Average Score	11.7%	7.9%		Average Score	4.9%	6.7%
Parameter Group #3										
Date of minimum	273	0.06876	263.8	0.08837	3.4%	28.5%	271.9	0.09043	0.4%	31.5%
Date of maximum	136.5	0.2536	135.1	0.2566	1.0%	1.2%	137.2	0.2452	0.5%	3.3%
				Average Score	2.2%	14.9%		Average Score	0.5%	17.4%
Parameter Group #4										
Low pulse count	3.238	0.6471	3.667	0.4076	13.2%	37.0%	2.905	0.5647	10.3%	12.7%
Low pulse duration	40.26	1.118	30.06	0.9182	25.3%	17.9%	41.88	0.8572	4.0%	23.3%
High pulse count	3.238	0.6322	4.238	0.7307	30.9%	15.6%	3.095	0.7071	4.4%	11.8%
High pulse duration	13.54	0.6896	9.354	0.6405	30.9%	7.1%	13.41	0.6548	1.0%	5.0%
				Average Score	25.1%	19.4%		Average Score	4.9%	13.2%
Parameter Group #5										
Rise rate	1123	0.38	1621	0.6093	44.3%	60.3%	1029	0.5861	8.4%	54.2%
Fall rate	-721.9	-0.3911	-803.9	-0.5558	11.4%	42.1%	-565.4	-0.5564	21.7%	42.3%
Number of reversals	48.86	0.07187	69.71	0.1699	42.7%	136.4%	54.14	0.1394	10.8%	94.0%
				Average Score	32.8%	79.6%		Average Score	13.6%	63.5%
					0	1			0	1
Total Point					1		Total Point		1	
Classification					2		Classification		2	
note:					Low risk of impact		note:		Low risk of impact	

Annual median flow duration curves represent the overall hydrologic state of a flow such as drought and flood years. Table 6.21 summarizes the comparative scores, made through the qualitative comparison of both annual median flow duration curves at a control point. Annual flow duration curves of each flow sequence at the six control points are as shown in Figures 6.4 to 6.9, respectively.

Table 6.21 Comparative Evaluation of Annual Median Flow Duration Curve
for the Sabine WAM

CP	Data Period	Comparative Score	
		USGS vs. MDNF	USGS vs. DDNF
<u>Total</u>		<u>6</u>	<u>6</u>
BSBS	01/01/1940-12/31/1960	1	1
LFQT	01/01/1940-12/31/1960	1	1
SRBE	01/01/1940-12/31/1960	1	1
SRGW	01/01/1940-12/31/1960	1	1
SRRL	01/01/1940-12/31/1960	1	1
SRBW	01/01/1940-12/31/1960	1	1

Both the annual median flows of MDNF and DDNF at BSBS are underestimated compared to the median of USGS daily recorded flow, but both the disaggregated flows have proportionally similar hydrological state to the USGS flows as shown in Figure 6.4.

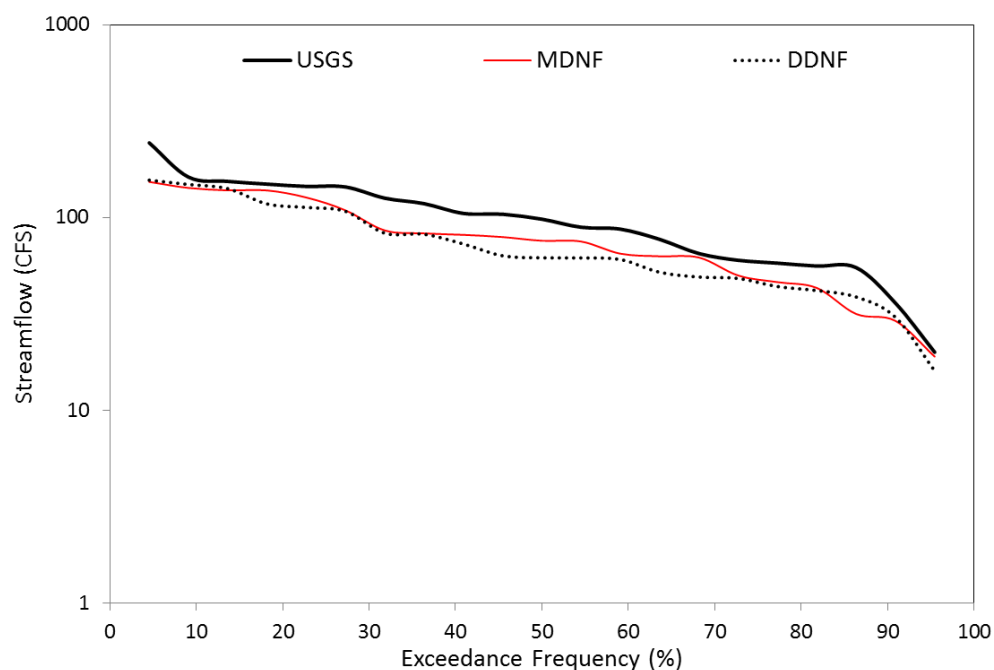


Figure 6.4 Annual Median Flow Duration Curves at BSBS

Both the annual median flows of MDNF and DDNF at LFQT are greater than the median of USGS daily recorded flow. Both the disaggregated flows particularly overestimated than the median of USGS daily recorded flow for relatively flood and normal years, but practically similar for relatively drought years as shown in Figure 6.5.

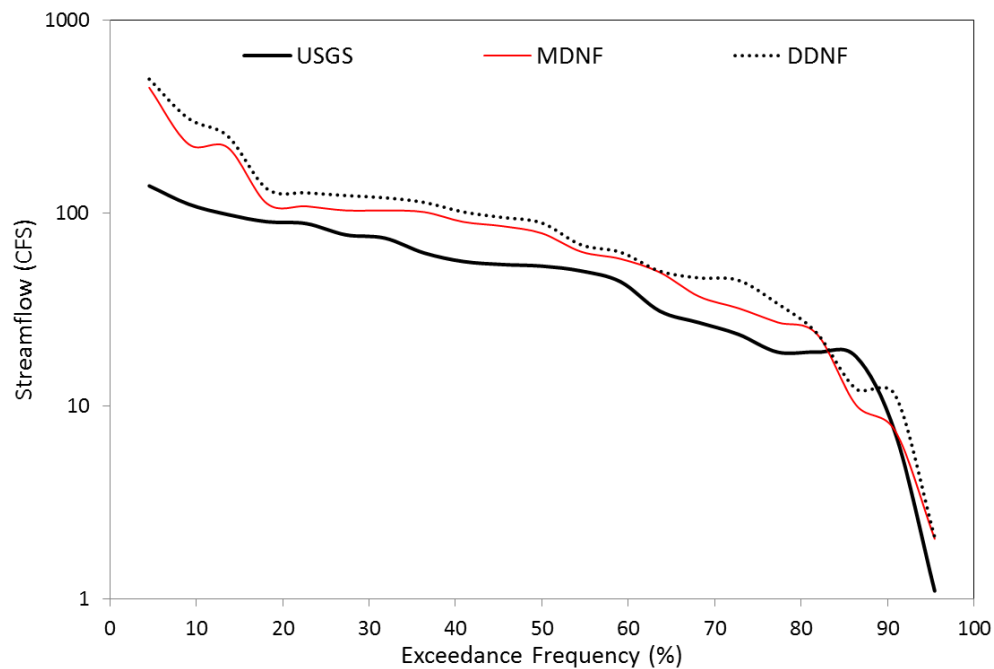


Figure 6.5 Annual Median Flow Duration Curves at LFQT

The annual median duration curves at other four control points are practically identical to one another even though both the annual median flows of MDNF and DDNF are slightly overestimated than the median of USGS daily recorded flow for relatively flood and normal years as shown in Figures 6.6 to 6.9.

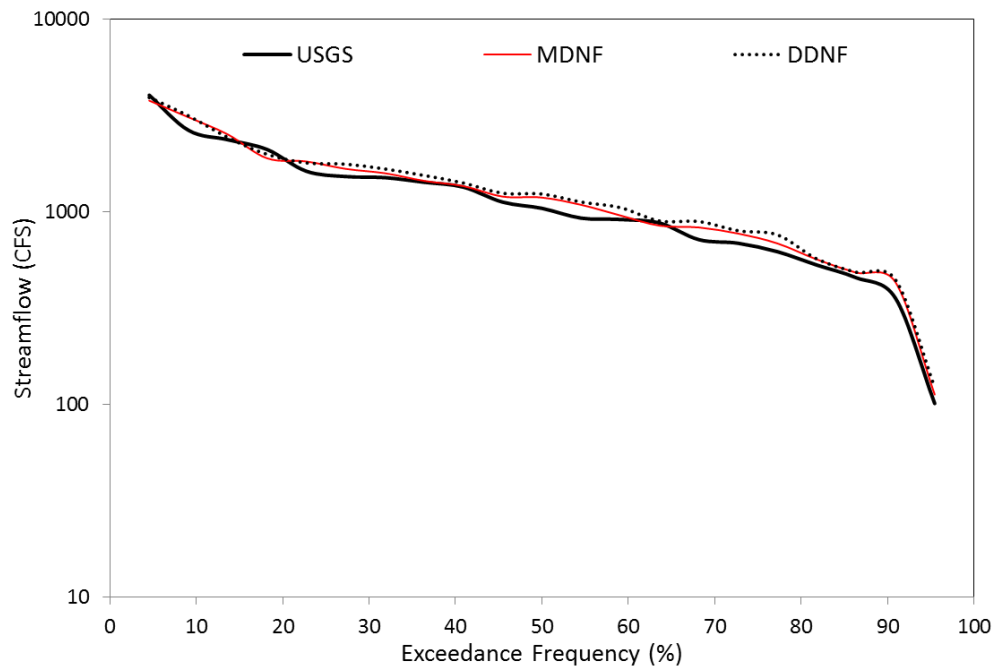


Figure 6.6 Annual Median Flow Duration Curves at SRBE

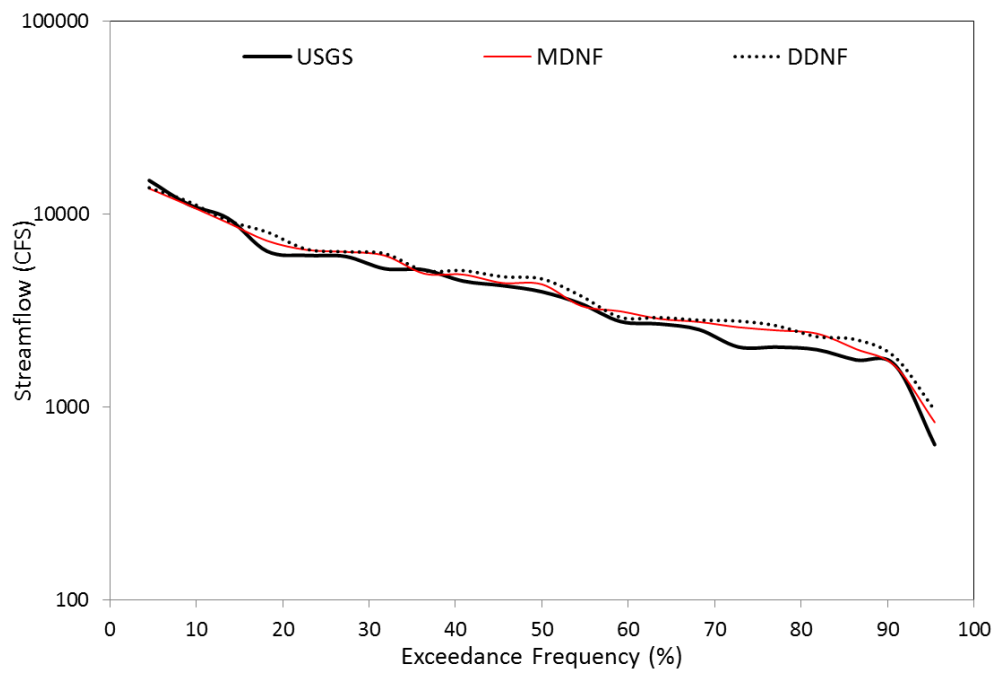


Figure 6.7 Annual Median Flow Duration Curves at SRBW

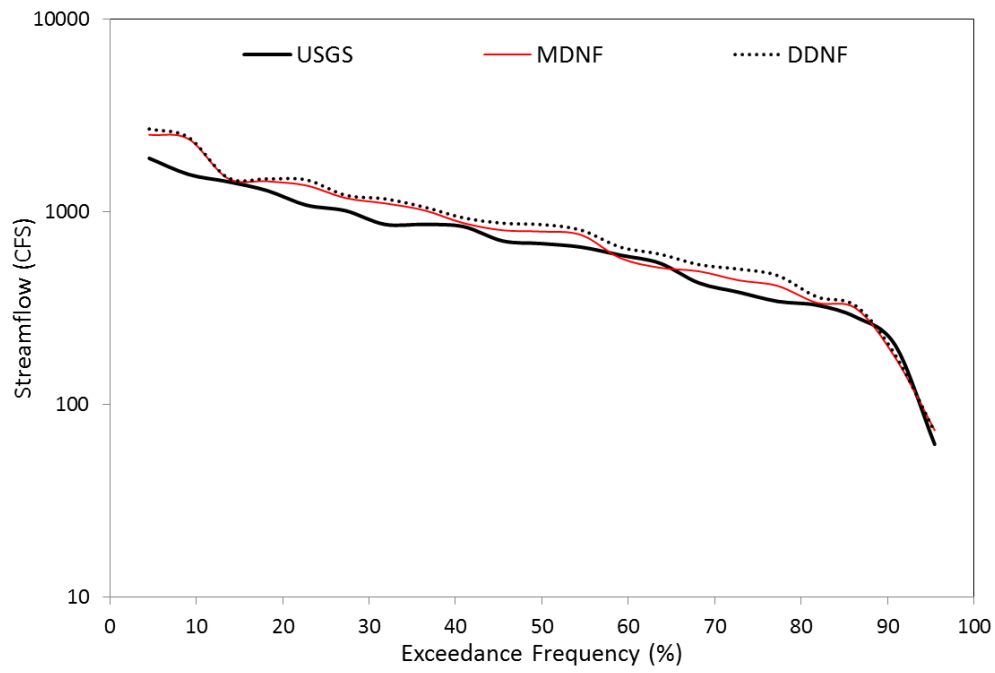


Figure 6.8 Annual Median Flow Duration Curves at SRGW

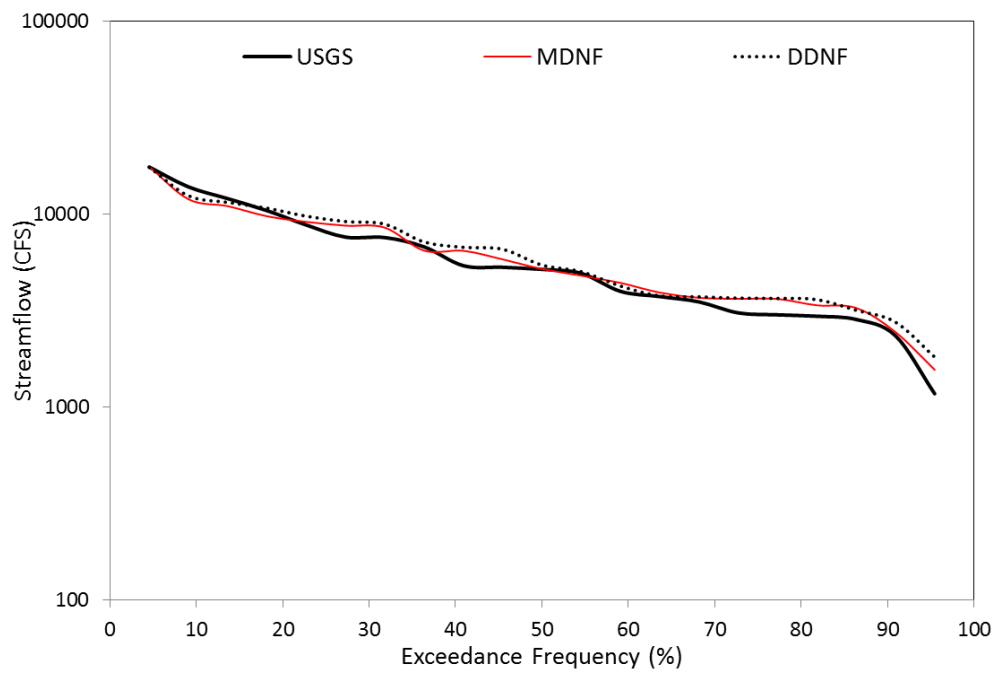


Figure 6.9 Annual Median Flow Duration Curves at SRRL

The calibration strategy with USGS daily recorded flows for the daily SWAT model to synthesize daily flow sequences at 21 control point for 1940-2013 is selected for the daily Sabine WAM as tabulated in Table 6.22. DDNF has better performances on statistic (NSE) and DHRAM evaluations than MDNF while both disaggregated daily flow sequences have similar performance on the qualitative evaluations, flow frequency metrics and median annual flow duration curves.

Table 6.22 Selection of calibration Strategy for the Sabine WAM

Methods	Evaluation purposes	Calibration Strategy	
		Monthly WAM datasets	Daily USGS recorded data
Total Score		2	6
Statistic Evaluation (NSE)	Streamflow timing	0	2
Flow Frequency Metric	Streamflow regime	1	1
DHRAM (IHA)	Hydrologic characteristics alteration	0	2
Median Annual Flow Duration Curve	Overall hydrologic state of a river	1	1

6.4.2 Neches River Basin

NSE values of MDNF and DDNF are computed based on the USGS recorded flows for 21 years at 7 control points, and the comparative scores are made through the comparison of both NSE at a control point as listed in Table 6.23. DDNF outperforms MDNF on NSE evaluation at all the 7 control points. Especially, NSE values of MDNF are not satisfied ($NSE < 0.5$) at MUJA and VIKO, but NSE values of DDNF are satisfied at both same control points. Total comparative scores are 0 for MDNF, and 14 for DDNF.

Table 6.23 Comparative Evaluation of Nash-Sutcliffe Efficiency
for the Neches WAM

CP	Data Period	NSE		Comparative Score	
		USGS vs. MDNF	USGS vs. DDNF	USGS vs. MDNF	USGS vs. DDNF
<u>Total</u>				<u>0</u>	<u>14</u>
MUJA	01/01/1940-12/31/1960	0.33	0.58	0	2
ANLU	01/01/1940-12/31/1960	0.62	0.82	0	2
NENE	01/01/1940-12/31/1960	0.54	0.71	0	2
NEDI	01/01/1940-12/31/1960	0.60	0.84	0	2
NERO	01/01/1940-12/31/1960	0.56	0.88	0	2
VIKO	01/01/1940-12/31/1960	0.39	0.51	0	2
NEEV	01/01/1940-12/31/1960	0.66	0.90	0	2

The three different flow frequency metrics of USGS daily recorded flows, MDNF, and DDNF at 7 control points are tabulated in Tables 6.24, 6.25 and 6.26. Both mean flows of MDNF and DDNF are very similar to the mean flows of USGS daily recorded flows at all control points. Both median (50% exceedance frequency) values of the MDNF and DDNF are also closely similar to the median values of USGS daily recorded flows at all control points. However, maximum values of MDNF are mostly much higher than USGS daily recorded flows except for NENE. However, DDNF are mostly less than USGS daily recorded flows except for NEEV. These indicate that the flow regimes of MDNF are more variable than DDNF.

Table 6.24 Flow Frequency Metrics for Control Points MUJA, ANLU, and NENE

	MUJA (CFS)			ANLU (CFS)			NENE (CFS)		
	USGS	MDNF	DDNF	USGS	MDNF	DDNF	USGS	MDNF	DDNF
Mean	293.6	301.3	301.3	1,298	1,303	1,303	799.9	796.4	796.4
Std Dev	717.2	667.3	625.7	2,120	2,069	2,126	1,671	1,362	1,471
Min	0.0	0.0	0.0	1.0	0.0	0.0	0.4	0.0	0.0
Max	18,400	25,939	14,106	30,200	31,468	22,908	44,100	30,746	25,516
0.1%	8,146	5,996	6,865	21,831	20,207	19,354	17,826	13,390	16,210
0.2%	6,400	4,747	4,962	18,662	16,188	17,657	15,100	10,767	11,382
0.5%	4,443	3,207	3,855	13,400	12,335	13,214	11,128	8,017	9,481
1%	3,403	2,437	3,018	9,576	9,568	9,797	7,319	6,300	6,803
2%	2,206	1,911	2,319	7,569	7,400	7,800	4,751	4,828	5,252
5%	1,080	1,253	1,356	4,770	5,202	5,500	2,950	3,292	3,433
10%	680.0	806.8	828.8	3,350	3,474	3,558	1,910	2,121	2,160
15%	502.0	579.0	553.2	2,600	2,638	2,627	1,380	1,519	1,515
20%	363.0	437.0	408.0	2,060	2,074	2,014	1,080	1,190	1,097
30%	227.0	266.0	222.0	1,310	1,292	1,195	734.0	752.0	681.0
40%	144.0	157.0	124.0	842.0	809.0	773.0	488.0	475.0	438.0
50%	89.0	95.0	75.0	508.0	498.0	478.0	320.0	312.0	280.0
60%	56.0	59.0	45.0	328.0	321.0	296.0	204.0	190.0	165.0
70%	32.0	33.0	26.0	204.0	199.0	174.0	121.0	108.0	92.0
80%	17.0	15.0	12.0	110.0	103.0	93.0	60.0	51.0	45.0
85%	10.0	8.0	7.0	80.0	70.0	57.0	42.0	30.0	27.0
90%	4.0	3.0	3.0	46.0	40.0	28.0	27.0	15.0	13.0
95%	0.0	0.0	0.0	21.0	8.0	4.0	13.0	1.0	1.0
98%	0.0	0.0	0.0	5.0	0.0	0.0	5.0	0.0	0.0
99%	0.0	0.0	0.0	3.0	0.0	0.0	3.0	0.0	0.0
99.5%	0.0	0.0	0.0	2.0	0.0	0.0	2.0	0.0	0.0
99.8%	0.0	0.0	0.0	1.0	0.0	0.0	1.0	0.0	0.0
99.9%	0.0	0.0	0.0	1.0	0.0	0.0	1.0	0.0	0.0

Table 6.25 Flow Frequency Metrics for Control Points NEDI, NERO and VIKO

	NEDI (CFS)			NERO (CFS)			VIKO (CFS)		
	USGS	MDNF	DDNF	USGS	MDNF	DDNF	USGS	MDNF	DDNF
Mean	1,827	1,822	1,822	2,560	2,554	2,554	848.3	847.4	847.4
Std Dev	2,928	2,949	2,719	3,973	4,362	3,893	2,035	1,737	1,627
Min	1.0	0.0	0.0	1.8	0.0	0.0	15.9	1.0	0.0
Max	49,900	53,444	30,203	49,700	81,250	46,166	62,200	70,122	32,616
0.1%	32,794	32,407	26,612	37,298	47,211	36,569	30,422	16,538	17,052
0.2%	24,922	21,797	23,655	32,231	38,947	30,774	16,528	11,243	14,202
0.5%	18,756	16,704	15,270	25,564	24,993	23,040	10,100	7,737	8,829
1%	12,600	13,832	12,725	18,084	19,629	18,227	7,011	5,746	7,048
2%	9,780	10,449	9,518	13,600	15,193	14,454	4,917	4,838	5,539
5%	6,720	6,839	6,928	9,960	10,027	9,706	3,234	3,345	3,758
10%	4,680	4,902	4,929	6,970	6,782	6,787	2,050	2,214	2,333
15%	3,730	3,623	3,880	5,302	5,111	5,215	1,472	1,613	1,601
20%	2,870	2,830	2,966	4,170	3,927	4,159	1,100	1,212	1,194
30%	1,940	1,869	1,920	2,690	2,462	2,586	686.0	787.0	709.0
40%	1,250	1,175	1,200	1,640	1,494	1,667	434.0	522.0	440.0
50%	748.0	727.0	760.0	1,020	943.0	1,059	300.0	325.0	275.0
60%	450.0	439.0	474.0	580.0	596.0	680.0	218.0	216.0	167.0
70%	270.0	266.0	273.0	349.0	354.0	379.0	156.0	140.0	97.0
80%	148.0	140.0	148.0	192.0	172.0	201.0	117.0	84.0	45.0
85%	113.0	97.0	102.0	142.0	123.0	131.0	97.0	62.0	26.0
90%	70.0	53.0	56.0	88.0	80.0	75.0	80.0	40.0	13.0
95%	30.0	12.0	9.0	39.0	20.0	24.0	55.0	20.0	2.0
98%	7.0	0.0	0.0	10.0	1.0	0.0	40.0	7.0	0.0
99%	3.0	0.0	0.0	6.0	0.0	0.0	32.0	4.0	0.0
99.5%	2.0	0.0	0.0	4.0	0.0	0.0	25.0	2.0	0.0
99.8%	1.0	0.0	0.0	2.0	0.0	0.0	22.0	2.0	0.0
99.9%	1.0	0.0	0.0	2.0	0.0	0.0	20.0	1.0	0.0

Table 6.26 Flow Frequency Metrics for Control Point NEEV

	NEEV (CFS)		
	USGS	MDNF	DDNF
Mean	6,593	6,564	6,564
Std Dev	9,001	9,150	8,833
Min	62.9	21.4	0.0
Max	92,100	103,114	103,261
0.1%	77,520	77,362	75,502
0.2%	65,394	67,497	63,482
0.5%	52,000	55,482	49,836
1%	44,000	42,533	41,995
2%	32,600	33,772	33,878
5%	23,800	23,458	23,243
10%	17,600	17,199	17,139
15%	13,600	13,338	13,456
20%	10,900	10,802	10,603
30%	7,394	7,091	7,172
40%	4,600	4,766	4,773
50%	2,840	3,053	3,127
60%	1,880	1,910	2,118
70%	1,330	1,212	1,292
80%	819.0	717.0	765.0
85%	602.0	528.0	573.0
90%	443.0	373.0	375.0
95%	267.0	182.0	205.0
98%	165.0	94.0	72.0
99%	123.0	63.0	17.0
99.5%	93.0	51.0	0.0
99.8%	81.0	39.0	0.0
99.9%	74.0	24.0	0.0

Table 6.27 summarizes the comparative scores, made through the qualitative comparison of both flow frequency metrics at a control.

Table 6.27 Comparative Evaluation of Flow Frequency Metrics
for the Neches WAM

CP	Data Period	Comparative Score	
		USGS vs. MDNF	USGS vs. DDNF
<u>Total</u>		<u>7</u>	<u>7</u>
MUJA	01/01/1940-12/31/1960	1	1
ANLU	01/01/1940-12/31/1960	1	1
NENE	01/01/1940-12/31/1960	1	1
NEDI	01/01/1940-12/31/1960	1	1
NERO	01/01/1940-12/31/1960	1	1
VIKO	01/01/1940-12/31/1960	1	1
NEEV	01/01/1940-12/31/1960	1	1

The DHRAM method quantitatively evaluates how much the hydrologic characteristics of MDNF and DDNF are similar to or different from USGS daily recorded flows at the seven control points. Table 6.28 summarizes the scores of MDNF and DDNF based on the USGS daily recorded flows at seven control points.

Table 6.28 indicates that both MDNF and DDNF have similar hydrological characteristics to USGS daily recorded flows at seven control points. MDNFs at the two control points, MUJA and VIKO are more identical to USGS daily recorded flows than DDNFs, while DDNF at NERO is more similar to USGS daily recorded flows than MDNF. Both MDNF and DDNF at the other four control points have the same impact points. The detailed evaluation sheets for the seven control points are tabulated in Tables 6.29 to 6.35.

Table 6.28 Impact Points by the DHRAM Method for the Neches WAM

CP	Data Period	Impact Points	
		USGS vs. MDNF	USGS vs. DDNF
<u>Total</u>		<u>20</u>	<u>22</u>
MUJA	01/01/1940-12/31/1960	5	6
ANLU	01/01/1940-12/31/1960	4	4
NENE	01/01/1940-12/31/1960	3	3
NEDI	01/01/1940-12/31/1960	1	1
NERO	01/01/1940-12/31/1960	3	0
VIKO	01/01/1940-12/31/1960	3	7
NEEV	01/01/1940-12/31/1960	1	1

Table 6.29 indicates that the amount of daily maximum and minimum flows of NDMF and DDMF are nearly to the same as the USGS daily recorded flows according to the results of parameter group #2, but the flow timings of both MDNF and DDNF are slightly different from the USGS daily recorded flows according to the results of parameter group #3 and #4 at MUJA.

Table 6.29 Evaluation Sheet by DHRAM Method for Control Point MUJA

Control Point:	MUJA	Period:	1940-1960							
IHA statistics group	USGS		MDNF		Absolute Chages		DDNF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	394.4	0.8771	404.1	0.851	2.5%	3.0%	404.1	0.851	2.5%	3.0%
February	463.2	0.6879	482.1	0.6637	4.1%	3.5%	482.1	0.6636	4.1%	3.5%
March	440.2	0.7642	454.4	0.7327	3.2%	4.1%	454.4	0.7327	3.2%	4.1%
April	440.6	0.7924	454.6	0.7963	3.2%	0.5%	454.6	0.7963	3.2%	0.5%
May	648.1	0.9665	654.5	0.9526	1.0%	1.4%	654.4	0.9526	1.0%	1.4%
June	230.5	1.092	233.7	1.083	1.4%	0.8%	233.8	1.083	1.4%	0.8%
July	91.13	2.255	93.33	2.199	2.4%	2.5%	93.33	2.199	2.4%	2.5%
August	32.16	1.501	40.2	1.334	25.0%	11.1%	40.19	1.334	25.0%	11.1%
September	42.99	1.168	41.05	1.018	4.5%	12.8%	41.04	1.018	4.5%	12.8%
October	99.92	1.885	103.9	1.831	4.0%	2.9%	103.9	1.831	4.0%	2.9%
November	293.5	1.691	299.7	1.646	2.1%	2.7%	299.7	1.646	2.1%	2.7%
December	354.4	1.125	363.2	1.092	2.5%	2.9%	363.2	1.092	2.5%	2.9%
					Average Score	4.7% 0			Average Score	4.7% 0
Parameter Group #2										
1-day minimum	3.952	1.476	2.286	1.439	42.2%	2.5%	1.667	2.073	57.8%	40.4%
3-day minimum	4.159	1.417	2.73	1.494	34.4%	5.4%	2.222	1.802	46.6%	27.2%
7-day minimum	4.81	1.294	4.109	1.446	14.6%	11.7%	3.19	1.63	33.7%	26.0%
30-day minimum	9.541	0.9749	13.84	1.262	45.1%	29.4%	14.46	1.364	51.6%	39.9%
90-day minimum	37.97	1.492	40.76	1.332	7.3%	10.7%	39.92	1.246	5.1%	16.5%
1-day maximum	5809	0.7462	6461	1.018	11.2%	36.4%	4000	0.7645	31.1%	2.5%
3-day maximum	4776	0.7086	3611	0.7374	24.4%	4.1%	3354	0.6369	29.8%	10.1%
7-day maximum	3122	0.6227	2450	0.6204	21.5%	0.4%	2787	0.6007	10.7%	3.5%
30-day maximum	1205	0.5283	1175	0.4958	2.5%	6.2%	1234	0.4777	2.4%	9.6%
90-day maximum	682.1	0.4605	679.7	0.4319	0.4%	6.2%	692.6	0.4372	1.5%	5.1%
					Average Score	20.3% 0			Average Score	27.0% 0
Parameter Group #3										
Date of minimum	228.9	0.06412	225.9	0.2181	1.3%	240.1%	201.7	0.2397	11.9%	273.8%
Date of maximum	151.3	0.2532	151.6	0.262	0.2%	3.5%	78	0.1588	48.4%	37.3%
					Average Score	0.8% 0			Average Score	30.2% 2
Parameter Group #4										
Low pulse count	4.048	0.5551	6.333	0.4531	56.4%	18.4%	6.905	0.4195	70.6%	24.4%
Low pulse duration	32.47	1.109	16.23	0.7937	50.0%	28.4%	13.72	0.7882	57.7%	28.9%
High pulse count	4.095	0.7984	5.714	0.5369	39.5%	32.8%	4.667	0.5881	14.0%	26.3%
High pulse duration	4.812	0.3246	4.96	0.6773	3.1%	108.7%	6.665	0.3676	38.5%	13.2%
					Average Score	37.3% 1			Average Score	45.2% 1
Parameter Group #5										
Rise rate	146	0.6284	251.9	0.5009	72.5%	20.3%	133.6	0.4131	8.5%	34.3%
Fall rate	-71.76	-0.5708	-68.2	-0.5388	5.0%	5.6%	-53.79	-0.466	25.0%	18.4%
Number of reversals	70.95	0.1747	80.86	0.2305	14.0%	31.9%	61.48	0.1874	13.3%	7.3%
					Average Score	30.5% 0			Average Score	15.6% 0
					Total Point Classification	5 3			Total Point Classification	6 3
					note:	Moderate risk of impact			note:	Moderate risk of impact

Table 6.30 also describes that the amount of daily maximum and minimum flows of NDMF and DDMF are similar to the USGS daily recorded flows according to the results of parameter group #2, but the flow timings of both MDNF and DDNF are slightly different from the USGS daily recorded flows according to the results of parameter group #3 at ANLU.

Table 6.30 Evaluation Sheet by DHRAM Method for Control Point ANLU

Control Point:	ANLU		Period:	1940-1960						
IHA statistics group	USGS		MDNF		Absolute Chages		DDNF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	1881	0.9506	1892	0.9449	0.6%	0.6%	1892	0.9449	0.6%	0.6%
February	2123	0.6507	2140	0.6469	0.8%	0.6%	2140	0.6469	0.8%	0.6%
March	2055	0.6846	2065	0.6796	0.5%	0.7%	2065	0.6796	0.5%	0.7%
April	1819	0.6739	1830	0.6703	0.6%	0.5%	1830	0.6703	0.6%	0.5%
May	2777	0.963	2785	0.9611	0.3%	0.2%	2785	0.9611	0.3%	0.2%
June	1065	0.9056	1076	0.9185	1.0%	1.4%	1076	0.9185	1.0%	1.4%
July	423.8	1.32	424.8	1.313	0.2%	0.5%	424.8	1.313	0.2%	0.5%
August	143.8	0.7119	148.5	0.7294	3.3%	2.5%	148.5	0.7295	3.3%	2.5%
September	269.1	1.482	266.4	1.446	1.0%	2.4%	266.4	1.446	1.0%	2.4%
October	505.5	1.539	504.1	1.545	0.3%	0.4%	504.1	1.545	0.3%	0.4%
November	1091	1.533	1092	1.528	0.1%	0.3%	1092	1.528	0.1%	0.3%
December	1457	1.182	1459	1.178	0.1%	0.3%	1459	1.178	0.1%	0.3%
				Average	0.7%	0.9%		Average	0.7%	0.9%
				Score	0	0		Score	0	0
Parameter Group #2										
1-day minimum	45.33	0.7467	24.33	1.107	46.3%	48.3%	17.52	1.678	61.4%	124.7%
3-day minimum	47.06	0.7605	26.73	1.073	43.2%	41.1%	19.51	1.565	58.5%	105.8%
7-day minimum	50.86	0.7978	31.64	1.046	37.8%	31.1%	22.95	1.378	54.9%	72.7%
30-day minimum	71.8	0.7753	61.34	0.8031	14.6%	3.6%	51.55	0.872	28.2%	12.5%
90-day minimum	187.9	1.067	183.5	1.006	2.3%	5.7%	185.5	1.027	1.3%	3.7%
1-day maximum	11530	0.7044	11310	0.6779	1.9%	3.8%	9960	0.599	13.6%	15.0%
3-day maximum	10940	0.7021	9974	0.6811	8.8%	3.0%	9514	0.6152	13.0%	12.4%
7-day maximum	9309	0.6811	8403	0.6152	9.7%	9.7%	8772	0.6089	5.8%	10.6%
30-day maximum	4717	0.5809	4689	0.56	0.6%	3.6%	4850	0.5715	2.8%	1.6%
90-day maximum	2860	0.5014	2879	0.5068	0.7%	1.1%	2896	0.5068	1.3%	1.1%
				Average	16.6%	15.1%		Average	24.1%	36.0%
				Score	0	0		Score	0	0
Parameter Group #3										
Date of minimum	259.1	0.05849	234.5	0.2033	9.5%	247.6%	214.9	0.2492	17.1%	326.1%
Date of maximum	133	0.2495	112	0.241	15.8%	3.4%	115.1	0.2397	13.5%	3.9%
				Average	12.6%	125.5%		Average	15.3%	165.0%
				Score	1	3		Score	1	3
Parameter Group #4										
Low pulse count	3.238	0.4132	3.857	0.436	19.1%	5.5%	4.476	0.3645	38.2%	11.8%
Low pulse duration	32.89	0.8949	28.48	0.9479	13.4%	5.9%	21.47	0.7491	34.7%	16.3%
High pulse count	3.476	0.9697	3.81	0.7636	9.6%	21.3%	3.143	0.6919	9.6%	28.6%
High pulse duration	9.922	0.5863	9.019	0.6294	9.1%	7.4%	11.96	0.6221	20.5%	6.1%
				Average	12.8%	10.0%		Average	25.8%	15.7%
				Score	0	0		Score	0	0
Parameter Group #5										
Rise rate	248.1	0.6388	357.1	0.5568	43.9%	12.8%	265.4	0.5136	7.0%	19.6%
Fall rate	-144.9	-0.5704	-134.2	-0.4996	7.4%	12.4%	-136.4	-0.4913	5.9%	13.9%
Number of reversals	59.43	0.1839	62.43	0.2468	5.0%	34.2%	48.33	0.1909	18.7%	3.8%
				Average	18.8%	19.8%		Average	10.5%	12.4%
				Score	0	0		Score	0	0
Total Point					4		Total Point		4	
Classification					2		Classification		2	
note:					Low risk of impact		note:		Low risk of impact	

Table 6.31 also shows that the amount of daily maximum and minimum flows of NDMF and DDMF are similar to the USGS daily recorded flows according to the results of parameter group #2, but the flow timings of both MDNF and DDNF are much more variable, compared to the USGS daily recorded flows according to the absolute changes of coefficient of variance (CV) on parameter group #3 at NENE.

Table 6.31 Evaluation Sheet by DHRAM Method for Control Point NENE

Control Point:	NENE		Period:	1940-1960						
IHA statistics group	USGS		MDNF		Absolute Chages		DDNF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	1051	0.7852	1048	0.7887	0.3%	0.4%	1048	0.7886	0.3%	0.4%
February	1162	0.7472	1159	0.7506	0.3%	0.5%	1159	0.7506	0.3%	0.5%
March	1280	0.7531	1277	0.7559	0.2%	0.4%	1277	0.7559	0.2%	0.4%
April	1517	1.068	1514	1.071	0.2%	0.3%	1514	1.071	0.2%	0.3%
May	1738	0.8391	1735	0.8402	0.2%	0.1%	1735	0.8402	0.2%	0.1%
June	800.6	1.081	796.9	1.087	0.5%	0.6%	796.8	1.087	0.5%	0.6%
July	241.4	1.205	236.9	1.233	1.9%	2.3%	236.9	1.233	1.9%	2.3%
August	73.95	1.293	68.85	1.395	6.9%	7.9%	68.86	1.395	6.9%	7.9%
September	121.4	1.68	117.8	1.738	3.0%	3.5%	117.8	1.738	3.0%	3.5%
October	234.6	1.164	230.7	1.187	1.7%	2.0%	230.7	1.187	1.7%	2.0%
November	591.3	1.529	588.1	1.538	0.5%	0.6%	588	1.538	0.6%	0.6%
December	813	1.044	810.1	1.047	0.4%	0.3%	810.1	1.047	0.4%	0.3%
					Average Score	1.3% 0			Average Score	1.3% 0
Parameter Group #2										
1-day minimum	22.19	0.7294	11	1.59	50.4%	118.0%	10.76	1.742	51.5%	138.8%
3-day minimum	23.3	0.7385	12.57	1.648	46.1%	123.2%	12.46	1.874	46.5%	153.8%
7-day minimum	25.11	0.7448	16.4	1.698	34.7%	128.0%	18.48	2.291	26.4%	207.6%
30-day minimum	36.6	0.7336	37.99	1.202	3.8%	63.8%	38.56	1.453	5.4%	98.1%
90-day minimum	108.1	1.228	101.4	1.278	6.2%	4.1%	100.2	1.26	7.3%	2.6%
1-day maximum	11160	0.8361	9742	0.7079	12.7%	15.3%	8356	0.6535	25.1%	21.8%
3-day maximum	10220	0.8114	7131	0.5223	30.2%	35.6%	7671	0.6652	24.9%	18.0%
7-day maximum	7940	0.7466	5925	0.5073	25.4%	32.1%	6718	0.6076	15.4%	18.6%
30-day maximum	3323	0.5555	3246	0.5277	2.3%	5.0%	3320	0.5299	0.1%	4.6%
90-day maximum	1869	0.5084	1839	0.5023	1.6%	1.2%	1858	0.5048	0.6%	0.7%
					Average Score	21.3% 0			Average Score	20.3% 0
Parameter Group #3										
Date of minimum	246.7	0.05608	243	0.1924	1.5%	243.1%	233.9	0.1937	5.2%	245.4%
Date of maximum	136.1	0.2046	131.7	0.2063	3.2%	0.8%	140.9	0.2411	3.5%	17.8%
					Average Score	2.4% 0			Average Score	4.4% 0
Parameter Group #4										
Low pulse count	3.238	0.5066	4.095	0.5563	26.5%	9.8%	4.238	0.5218	30.9%	3.0%
Low pulse duration	39.03	1.044	29.04	1.083	25.6%	3.7%	29.08	1.149	25.5%	10.1%
High pulse count	3.238	0.7807	4.381	0.6394	35.3%	18.1%	4.714	0.5658	45.6%	27.5%
High pulse duration	7.654	0.3404	8.271	0.4766	8.1%	40.0%	7.394	0.4883	3.4%	43.4%
					Average Score	23.9% 0			Average Score	26.3% 0
Parameter Group #5										
Rise rate	206.1	0.7029	278.6	0.4889	35.2%	30.4%	201.1	0.435	2.4%	38.1%
Fall rate	-113.3	-0.5933	-118	-0.5907	4.1%	0.4%	-121.3	-0.5217	7.1%	12.1%
Number of reversals	55.76	0.1622	69.67	0.2605	24.9%	60.6%	53.33	0.19	4.4%	17.1%
					Average Score	21.4% 0			Average Score	4.6% 0
Total Point					3		Total Point		3	
Classification					2		Classification		2	
note:					Low risk of impact		note:		Low risk of impact	

The detailed evaluation sheets at control point NEDI and NEEV show that hydrologic characteristics of both NDMF and DDMF are almost identical to the USGS daily recorded flows as tabulated in Tables 6.32 and 6.33, respectively.

Table 6.32 Evaluation Sheet by DHRAM Method for Control Point NEDI

Control Point:	NEDI		Period:	1940-1960						
IHA statistics group	USGS		MDNF		Absolute Chages		DDNF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	2513	0.9603	2506	0.9637	0.3%	0.4%	2506	0.9637	0.3%	0.4%
February	2839	0.7398	2820	0.7462	0.7%	0.9%	2821	0.7461	0.6%	0.9%
March	2877	0.7336	2870	0.7366	0.2%	0.4%	2870	0.7367	0.2%	0.4%
April	2786	0.9575	2839	0.946	1.9%	1.2%	2839	0.946	1.9%	1.2%
May	4118	0.9686	4084	0.9543	0.8%	1.5%	4084	0.9543	0.8%	1.5%
June	1928	0.794	1903	0.7972	1.3%	0.4%	1903	0.7972	1.3%	0.4%
July	660.9	1.076	660.6	1.121	0.0%	4.2%	660.6	1.121	0.0%	4.2%
August	198.2	0.9611	188.5	0.9085	4.9%	5.5%	188.5	0.9085	4.9%	5.5%
September	300.8	1.485	285.4	1.479	5.1%	0.4%	285.4	1.479	5.1%	0.4%
October	496.5	1.41	515.2	1.365	3.8%	3.2%	515.2	1.365	3.8%	3.2%
November	1344	1.421	1339	1.432	0.4%	0.8%	1339	1.432	0.4%	0.8%
December	1921	1.232	1908	1.243	0.7%	0.9%	1908	1.243	0.7%	0.9%
				Average	1.7%	1.6%		Average	1.7%	1.6%
				Score	0	0		Score	0	0
Parameter Group #2										
1-day minimum	67.1	0.7394	41.48	1.306	38.2%	76.6%	38.57	1.383	42.5%	87.0%
3-day minimum	69.08	0.7369	43.35	1.287	37.2%	74.7%	40.37	1.345	41.6%	82.5%
7-day minimum	71.7	0.7351	47.2	1.238	34.2%	68.4%	45.59	1.277	36.4%	73.7%
30-day minimum	94.55	0.7492	87.65	0.9052	7.3%	20.8%	88.23	0.9952	6.7%	32.8%
90-day minimum	228.1	1.031	212.3	1.042	6.9%	1.1%	219.6	1.056	3.7%	2.4%
1-day maximum	15130	0.8048	18870	0.7151	24.7%	11.1%	11240	0.7156	25.7%	11.1%
3-day maximum	14060	0.8107	13190	0.7169	6.2%	11.6%	10720	0.6962	23.8%	14.1%
7-day maximum	11790	0.7757	10910	0.7071	7.5%	8.8%	10030	0.7004	14.9%	9.7%
30-day maximum	6697	0.6446	6624	0.6264	1.1%	2.8%	6725	0.6252	0.4%	3.0%
90-day maximum	4086	0.5638	4013	0.5515	1.8%	2.2%	4035	0.5554	1.2%	1.5%
				Average	16.5%	27.8%		Average	19.7%	31.8%
				Score	0	0		Score	0	0
Parameter Group #3										
Date of minimum	259	0.1678	246	0.1837	5.0%	9.5%	250.6	0.1007	3.2%	40.0%
Date of maximum	133.3	0.2524	132	0.2405	1.0%	4.7%	131.2	0.2468	1.6%	2.2%
				Average	3.0%	7.1%		Average	2.4%	21.1%
				Score	0	0		Score	0	0
Parameter Group #4										
Low pulse count	2.571	0.4183	3.476	0.6008	35.2%	43.6%	2.476	0.6589	3.7%	57.5%
Low pulse duration	39.72	0.7896	33.7	0.8477	15.2%	7.4%	50.98	0.9824	28.3%	24.4%
High pulse count	3.333	0.7727	5.143	0.7257	54.3%	6.1%	2.476	0.8626	25.7%	11.6%
High pulse duration	10.31	0.7513	6.856	0.6899	33.5%	8.2%	17.42	0.4724	69.0%	37.1%
				Average	34.5%	16.3%		Average	31.7%	32.7%
				Score	0	0		Score	0	1
Parameter Group #5										
Rise rate	337	0.7019	670.7	0.7211	99.0%	2.7%	257.7	0.6032	23.5%	14.1%
Fall rate	-185.2	-0.6598	-248.2	-0.6586	34.0%	0.2%	-125.7	-0.604	32.1%	8.5%
Number of reversals	55.76	0.1559	70.86	0.194	27.1%	24.4%	44.19	0.2131	20.7%	36.7%
				Average	53.4%	9.1%		Average	25.5%	19.7%
				Score	1	0		Score	0	0
Total Point					1		Total Point		1	
Classification					2		Classification		2	
note:					Low risk of impact		note:		Low risk of impact	

Table 6.33 Evaluation Sheet by DHRAM Method for Control Point NEEV

Control Point:	NEEV		Period:	1940-1960						
IHA statistics group	USGS		MDNF		Absolute Chages		DDNF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	9627	0.9046	9646	0.9	0.2%	0.5%	9646	0.9	0.2%	0.5%
February	10970	0.6999	11010	0.6911	0.4%	1.3%	11010	0.6911	0.4%	1.3%
March	10020	0.6157	10090	0.6068	0.7%	1.4%	10090	0.6068	0.7%	1.4%
April	9612	0.7028	9769	0.6835	1.6%	2.7%	9769	0.6835	1.6%	2.7%
May	13160	1.009	13110	1.011	0.4%	0.2%	13110	1.011	0.4%	0.2%
June	7301	0.8838	7128	0.9049	2.4%	2.4%	7128	0.9049	2.4%	2.4%
July	2540	0.6723	2395	0.7914	5.7%	17.7%	2395	0.7914	5.7%	17.7%
August	1407	0.7147	1178	0.8039	16.3%	12.5%	1178	0.8039	16.3%	12.5%
September	1241	0.9425	1241	1.156	0.0%	22.7%	1241	1.156	0.0%	22.7%
October	1903	1.252	1917	1.222	0.7%	2.4%	1917	1.222	0.7%	2.4%
November	4109	1.479	4533	1.433	10.3%	3.1%	4533	1.433	10.3%	3.1%
December	7484	1.367	7034	1.233	6.0%	9.8%	7034	1.233	6.0%	9.8%
				Average	3.7%	6.4%		Average	3.7%	6.4%
				Score	0	0		Score	0	0
Parameter Group #2										
1-day minimum	388	0.7217	219.8	0.7456	43.4%	3.3%	261.9	0.8542	32.5%	18.4%
3-day minimum	400.1	0.7354	230.4	0.746	42.4%	1.4%	273.9	0.858	31.5%	16.7%
7-day minimum	420.2	0.7206	268.2	0.7042	36.2%	2.3%	296.7	0.8291	29.4%	15.1%
30-day minimum	536.7	0.6952	440.4	0.6928	17.9%	0.3%	495.4	0.8076	7.7%	16.2%
90-day minimum	1013	0.7766	917.5	0.7594	9.4%	2.2%	942.6	0.7485	6.9%	3.6%
1-day maximum	36590	0.625	40890	0.6397	11.8%	2.4%	35780	0.6911	2.2%	10.6%
3-day maximum	36010	0.6225	38490	0.6531	6.9%	4.9%	34490	0.6494	4.2%	4.3%
7-day maximum	33960	0.6223	35080	0.6535	3.3%	5.0%	32350	0.6269	4.7%	0.7%
30-day maximum	22600	0.6282	22270	0.6118	1.5%	2.6%	22370	0.6327	1.0%	0.7%
90-day maximum	14270	0.5346	14210	0.5287	0.4%	1.1%	14200	0.5282	0.5%	1.2%
				Average	17.3%	2.6%		Average	12.1%	8.7%
				Score	0	0		Score	0	0
Parameter Group #3										
Date of minimum	284.6	0.1009	282.7	0.09833	0.7%	2.5%	257.5	0.1786	9.5%	77.0%
Date of maximum	129.1	0.2685	143.6	0.2537	11.2%	5.5%	134	0.2577	3.8%	4.0%
				Average	5.9%	4.0%		Average	6.7%	40.5%
				Score	0	0		Score	0	1
Parameter Group #4										
Low pulse count	3.333	0.496	3.429	0.4754	2.9%	4.2%	2.714	0.4381	18.6%	11.7%
Low pulse duration	31.65	0.8456	36.19	1.383	14.3%	63.6%	37.8	0.8698	19.4%	2.9%
High pulse count	2.619	0.6666	3.524	0.9223	34.6%	38.4%	2.905	0.8355	10.9%	25.3%
High pulse duration	16.92	0.7145	12.94	0.4682	23.5%	34.5%	16.27	0.6484	3.8%	9.3%
				Average	18.8%	35.1%		Average	13.2%	12.3%
				Score	0	1		Score	0	0
Parameter Group #5										
Rise rate	716.5	0.4394	1155	0.5363	61.2%	22.1%	635.9	0.5119	11.2%	16.5%
Fall rate	-495.9	-0.4231	-502.6	-0.5432	1.4%	28.4%	-375.2	-0.5398	24.3%	27.6%
Number of reversals	59.43	0.1984	69.57	0.2174	17.1%	9.6%	44.67	0.1563	24.8%	21.2%
				Average	26.5%	20.0%		Average	20.1%	21.8%
				Score	0	0		Score	0	0
Total Point					1	Total Point				
Classification					2	Classification				
note:					Low risk of impact	note:				

The DHRAH method evaluates 3 impact points for MDNF and 7 impact points for DDNF, respectively in comparison with the USGS daily recorded flows at control point VIKO as tabulated in Table 6.34. The minimum flows of DDNF are obviously different from the USGS daily recorded flows. The flow timings of both MDNF and DDNF are relatively variable and slightly different from the USGS daily recorded flows according to the absolute changes of coefficient of variance (CV) on parameter group #3.

Table 6.34 Evaluation Sheet by DHRAM Method for Control Point VIKO

Control Point:	VIKO		Period:		1940-1960					
IHA statistics group	USGS		MDNF		Absolute Chages		DDNF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	1352	0.9072	1351	0.9072	0.1%	0.0%	1351	0.9072	0.1%	0.0%
February	1328	0.6261	1327	0.6261	0.1%	0.0%	1327	0.6261	0.1%	0.0%
March	1063	0.6857	1061	0.6857	0.2%	0.0%	1061	0.6857	0.2%	0.0%
April	1081	0.7904	1080	0.7904	0.1%	0.0%	1080	0.7904	0.1%	0.0%
May	1257	1.215	1256	1.215	0.1%	0.0%	1256	1.215	0.1%	0.0%
June	829.2	1.78	828.3	1.78	0.1%	0.0%	828.2	1.78	0.1%	0.0%
July	424.7	0.8235	424.2	0.8237	0.1%	0.0%	424.2	0.8236	0.1%	0.0%
August	259.6	1.142	259.2	1.142	0.2%	0.0%	259.3	1.142	0.1%	0.0%
September	242	0.8419	241.7	0.8419	0.1%	0.0%	241.7	0.8419	0.1%	0.0%
October	411.7	2.135	411.2	2.135	0.1%	0.0%	411.2	2.135	0.1%	0.0%
November	937.9	1.718	936.8	1.718	0.1%	0.0%	936.8	1.718	0.1%	0.0%
December	1028	1.283	1026	1.283	0.2%	0.0%	1026	1.283	0.2%	0.0%
					Average Score	0.1% 0.0%			Average Score	0.1% 0.0%
Parameter Group #2										
1-day minimum	76	0.4845	12.62	1.147	83.4%	136.7%	1.714	1.61	97.7%	232.3%
3-day minimum	77.27	0.4885	13.76	1.137	82.2%	132.8%	1.921	1.514	97.5%	209.9%
7-day minimum	80.18	0.5002	17.59	1.107	78.1%	121.3%	2.524	1.357	96.9%	171.3%
30-day minimum	99.05	0.5334	56.71	0.797	42.7%	49.4%	26.46	1.178	73.3%	120.8%
90-day minimum	166.9	0.63	158.2	0.663	5.2%	5.2%	151.5	0.6725	9.2%	6.7%
1-day maximum	15610	1.088	13710	1.247	12.2%	14.6%	8496	0.8027	45.6%	26.2%
3-day maximum	12380	1.011	8351	1.022	32.5%	1.1%	7959	0.8346	35.7%	17.4%
7-day maximum	7794	0.8487	6148	0.8267	21.1%	2.6%	6812	0.7595	12.6%	10.5%
30-day maximum	3282	0.7681	3218	0.7108	2.0%	7.5%	3303	0.7079	0.6%	7.8%
90-day maximum	1807	0.5561	1801	0.5471	0.3%	1.6%	1817	0.5443	0.6%	2.1%
					Average Score	36.0% 47.3%			Average Score	47.0% 80.5%
Parameter Group #3										
Date of minimum	265	0.08349	236.9	0.185	10.6%	121.6%	198.7	0.2033	25.0%	143.5%
Date of maximum	165.5	0.2899	158.6	0.3023	4.2%	4.3%	136.2	0.2719	17.7%	6.2%
					Average Score	7.4% 62.9%			Average Score	21.4% 74.9%
Parameter Group #4										
Low pulse count	5.571	0.6657	5.286	0.4361	5.1%	34.5%	6.857	0.349	23.1%	47.6%
Low pulse duration	16.85	0.7441	18.19	0.6214	8.0%	16.5%	12.95	0.3337	23.1%	55.2%
High pulse count	3.81	0.7171	4.714	0.704	23.7%	1.8%	4.571	0.6868	20.0%	4.2%
High pulse duration	5.163	0.3755	6.124	0.5774	18.6%	53.8%	7.579	0.5566	46.8%	48.2%
					Average Score	13.9% 26.6%			Average Score	28.2% 38.8%
Parameter Group #5										
Rise rate	342.8	0.7148	507	0.7826	47.9%	9.5%	271.4	0.5199	20.8%	27.3%
Fall rate	-184	-0.7447	-137.6	-0.7988	25.2%	7.3%	-130.4	-0.5373	29.1%	27.9%
Number of reversals	69.14	0.1001	67.62	0.224	2.2%	123.8%	53.9	0.1677	22.0%	67.5%
					Average Score	25.1% 46.8%			Average Score	24.0% 40.9%
					Total Point Classification	3 2			Total Point Classification	7 3
					note:	Low risk of impact			note:	Moderate risk of impact

Hydrologic characteristics of both NDMF and DDMF are almost identical to the USGS daily recorded flows at control point NERO as tabulated in Table 6.35.

Table 6.35 Evaluation Sheet by DHRAM Method for Control Point NERO

Control Point:	NERO		Period:		1940-1960					
IHA statistics group	USGS		MDNF		Absolute Chages		DDNF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	3660	0.9727	3647	0.9776	0.4%	0.5%	3647	0.9776	0.4%	0.5%
February	4131	0.8002	4127	0.8005	0.1%	0.0%	4127	0.8005	0.1%	0.0%
March	3872	0.6988	3864	0.7009	0.2%	0.3%	3864	0.7009	0.2%	0.3%
April	3899	0.9472	3964	0.9291	1.7%	1.9%	3964	0.9291	1.7%	1.9%
May	5695	1.043	5650	1.029	0.8%	1.3%	5650	1.029	0.8%	1.3%
June	2762	0.8409	2735	0.8476	1.0%	0.8%	2735	0.8475	1.0%	0.8%
July	849.2	0.9768	861.8	1.009	1.5%	3.3%	861.7	1.009	1.5%	3.3%
August	335.6	1.081	303.7	1.075	9.5%	0.6%	303.7	1.074	9.5%	0.6%
September	354.4	1.282	349.7	1.315	1.3%	2.6%	349.7	1.315	1.3%	2.6%
October	641	1.29	666	1.263	3.9%	2.1%	666.1	1.263	3.9%	2.1%
November	1905	1.553	1884	1.566	1.1%	0.8%	1884	1.566	1.1%	0.8%
December	2712	1.302	2694	1.313	0.7%	0.8%	2694	1.313	0.7%	0.8%
					Average Score	1.8% 0			Average Score	1.8% 0
Parameter Group #2										
1-day minimum	85.52	0.7631	55.33	1.06	35.3%	38.9%	59.52	0.987	30.4%	29.3%
3-day minimum	88	0.7714	56.86	1.049	35.4%	36.0%	62.29	0.9962	29.2%	29.1%
7-day minimum	93.5	0.7813	60.83	1.022	34.9%	30.8%	69.37	1.013	25.8%	29.7%
30-day minimum	125	0.78	106.7	0.9015	14.6%	15.6%	118.5	0.934	5.2%	19.7%
90-day minimum	298.6	0.9893	272.7	0.9162	8.7%	7.4%	291.5	0.9764	2.4%	1.3%
1-day maximum	18100	0.7032	29450	0.6672	62.7%	5.1%	17060	0.636	5.7%	9.6%
3-day maximum	17550	0.7041	21560	0.711	22.8%	1.0%	15990	0.6484	8.9%	7.9%
7-day maximum	15830	0.6945	16340	0.7104	3.2%	2.3%	14700	0.6686	7.1%	3.7%
30-day maximum	9420	0.6629	9445	0.6452	0.3%	2.7%	9497	0.6516	0.8%	1.7%
90-day maximum	5762	0.5623	5693	0.5583	1.2%	0.7%	5710	0.5624	0.9%	0.0%
					Average Score	21.9% 0			Average Score	11.6% 0
Parameter Group #3										
Date of minimum	255.8	0.1731	248.6	0.1729	2.8%	0.1%	251.3	0.1752	1.8%	1.2%
Date of maximum	136.6	0.2463	151.2	0.2872	10.7%	16.6%	134.7	0.258	1.4%	4.8%
					Average Score	6.8% 0			Average Score	1.6% 0
Parameter Group #4										
Low pulse count	2.762	0.5714	2.524	0.4271	8.6%	25.3%	2.714	0.4681	1.7%	18.1%
Low pulse duration	43.88	0.9441	41.85	0.8764	4.6%	7.2%	43.51	1.078	0.8%	14.2%
High pulse count	2.762	0.7842	6.381	0.6911	131.0%	11.9%	3.619	0.8031	31.0%	2.4%
High pulse duration	14.75	0.6411	4.993	0.7196	66.1%	12.2%	10.63	0.6724	27.9%	4.9%
					Average Score	52.6% 1			Average Score	15.4% 0
Parameter Group #5										
Rise rate	379.1	0.5561	1172	0.6689	209.2%	20.3%	441.3	0.617	16.4%	11.0%
Fall rate	-208.7	-0.5586	-415	-0.6271	98.9%	12.3%	-203.9	-0.6179	2.3%	10.6%
Number of reversals	59.9	0.1543	73.67	0.3138	23.0%	103.4%	52.67	0.2964	12.1%	92.1%
					Average Score	110.3% 2			Average Score	10.3% 0
					Total Point Classification	3 2			Total Point Classification	0 1
					note:	Low risk of impact			note:	Un-impacted condition

Table 6.36 Comparative Evaluation of Annual Median Flow Duration Curve
for the Neches WAM

CP	Data Period	Comparative Score	
		USGS vs. MDNF	USGS vs. DDNF
<u>Total</u>		<u>7</u>	<u>7</u>
MUJA	01/01/1940-12/31/1960	1	1
ANLU	01/01/1940-12/31/1960	1	1
NENE	01/01/1940-12/31/1960	1	1
NEDI	01/01/1940-12/31/1960	1	1
NERO	01/01/1940-12/31/1960	1	1
VIKO	01/01/1940-12/31/1960	1	1
NEEV	01/01/1940-12/31/1960	1	1

Annual flow duration curves of each flow at all the seven control points are practically very similar as shown in Figures 6.10 to 6.16, respectively. This indicates both the disaggregated flows have very similar hydrologic state to the USGS daily recorded flows at the same sites for the drought and flood years.

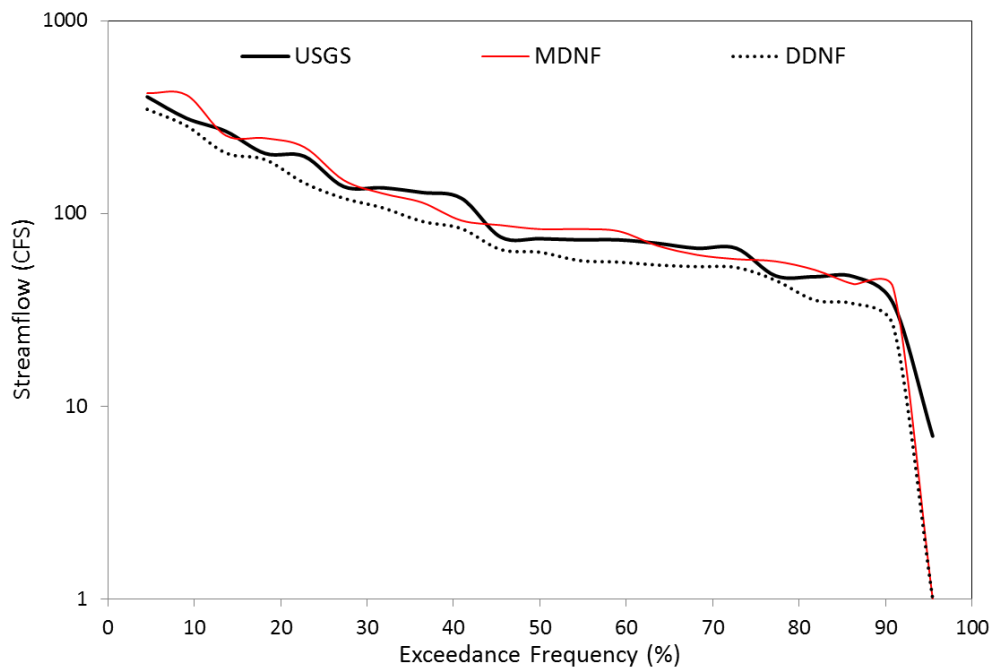


Figure 6.10 Annual Median Flow Duration Curves at MUJA

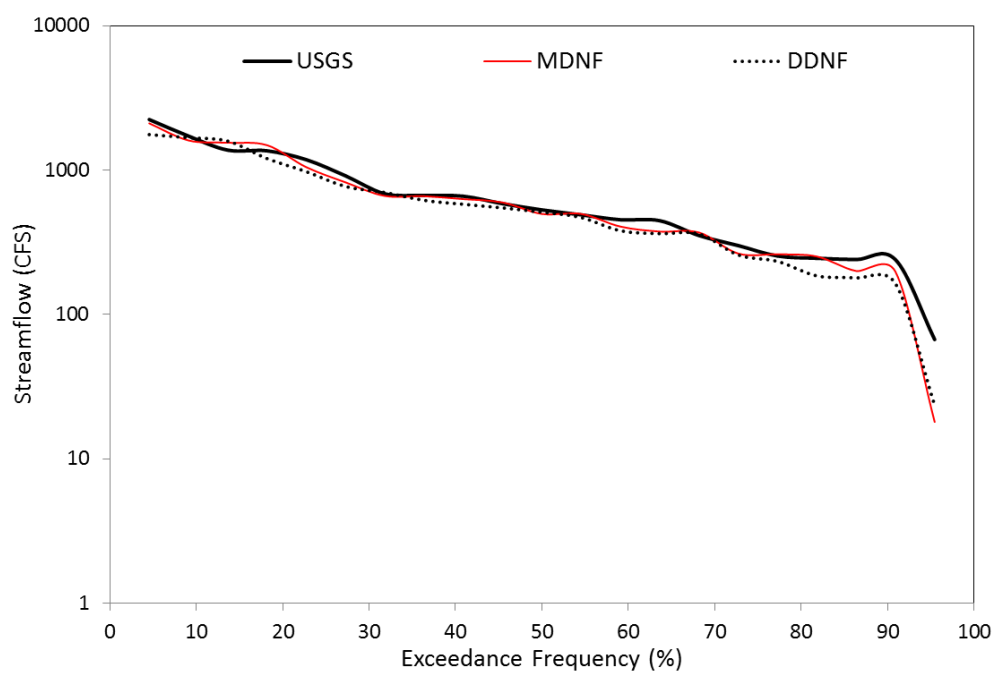


Figure 6.11 Annual Median Flow Duration Curves at ANLU

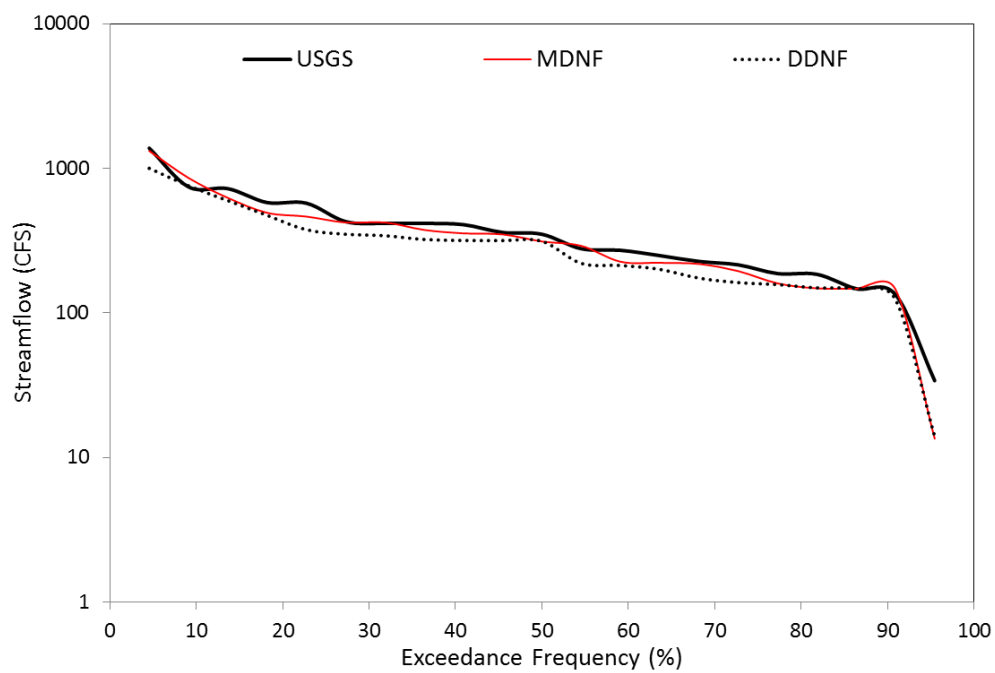


Figure 6.12 Annual Median Flow Duration Curves at NENE

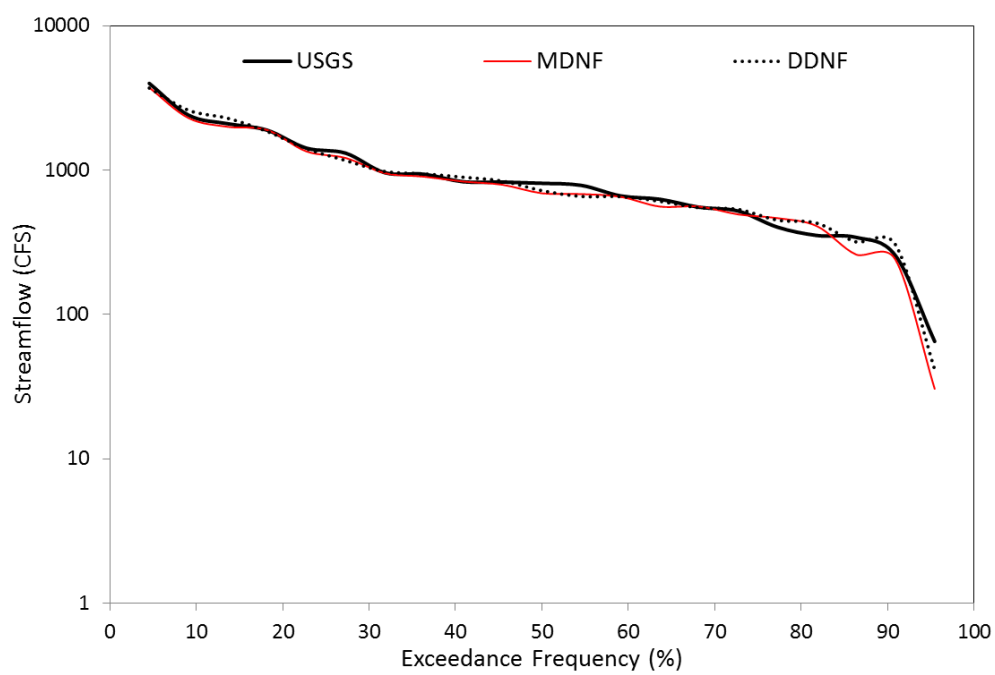


Figure 6.13 Annual Median Flow Duration Curves at NEDI

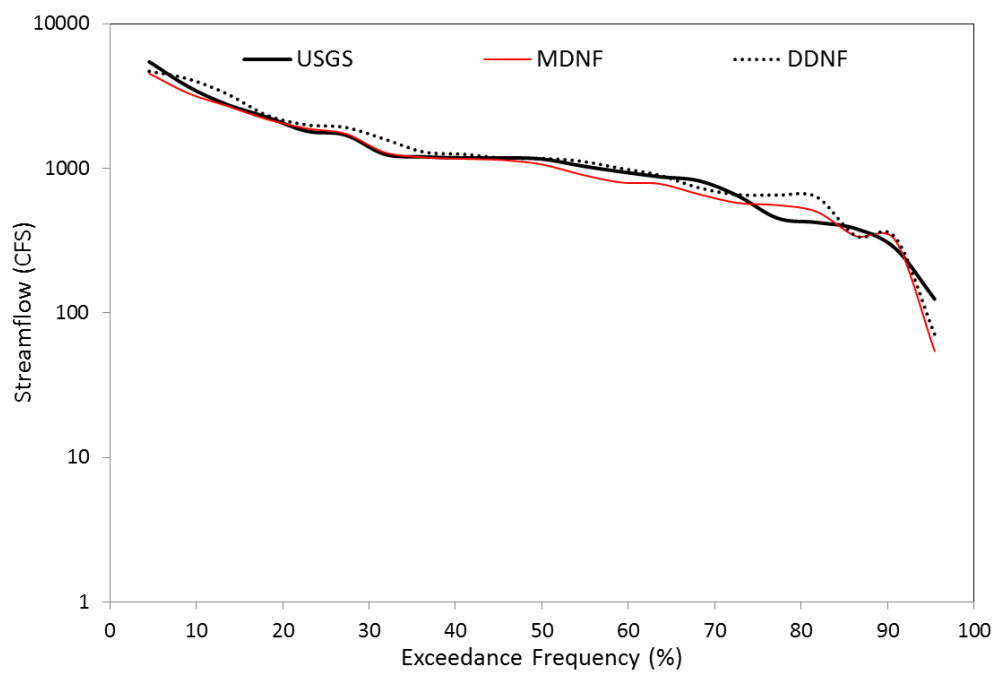


Figure 6.14 Annual Median Flow Duration Curves at NERO

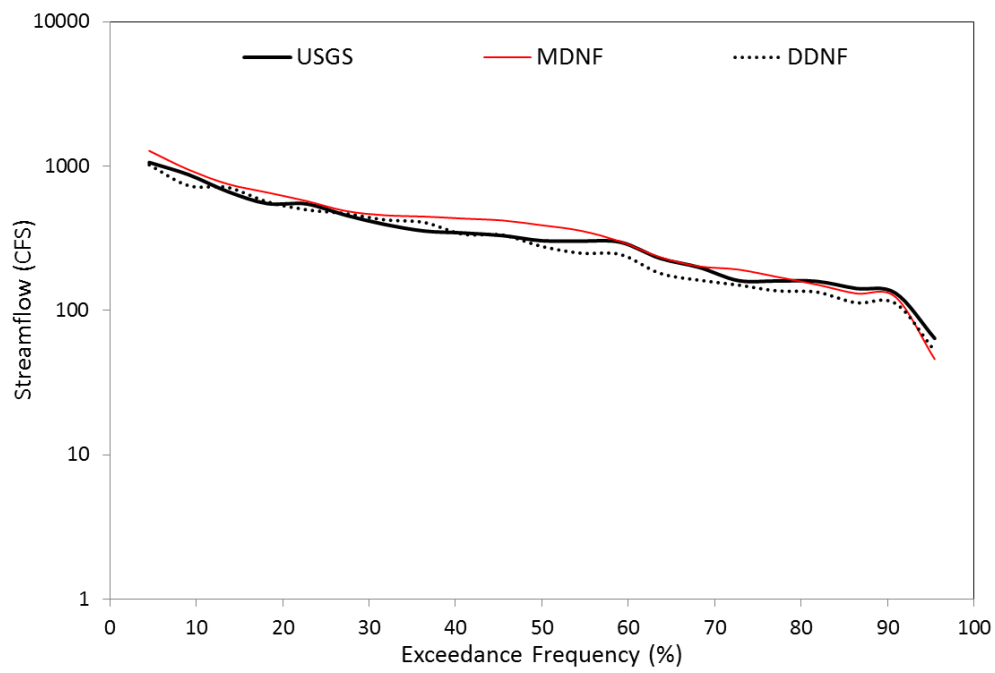


Figure 6.15 Annual Median Flow Duration Curves at VIKO

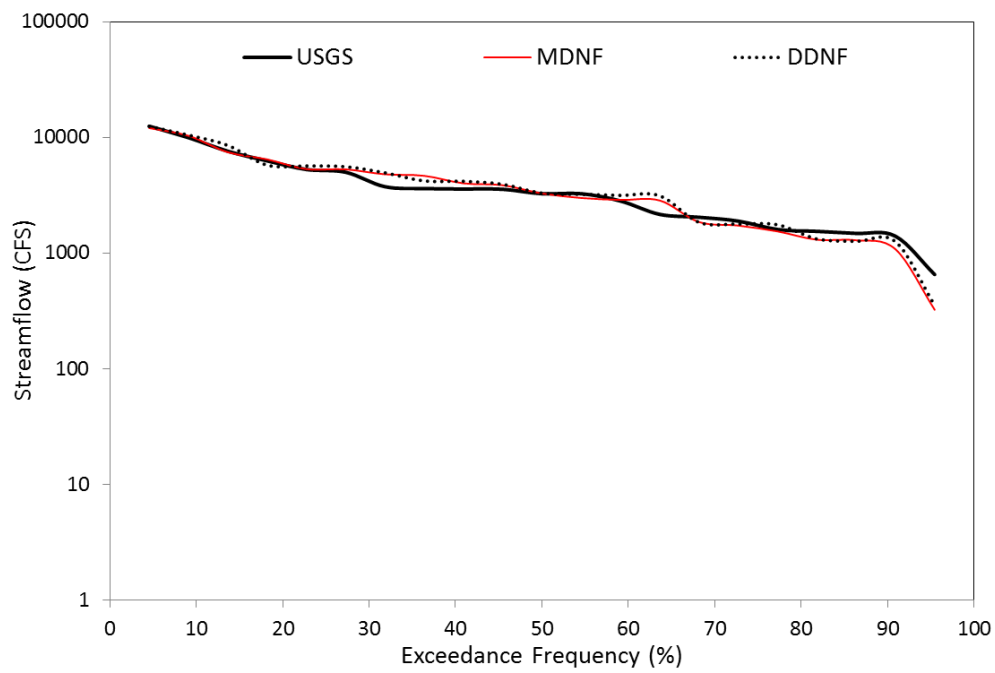


Figure 6.16 Annual Median Flow Duration Curves at NEEV

The total scores are the same through the comparison of both the flows as tabulated in Table 6.37. On a closer view, DDNF overwhelmingly outperforms than MDNF on statistic (NSE), while MDNF has slightly better performance than DDNF on DHRAM evaluation. Both disaggregated daily flow sequences have similar performance on the qualitative evaluation like flow frequency metrics and median annual flow duration curves like the Sabine WAM. Accordingly, the calibration strategy with USGS daily recorded flows for the daily SWAT model is selected for the daily Neches WAM.

Table 6.37 Selection of Calibration Strategy for the Neches WAM

Methods	Evaluation purposes	Calibration Strategy	
		Monthly WAM datasets	Daily USGS recorded data
Total Score		4	4
Statistic Evaluation (NSE)	Streamflow timing	0	2
Flow Frequency Metric	Streamflow regime	1	1
DHRAM (IHA)	Hydrologic characteristics alteration	2	0
Median Annual Flow Duration Curve	Overall hydrologic state of a river	1	1

6.4.3 GSA River Basins

NSE values of MDNF and DDNF are computed based on the USGS recorded flows at the 12 control points, and the comparative scores are made through the comparison of both the NSEs at a control point as listed in Table 6.38. MDNF outperforms DDNF on NSE evaluation at 8 out of the 12 control points in total. But, NSE values of DDNF are higher than MDNF at the 2 control points CP35 and CP37. MDNF has unacceptable value of NSE ($NSE < 0$) at the 2 control points, CP15 and CP18 while DDNF has the unacceptable value at the 3 control points, CP15, CP18, and CP28, respectively. Total comparative scores are 18 for MDNF, and 6 for DDNF, respectively.

Table 6.38 Comparative Evaluation of Nash-Sutcliffe Efficiency for the GSA WAM

CP	Data Period	NSE		Comparative Score	
		USGS vs. MDNF	USGS vs. DDNF	USGS vs. MDNF	USGS vs. DDNF
<u>Total</u>				<u>18</u>	<u>6</u>
CP01	06/01/1939-12/31/1960	0.49	0.28	2	0
CP02	01/01/1934-12/31/1960	0.66	0.23	2	0
CP04	01/01/1934-12/31/1960	0.61	0.54	2	0
CP08	01/01/1934-12/31/1960	0.72	0.08	2	0
CP10	05/01/1939-12/31/1960	0.53	0.38	2	0
CP11	01/01/1934-12/31/1960	0.40	0.37	2	0
CP15	01/01/1934-12/31/1960	-0.47	-0.66	1	1
CP18	03/01/1939-12/31/1960	-4.45	-2.45	1	1
CP28	08/01/1939-12/31/1960	0.20	-0.35	2	0
CP32	01/01/1934-12/31/1960	0.55	0.41	2	0
CP35	01/01/1934-12/31/1960	0.53	0.57	0	2
CP37	03/01/1939-12/31/1960	0.62	0.64	0	2

The three different flow frequency metrics of USGS daily recorded flows, MDNF, and DDNF at 12 control points are tabulated in Tables 6.39 to 6.42. Both mean flows of MDNF and DDNF are very similar to the mean flows of USGS daily recorded flows at all control points. Median (50% exceedance frequency) values of MDCF are also closely similar to the median flows of USGS daily recorded flows at all control points while median values of DDNF are underestimated than the value of the USGS flows. Minimum values of both MDNF and DDNF are almost similar to the USGS flows. Maximum values of DDNF are mostly similar to USGS daily recorded flows except for CP11. However, maximum values of DDNF are greatly variable depending on the control points

Maximum values of DDNF are more similar to USGS daily recorded flows than MDNF, but overall flow regimes of MDNF are more identical to the USGS flows than DDNF at control points CP01, CP02 and CP04. This indicates that MDNFs are better than DDNF on the evaluation of flow frequency metric at control points CP01, CP02 and CP04.

Table 6.39 Flow Frequency Metrics for Control Points CP01, CP02, and CP04

	CP01 (CFS)			CP02 (CFS)			CP04 (CFS)		
	USGS	MDNF	DDNF	USGS	MDNF	DDNF	USGS	MDNF	DDNF
Mean	143.3	143.7	143.7	284.0	284.4	284.4	381.0	372.8	372.8
Std Dev	488.9	344.1	609.6	1,158	820.2	1,576	1,258	835.2	1,366
Min	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max	25,300	10,281	22,230	66,100	30,855	65,049	52,300	28,635	53,913
0.1%	6,900	5,306	10,323	14,137	12,011	27,102	19,252	10,712	17,397
0.2%	4,307	3,806	7,137	9,752	6,771	13,878	9,181	7,886	12,549
0.5%	2,081	1,703	2,787	4,205	3,603	6,582	5,051	4,694	7,184
1%	1,291	1,226	1,685	2,685	2,478	3,552	3,070	3,194	4,808
2%	732.0	827.8	973.0	1,697	1,640	1,892.0	2,017	2,116	3,055
5%	414.0	499.1	468.0	852.0	973	897.0	1,160	1,311	1,352
10%	270.0	325.0	269.0	550.0	648	487.0	794.0	906.0	758.0
15%	200.0	250.0	187.0	396.0	486.0	345.0	589.0	673.0	530.0
20%	165.0	202.0	143.0	314.0	376.0	250.0	479.0	532.0	392.0
30%	121.0	130.0	88.0	220.0	240.0	144.0	334.0	341.0	227.0
40%	90.0	84.0	56.0	157.0	158.0	89.0	235.0	215.0	137.0
50%	67.0	57.0	36.0	106.0	106.0	55.0	163.0	141.0	88.0
60%	50.0	36.0	24.0	77.0	71.0	33.0	105.0	91.0	53.0
70%	37.0	21.0	14.0	58.0	43.0	17.0	78.0	55.0	28.0
80%	27.0	9.0	7.0	40.0	24.0	8.0	49.0	30.0	10.0
85%	20.0	5.0	5.0	29.0	16.0	4.0	36.0	21.0	4.0
90%	12.0	2.0	2.0	18.0	8.0	1.0	23.0	11.0	0.0
95%	2.0	1.0	1.0	4.0	2.0	0.0	11.0	3.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0	5.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0	1.0	0.0	0.0
99.5%	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
99.8%	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
99.9%	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Maximum values of MDNF are more similar to USGS daily recorded flows than MDNF, but overall flow regimes of both MDNF and DDNF are identical to the USGS flows at control points CP08. Maximum values of both MDNF and DDNF are similar to USGS daily recorded flows, but overall flow regime of MDNF is more similar to the USGS flows than DDNF at the control points CP10. Maximum values of both MDNF and DDNF are underestimated than USGS daily recorded flows, and overall flow regimes of both MDNF and DDNF are similar to the USGS flows than DDNF at the control points CP11. These mean that MDNFs are the same as DDNFs at the control points CP08 and CP11, but MDNF is better than DDNF at the control point CP10.

Table 6.40 Flow Frequency Metrics for Control Points CP08, CP10 and CP11

	CP08 (CFS)			CP10 (CFS)			CP11 (CFS)		
	USGS	MDNF	DDNF	USGS	MDNF	DDNF	USGS	MDNF	DDNF
Mean	115.7	115.7	115.7	333.5	321.9	321.9	95.8	94.9	94.9
Std Dev	507.9	483.4	736.9	840.8	725.5	928.2	696.1	499.9	521.0
Min	0.7	0.0	0.0	43.0	0.0	0.0	0.0	0.0	0.0
Max	36,900	36,965	40,670	25,000	22,214	24,272	43,800	19,517	18,906
0.1%	4,833	3,740	10,473	12,800	9,063	11,471	9,306	5,986	6,281
0.2%	3,564	2,645	4,751	10,716	7,079	9,657	5,983	4,543	4,712
0.5%	1,874	1,303	1,620	5,031	4,693	5,947	3,541	2,692	2,728
1%	1,110	1,013	818.0	3,068	2,905	4,051	2,180	1,823	1,706
2%	667.7	691.4	563.0	1,597	1,961	2,873	1,047	1,057	1,031
5%	380.0	449.8	339.0	843.0	1,111	1,556	298.8	387.8	397.7
10%	244.7	291.1	213.0	585.0	776.3	802.0	64.0	160.0	153.0
15%	172.0	202.0	157.0	466.0	573.0	507.0	37.0	81.0	72.0
20%	133.0	147.9	120.0	368.0	446.0	334.0	27.0	44.0	39.0
30%	83.0	85.8	79.0	272.0	287.0	164.0	16.0	17.0	13.0
40%	53.0	50.8	55.0	210.0	188.0	86.0	10.0	8.0	5.0
50%	39.0	32.7	36.0	170.0	122.0	45.0	8.0	4.0	2.0
60%	26.0	21.8	24.0	138.0	80.0	21.0	5.2	2.0	1.0
70%	18.0	13.7	15.0	111.0	47.0	9.0	4.0	1.0	0.0
80%	13.0	8.5	8.0	94.0	23.0	2.0	2.0	0.0	0.0
85%	11.0	6.3	4.0	87.0	13.0	0.0	2.0	0.0	0.0
90%	8.6	4.1	1.0	80.0	6.0	0.0	1.0	0.0	0.0
95%	7.0	1.6	0.0	73.0	1.0	0.0	0.0	0.0	0.0
98%	5.5	0.0	0.0	62.0	0.0	0.0	0.0	0.0	0.0
99%	4.0	0.0	0.0	56.0	0.0	0.0	0.0	0.0	0.0
99.5%	2.5	0.0	0.0	52.0	0.0	0.0	0.0	0.0	0.0
99.8%	1.4	0.0	0.0	49.0	0.0	0.0	0.0	0.0	0.0
99.9%	0.9	0.0	0.0	48.0	0.0	0.0	0.0	0.0	0.0

Maximum values of both MDNF and DDNF are underestimated than USGS daily recorded flows, but overall flow regimes of MDNF more identical to the USGS flows than DDNF at the control point CP15. Maximum value of MDNF is overestimated than USGS daily recorded flows, and maximum value of DDNF is almost similar to the USGS flows at the control point 18. However, overall flow regimes of both MDNF and DDNF are totally different from the USGS flows at the control point 18. Maximum values of both MDNF and DDNF are similar to USGS daily recorded flows, but overall flow regimes of MDNF more similar to the USGS flows than DDNF at the control point CP28. In conclusion, MDNFs is better than DDNFs at the control points 15 and 28, but both are the same at the control point 18.

Table 6.41 Flow Frequency Metrics for Control Points CP15, CP18, and CP28

	CP15 (CFS)			CP18 (CFS)			CP28 (CFS)		
	USGS	MDNF	DDNF	USGS	MDNF	DDNF	USGS	MDNF	DDNF
Mean	1,619	1,533	1,533	43.3	35.8	35.8	103.1	173.2	173.2
Std Dev	3,602	2,650	3,106	61.2	151.6	121.3	375.8	417.2	542.4
Min	5.0	0.0	0.0	1.0	0.0	0.0	3.0	2.0	0.0
Max	129,000	48,176	62,687	2,690	3,562	2,680	15,900	14,291	15,441
0.1%	40,019	27,830	34,098	612.3	2,069	1,619	5,040	5,074	6,767
0.2%	30,455	24,869	27,544	549.1	1,729	1,215	3,557	4,142	5,439
0.5%	22,000	17,609	18,338	323.8	960.3	781.8	1,569	2,518	3,189
1%	16,700	12,752	14,197	210.2	695.8	594.9	1,040	1,925	2,594
2%	10,800	9,503	10,338	156.0	421.4	351.4	604.5	1,207	1,660
5%	5,128	5,470	6,007	131.0	155.0	167.1	277.0	592.8	729.8
10%	2,887	3,496	3,684	102.0	65.0	87.0	148.0	335.5	377.0
15%	2,160	2,588	2,588	83.3	34.0	52.0	113.0	251.0	247.0
20%	1,720	2,021	2,002	67.0	21.0	31.0	97.0	200.0	175.0
30%	1,320	1,410	1,338	43.0	9.0	14.0	82.0	139.0	94.0
40%	1,040	1,049	903.0	30.0	2.0	5.0	72.0	106.0	55.0
50%	824.0	768.0	590.0	24.0	0.0	1.0	60.0	79.0	29.0
60%	680.0	551.0	370.0	21.0	0.0	0.0	42.0	57.0	15.0
70%	555.0	364.0	220.0	16.0	0.0	0.0	23.0	38.0	6.0
80%	388.0	231.0	111.0	12.0	0.0	0.0	14.0	21.0	2.0
85%	310.0	163.0	70.0	11.0	0.0	0.0	12.0	15.0	1.0
90%	188.0	104.0	36.0	9.0	0.0	0.0	9.0	11.0	0.0
95%	94.0	51.0	6.0	7.0	0.0	0.0	7.0	7.0	0.0
98%	25.0	19.0	0.0	6.0	0.0	0.0	6.0	5.0	0.0
99%	10.0	11.0	0.0	5.0	0.0	0.0	5.0	3.2	0.0
99.5%	7.0	5.0	0.0	4.0	0.0	0.0	4.0	3.0	0.0
99.8%	5.0	1.0	0.0	4.0	0.0	0.0	4.0	2.0	0.0
99.9%	5.0	0.0	0.0	3.0	0.0	0.0	4.0	2.0	0.0

Maximum values of both MDNF and DDNF are similar to USGS daily recorded flows at the control points CP32, CP35, and CP37. Overall flow regimes of MDNF are better than DDNF at the control points CP32 and CP37, but, both are the same at the control point CP35.

Table 6.42 Flow Frequency Metrics for Control Points CP32, CP35, and CP37

	CP32 (CFS)			CP35 (CFS)			CP37 (CFS)		
	USGS	MDNF	DDNF	USGS	MDNF	DDNF	USGS	MDNF	DDNF
Mean	345.0	346.9	346.9	118.7	118.8	118.8	572.1	568.4	568.4
Std Dev	885.7	651.6	823.0	689.8	401.2	492.5	1,579	1,151	1,417
Min	19.0	1.2	0.0	0.0	0.0	0.0	2.0	4.0	0.0
Max	42,200	12,868	17,652	20,900	10,159	12,386	32,000	16,141	26,413
0.1%	12,141	7,713	9,633	10,482	5,794	7,081	23,404	13,783	18,576
0.2%	9,147	6,328	7,828	6,686	3,866	4,733	15,513	11,222	13,416
0.5%	5,044	4,368	5,530	4,447	2,390	2,947	10,622	8,492	10,125
1%	3,277	3,312	3,939	2,425	1,810	2,178	7,800	6,274	7,233
2%	2,050	2,221	2,545	1,040	1,074	1,220	4,889	3,959	4,505
5%	934.0	1,106	1,337	282.0	485.3	471.6	1,650	2,043	2,250
10%	538.0	687.8	772.6	101.0	225.9	204.5	826.0	1,198.0	1,186
15%	414.0	523.0	558.2	58.0	143.3	120.7	596.0	850.0	852.0
20%	351.0	432.5	434.7	44.0	104.9	84.0	488.0	633.0	644.0
30%	271.0	315.8	282.6	32.0	60.0	46.0	366.0	438.0	422.0
40%	228.0	235.1	182.8	25.0	38.4	25.6	299.0	330.0	279.0
50%	194.0	179.4	116.0	21.0	26.0	14.6	252.0	255.0	184.0
60%	163.0	133.7	69.6	18.0	18.0	7.3	208.0	191.0	119.0
70%	129.0	96.3	36.4	15.0	12.9	3.2	161.0	139.0	67.0
80%	102.0	63.0	11.5	12.0	8.9	0.8	128.0	99.0	27.0
85%	89.0	46.8	2.8	11.0	7.0	0.1	111.0	79.0	11.0
90%	79.0	33.9	0.0	9.0	5.3	0.0	94.0	61.0	1.0
95%	61.0	20.7	0.0	7.0	3.0	0.0	74.0	39.0	0.0
98%	47.0	9.5	0.0	5.0	1.3	0.0	52.0	23.0	0.0
99%	40.0	6.0	0.0	3.0	0.7	0.0	39.0	16.0	0.0
99.5%	35.0	4.9	0.0	1.0	0.5	0.0	21.0	10.0	0.0
99.8%	28.0	3.8	0.0	0.0	0.2	0.0	16.0	7.0	0.0
99.9%	25.0	2.1	0.0	0.0	0.0	0.0	8.0	5.0	0.0

Table 6.43 summarizes the comparative scores, made through the qualitative comparison of both flow frequency metrics at the control points.

Table 6.43 Comparative Evaluation of Flow Frequency Metrics for the GSA WAM

CP	Data Period	Comparative Score	
		USGS vs. MDNF	USGS vs. DDNF
<u>Total</u>		<u>20</u>	<u>4</u>
CP01	06/01/1939-12/31/1960	2	0
CP02	01/01/1934-12/31/1960	2	0
CP04	01/01/1934-12/31/1960	2	0
CP08	01/01/1934-12/31/1960	1	1
CP10	05/01/1939-12/31/1960	2	0
CP11	01/01/1934-12/31/1960	1	1
CP15	01/01/1934-12/31/1960	2	0
CP18	03/01/1939-12/31/1960	1	1
CP28	08/01/1939-12/31/1960	2	0
CP32	01/01/1934-12/31/1960	2	0
CP35	01/01/1934-12/31/1960	1	1
CP37	03/01/1939-12/31/1960	2	0

DHRAM method quantitatively evaluates how much the hydrologic characteristics of MDNF and DDNF are similar to or different from USGS daily recorded flows at 12 control points. Table 6.44 summarizes the scores of MDNF and DDNF based on the USGS daily recorded flows at 12 control points. Table 6.44 shows that MDNF has less impact points than DDNF at 10 of the 12 control points in total, and has the same points with DDNF at the control points, CP18 and CP37. This table also indicates that both MDNF and DDNF have slightly different hydrologic characteristics from USGS daily recorded flows at all the 12 control points.

Table 6.44 Impact points by the DHRAM Method for the GSA WAM

CP	Data Period	Impact Points	
		USGS vs. MDNF	USGS vs. DDNF
<u>Total</u>		<u>98</u>	<u>118</u>
CP01	06/01/1939-12/31/1960	7	8
CP02	01/01/1934-12/31/1960	6	9
CP04	01/01/1934-12/31/1960	7	8
CP08	01/01/1934-12/31/1960	6	7
CP10	05/01/1939-12/31/1960	8	11
CP11	01/01/1934-12/31/1960	8	9
CP15	01/01/1934-12/31/1960	9	11
CP18	03/01/1939-12/31/1960	15	15
CP28	08/01/1939-12/31/1960	9	15
CP32	01/01/1934-12/31/1960	6	7
CP35	01/01/1934-12/31/1960	9	10
CP37	03/01/1939-12/31/1960	8	8

The detailed evaluation sheets for the 12 control points are tabulated in Tables 6.45 to 6.54. The evaluation sheets, tabulated in Tables 6.45, 6.46, and 6.47, commonly indicate that minimum flows of both MDNF and DDNF are underestimated than USGS daily recorded flows. The average of high pulse duration is considerably different between MDNF and the USGS flows while the average of low pulse duration is obviously different between DDNF and the USGS flows at the control points CP01, CP02 and CP04 that are located upstream of the Guadalupe River.

Table 6.45 Evaluation Sheet by DHRAM Method for Control Point CP01

Control Point:	CP01		Period:	1939-1960						
IHA statistics group	USGS		MDNF		Absolute Chages		DDNF		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	113.2	0.9421	115	0.9194	1.6%	2.4%	114.9	0.9198	1.5%	2.4%
February	154	1.037	155.8	1.018	1.2%	1.8%	155.7	1.018	1.1%	1.8%
March	143.4	0.9987	145.3	0.9778	1.3%	2.1%	145.3	0.9783	1.3%	2.0%
April	199.5	1.017	201.5	0.9998	1.0%	1.7%	201.5	0.9998	1.0%	1.7%
May	233.4	1.167	235.5	1.151	0.9%	1.4%	235.5	1.152	0.9%	1.3%
June	152.9	0.8769	153.7	0.8719	0.5%	0.6%	153.7	0.8719	0.5%	0.6%
July	76.65	0.6992	77.53	0.6894	1.1%	1.4%	77.54	0.6889	1.2%	1.5%
August	71.27	1.773	72.06	1.754	1.1%	1.1%	72.08	1.753	1.1%	1.1%
September	103.3	1.22	103.7	1.212	0.4%	0.7%	103.7	1.212	0.4%	0.7%
October	204.8	1.386	205	1.384	0.1%	0.1%	205	1.385	0.1%	0.1%
November	114.2	1.014	114.4	1.013	0.2%	0.1%	114.3	1.013	0.1%	0.1%
December	130.2	0.9825	130.2	0.982	0.0%	0.1%	130.2	0.9821	0.0%	0.0%
					Average	0.8%			Average	0.8%
					Score	0			Score	0
Parameter Group #2										
1-day minimum	20.41	0.9874	0.5909	1.93	97.1%	95.5%	0.5909	1.623	97.1%	64.4%
3-day minimum	21.18	0.9717	0.7273	1.689	96.6%	73.8%	0.7121	1.403	96.6%	44.4%
7-day minimum	22.74	0.9832	1.338	1.706	94.1%	73.5%	1.078	1.419	95.3%	44.3%
30-day minimum	28.48	0.9251	11.04	1.406	61.2%	52.0%	7.535	0.7511	73.5%	18.8%
90-day minimum	48.54	0.8482	49	0.8837	0.9%	4.2%	45.43	0.9016	6.4%	6.3%
1-day maximum	5120	1.145	3255	0.8788	36.4%	23.2%	6505	0.879	27.1%	23.2%
3-day maximum	2542	0.9565	1795	0.8127	29.4%	15.0%	2990	0.8623	17.6%	9.8%
7-day maximum	1348	0.8574	1159	0.7554	14.0%	11.9%	1594	0.8304	18.2%	3.1%
30-day maximum	496.9	0.7719	494.1	0.7703	0.6%	0.2%	552.8	0.7792	11.2%	0.9%
90-day maximum	272.8	0.7155	277.2	0.7098	1.6%	0.8%	284.6	0.7099	4.3%	0.8%
					Average	43.2%			Average	44.7%
					Score	1			Score	2
Parameter Group #3										
Date of minimum	204.8	0.1372	144.5	0.2363	29.4%	72.2%	135.1	0.2325	34.0%	69.5%
Date of maximum	181.4	0.1945	177.3	0.2039	2.3%	4.8%	171.2	0.2238	5.6%	15.1%
					Average	15.9%			Average	19.8%
					Score	0			Score	0
Parameter Group #4										
Low pulse count	4.5	0.9837	6.545	0.5731	45.4%	41.7%	8.5	0.4277	88.9%	56.5%
Low pulse duration	17.72	0.7425	12.41	0.4822	30.0%	35.1%	9.8	0.4537	44.7%	38.9%
High pulse count	3.727	0.7414	3.545	0.7057	4.9%	4.8%	4.864	0.6784	30.5%	8.5%
High pulse duration	2.113	0.3375	4.234	0.7006	100.4%	107.6%	1.816	0.4738	14.1%	40.4%
					Average	45.2%			Average	44.5%
					Score	2			Score	2
Parameter Group #5										
Rise rate	137.4	0.7976	95.29	0.7988	30.6%	0.2%	257	0.7273	87.0%	8.8%
Fall rate	-51.46	-0.7438	-33.75	-0.6859	34.4%	7.8%	-75.99	-0.7175	47.7%	3.5%
Number of reversals	86.91	0.1602	57.91	0.2281	33.4%	42.4%	79.27	0.2014	8.8%	25.7%
					Average	32.8%			Average	47.8%
					Score	1			Score	2
Total Point					7	Total Point				
Classification					3	Classification				
note:					Moderate risk of impact	note:				

Table 6.46 Evaluation Sheet by DHRAM Method for Control Point CP02

Control Point:	CP02		Period:	1934-1960						
IHA statistics group	USGS		DCFM		Absolute Chages		DCFD		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	225.2	1.1	225.4	1.099	0.1%	0.1%	225.4	1.099	0.1%	0.1%
February	272.7	1.168	272.8	1.167	0.0%	0.1%	272.8	1.167	0.0%	0.1%
March	267.9	1.123	268.1	1.121	0.1%	0.2%	268.1	1.122	0.1%	0.1%
April	335.2	1.144	335.6	1.142	0.1%	0.2%	335.6	1.142	0.1%	0.2%
May	460	1.126	460.3	1.125	0.1%	0.1%	460.3	1.125	0.1%	0.1%
June	426.4	1.782	427	1.779	0.1%	0.2%	427	1.779	0.1%	0.2%
July	174.3	1.33	174.9	1.324	0.3%	0.5%	174.9	1.324	0.3%	0.5%
August	104.9	1.46	105.5	1.453	0.6%	0.5%	105.5	1.453	0.6%	0.5%
September	395	2.152	395.4	2.15	0.1%	0.1%	395.4	2.15	0.1%	0.1%
October	324.9	1.365	325.1	1.364	0.1%	0.1%	325.1	1.364	0.1%	0.1%
November	201.9	1.151	202	1.15	0.0%	0.1%	202	1.15	0.0%	0.1%
December	227.3	0.9571	227.4	0.9568	0.0%	0.0%	227.4	0.9568	0.0%	0.0%
				Average	0.1%	0.2%		Average	0.1%	0.2%
				Score	0	0		Score	0	0
Parameter Group #2										
1-day minimum	35.52	0.9015	3.915	1.682	89.0%	86.6%	0	0	100.0%	100.0%
3-day minimum	36.37	0.9006	4.748	1.616	86.9%	79.4%	0.02469	3.603	99.9%	300.1%
7-day minimum	38.25	0.9035	7.844	1.764	79.5%	95.2%	0.2011	2.079	99.5%	130.1%
30-day minimum	47.92	0.8804	32.55	1.015	32.1%	15.3%	10.48	1.366	78.1%	55.2%
90-day minimum	81.29	0.9154	82.02	0.9638	0.9%	5.3%	72.73	1.003	10.5%	9.6%
1-day maximum	10280	1.484	6683	1.342	35.0%	9.6%	15850	1.162	54.2%	21.7%
3-day maximum	5388	1.265	3862	1.294	28.3%	2.3%	7237	1.197	34.3%	5.4%
7-day maximum	2915	1.122	2470	1.186	15.3%	5.7%	3652	1.086	25.3%	3.2%
30-day maximum	1122	1.051	1102	1.05	1.8%	0.1%	1206	0.9979	7.5%	5.1%
90-day maximum	602.6	0.9058	596.8	0.903	1.0%	0.3%	622.1	0.9007	3.2%	0.6%
				Average	37.0%	30.0%		Average	51.3%	63.1%
				Score	1	1		Score	2	1
Parameter Group #3										
Date of minimum	203.9	0.1904	160	0.2248	21.5%	18.1%	118	0.2218	42.1%	16.5%
Date of maximum	182.6	0.2395	170.4	0.214	6.7%	10.6%	176.4	0.229	3.4%	4.4%
				Average	14.1%	14.4%		Average	22.8%	10.4%
				Score	0	0		Score	1	0
Parameter Group #4										
Low pulse count	4.037	0.9382	6.593	0.6201	63.3%	33.9%	7.556	0.4158	87.2%	55.7%
Low pulse duration	19.83	0.7788	12.63	0.6983	36.3%	10.3%	11.67	0.4766	41.1%	38.8%
High pulse count	3.407	0.9037	2.296	0.9496	32.6%	5.1%	4.556	0.8851	33.7%	2.1%
High pulse duration	2.297	0.5707	5.576	0.7008	142.8%	22.8%	1.556	0.3678	32.3%	35.6%
				Average	68.7%	18.0%		Average	48.6%	33.0%
				Score	3	0		Score	2	1
Parameter Group #5										
Rise rate	310.3	1.053	147	1.137	52.6%	8.0%	502.5	0.9616	61.9%	8.7%
Fall rate	-94.93	-1.094	-54.31	-1.042	42.8%	4.8%	-178.5	-0.9036	88.0%	17.4%
Number of reversals	75.63	0.1886	64.63	0.1212	14.5%	35.7%	75.52	0.1901	0.1%	0.8%
				Average	36.7%	16.2%		Average	50.0%	9.0%
				Score	1	0		Score	2	0
Total Point					6	Total Point				
Classification					3	Classification				
note:					Moderate risk of impact	note:				

Table 6.47 Evaluation Sheet by DHRAM Method for Control Point CP04

Control Point:	CP04	Period:	1934-1960								
IHA statistics group	USGS		DCFM		Absolute Chages		DCFD		Absolute Chages		
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)	
Parameter Group #1											
January	314.3	1.088	306.7	1.112	2.4%	2.2%	306.7	1.112	2.4%	2.2%	
February	368.2	1.088	360.7	1.106	2.0%	1.7%	360.7	1.106	2.0%	1.7%	
March	374.3	1.07	366.7	1.087	2.0%	1.6%	366.7	1.087	2.0%	1.6%	
April	449.9	1.096	442	1.113	1.8%	1.6%	441.9	1.113	1.8%	1.6%	
May	589.8	1.107	581.7	1.12	1.4%	1.2%	581.7	1.12	1.4%	1.2%	
June	533.3	1.623	525.1	1.648	1.5%	1.5%	525.1	1.648	1.5%	1.5%	
July	267.9	1.406	259.7	1.443	3.1%	2.6%	259.7	1.443	3.1%	2.6%	
August	150.2	1.299	141.7	1.374	5.7%	5.8%	141.7	1.375	5.7%	5.9%	
September	503.3	1.909	494.7	1.943	1.7%	1.8%	494.6	1.943	1.7%	1.8%	
October	425.4	1.283	416.5	1.312	2.1%	2.3%	416.5	1.312	2.1%	2.3%	
November	285.8	1.191	276.9	1.228	3.1%	3.1%	276.9	1.228	3.1%	3.1%	
December	317.4	0.9681	309.6	0.9891	2.5%	2.2%	309.6	0.9891	2.5%	2.2%	
Average Score					2.4%	2.3%	Average Score				
					0	0					
Parameter Group #2											
1-day minimum	56.59	0.9031	5.93	1.83	89.5%	102.2%	0	0	100.0%	100.0%	
3-day minimum	57.44	0.9051	7.20	1.66	87.5%	83.1%	0.012	5.196	100.0%	474.1%	
7-day minimum	59.93	0.9102	12.07	1.76	79.9%	93.1%	0.291	2.523	99.5%	177.2%	
30-day minimum	73.72	0.8857	43.79	1.05	40.6%	18.7%	14.86	1.29	79.8%	45.6%	
90-day minimum	118.2	0.8834	111.20	0.98	5.9%	11.2%	98.78	0.9969	16.4%	12.8%	
1-day maximum	10930	1.299	5260.00	1.33	51.9%	2.5%	12460	1.177	14.0%	9.4%	
3-day maximum	6277	1.247	4160.00	1.24	33.7%	0.7%	7226	1.075	15.1%	13.8%	
7-day maximum	3459	1.087	2928.00	1.12	15.4%	3.2%	4315	0.9913	24.7%	8.8%	
30-day maximum	1402	0.9639	1374.00	0.97	2.0%	0.1%	1511	0.9263	7.8%	3.9%	
90-day maximum	788.2	0.8443	771.40	0.85	2.1%	0.4%	805.5	0.848	2.2%	0.4%	
Average Score					40.8%	31.5%	Average Score				
					1	1					
Parameter Group #3											
Date of minimum	218.8	0.2055	152	0.2333	30.5%	13.5%	102.9	0.2095	53.0%	1.9%	
Date of maximum	185.7	0.2595	180.7	0.2292	2.7%	11.7%	171.4	0.2347	7.7%	9.6%	
Average Score					16.6%	12.6%	Average Score				
					0	0					
Parameter Group #4											
Low pulse count	3.111	1.299	5.333	0.686	71.4%	47.2%	7.593	0.4387	144.1%	66.2%	
Low pulse duration	32.46	0.9283	15.03	0.6472	53.7%	30.3%	11.35	0.434	65.0%	53.2%	
High pulse count	3.37	0.8715	2.778	0.9951	17.6%	14.2%	4.074	0.8636	20.9%	0.9%	
High pulse duration	2.744	0.5501	6.848	0.4423	149.6%	19.6%	3.046	0.4195	11.0%	23.7%	
Average Score					73.1%	27.8%	Average Score				
					3	0					
Parameter Group #5											
Rise rate	337.6	1.092	128.1	0.9568	62.1%	12.4%	400.3	0.9231	18.6%	15.5%	
Fall rate	-98.77	-1.024	-48.05	-0.869	51.4%	15.1%	-137.4	-0.8373	39.1%	18.2%	
Number of reversals	77.81	0.1695	60.37	0.153	22.4%	9.7%	70.63	0.1497	9.2%	11.7%	
Average Score					45.3%	12.4%	Average Score				
					2	0					
Total Point					7		Total Point				
Classification					3		Classification				
note:					Moderate risk of impact		note:				
Total Point					8		Total Point				
Classification					3		Classification				
note:					Moderate risk of impact		note:				

The minimum flows and low pulse durations of both MDNF and DDNF are underestimated while both the occurrence numbers of low pulse are overestimated than the USGS flows at the control points CP08 and CP10 as tabulated in Tables 6.48 and 6.49.

Table 6.48 Evaluation Sheet by DHRAM Method for Control Point CP08

Control Point:	CP08	Period:	1934-1960							
IHA statistics group	USGS		DCFM		Absolute Chages		DCFD		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	94.35	1.183	94.33	1.183	0.0%	0.0%	94.33	1.183	0.0%	0.0%
February	124.9	1.2	124.9	1.2	0.0%	0.0%	124.9	1.2	0.0%	0.0%
March	135.5	1.193	135.5	1.193	0.0%	0.0%	135.5	1.193	0.0%	0.0%
April	175	1.209	175.1	1.209	0.1%	0.0%	175.1	1.209	0.1%	0.0%
May	181.8	1.334	181.9	1.334	0.1%	0.0%	181.9	1.334	0.1%	0.0%
June	137.8	1.326	137.8	1.326	0.0%	0.0%	137.8	1.326	0.0%	0.0%
July	69.37	1.595	69.36	1.595	0.0%	0.0%	69.39	1.595	0.0%	0.0%
August	37.25	0.8768	37.24	0.8785	0.0%	0.2%	37.26	0.8782	0.0%	0.2%
September	134.6	2.09	134.5	2.09	0.1%	0.0%	134.5	2.09	0.1%	0.0%
October	108.9	1.595	108.9	1.595	0.0%	0.0%	108.9	1.596	0.0%	0.1%
November	90.71	1.424	90.69	1.425	0.0%	0.1%	90.68	1.425	0.0%	0.1%
December	101.5	1.241	101.5	1.242	0.0%	0.1%	101.5	1.242	0.0%	0.1%
					Average	0.0%			Average	0.0%
					Score	0			Score	0
Parameter Group #2										
1-day minimum	15.59	0.7717	2.189	1.185	86.0%	53.6%	1.741	2.512	88.8%	225.5%
3-day minimum	15.95	0.7614	2.375	1.173	85.1%	54.1%	2.284	2.466	85.7%	223.9%
7-day minimum	16.64	0.7489	3.104	1.229	81.3%	64.1%	2.852	2.244	82.9%	199.6%
30-day minimum	19.2	0.7163	8.504	0.8047	55.7%	12.3%	10.47	1.098	45.5%	53.3%
90-day minimum	29.26	0.8612	27.75	0.9032	5.2%	4.9%	27.47	0.891	6.1%	3.5%
1-day maximum	4985	1.452	4128	1.801	17.2%	24.0%	8752	1.169	75.6%	19.5%
3-day maximum	2274	1.206	2150	1.361	5.5%	12.9%	3253	1.095	43.1%	9.2%
7-day maximum	1257	1.055	1222	1.127	2.8%	6.8%	1528	1.007	21.6%	4.5%
30-day maximum	477.9	0.8241	472	0.8028	1.2%	2.6%	494.2	0.8095	3.4%	1.8%
90-day maximum	264.6	0.7941	263.9	0.783	0.3%	1.4%	267	0.7966	0.9%	0.3%
					Average	34.0%			Average	45.3%
					Score	1			Score	2
Parameter Group #3										
Date of minimum	219.3	0.266	279.9	0.3057	27.6%	14.9%	137.8	0.2542	37.2%	4.4%
Date of maximum	185.6	0.2422	173.1	0.2679	6.7%	10.6%	179	0.2599	3.6%	7.3%
					Average	17.2%			Average	20.4%
					Score	0			Score	1
Parameter Group #4										
Low pulse count	4	1.183	7.37	0.6971	84.3%	41.1%	9.037	0.7737	125.9%	34.6%
Low pulse duration	20.89	0.7651	11.04	0.4069	47.2%	46.8%	9.436	0.5306	54.8%	30.6%
High pulse count	3.593	0.911	2.259	0.7724	37.1%	15.2%	2.259	0.8732	37.1%	4.1%
High pulse duration	2.182	0.5113	4.079	0.6849	86.9%	34.0%	1.512	0.5065	30.7%	0.9%
					Average	63.9%			Average	62.1%
					Score	2			Score	2
Parameter Group #5										
Rise rate	189	1.059	67.32	1.281	64.4%	21.0%	144.4	1.04	23.6%	1.8%
Fall rate	-53.24	-0.9952	-26.05	-1.139	51.1%	14.4%	-65.32	-0.9579	22.7%	3.7%
Number of reversals	70.67	0.1352	90.07	0.1268	27.5%	6.2%	84.74	0.1756	19.9%	29.9%
					Average	47.6%			Average	22.1%
					Score	2			Score	1
Total Point					6	Total Point				
Classification					3	Classification				
note:					Moderate risk of impact	note:				

Table 6.49 Evaluation Sheet by DHRAM Method for Control Point CP10

Control Point:	CP10	Period:	1939-1960							
IHA statistics group	USGS		DCFM		Absolute Chages		DCFD		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	274.8	0.8497	271	0.8692	1.4%	2.3%	268.5	0.8753	2.3%	3.0%
February	358.2	1.034	354.4	1.047	1.1%	1.3%	351.9	1.056	1.8%	2.1%
March	343.4	0.9242	339.6	0.9351	1.1%	1.2%	337.1	0.9425	1.8%	2.0%
April	512.3	0.947	509.7	0.9479	0.5%	0.1%	507.2	0.9546	1.0%	0.8%
May	427.3	0.979	416.9	1.008	2.4%	3.0%	416.9	1.008	2.4%	3.0%
June	392	0.8379	382.5	0.8723	2.4%	4.1%	382.5	0.8722	2.4%	4.1%
July	224.6	0.8876	211.7	0.9521	5.7%	7.3%	211.7	0.9523	5.7%	7.3%
August	165.4	0.5871	151.5	0.6528	8.4%	11.2%	151.5	0.6527	8.4%	11.2%
September	328.1	1.231	316.1	1.292	3.7%	5.0%	316.1	1.292	3.7%	5.0%
October	354	1.061	344.5	1.115	2.7%	5.1%	344.5	1.115	2.7%	5.1%
November	293.9	0.8876	281.4	0.9392	4.3%	5.8%	281.4	0.9391	4.3%	5.8%
December	310.4	0.8937	298.9	0.9403	3.7%	5.2%	298.9	0.9402	3.7%	5.2%
				Average	3.1%	4.3%		Average	3.3%	4.5%
				Score	0	0		Score	0	0
Parameter Group #2										
1-day minimum	101.3	0.4697	1.227	2.011	98.8%	328.1%	0	0	100.0%	100.0%
3-day minimum	108.6	0.4296	1.606	1.889	98.5%	339.7%	0	0	100.0%	100.0%
7-day minimum	113.1	0.4119	3.013	2.201	97.3%	434.4%	0.01948	3.429	100.0%	732.5%
30-day minimum	121	0.4089	22.34	1.018	81.5%	149.0%	9.994	1.412	91.7%	245.3%
90-day minimum	145.4	0.4777	116.9	0.5931	19.6%	24.2%	111	0.5737	23.7%	20.1%
1-day maximum	8509	0.7891	6544	0.8043	23.1%	1.9%	7728	0.7278	9.2%	7.8%
3-day maximum	4704	0.7619	4187	0.729	11.0%	4.3%	5185	0.6848	10.2%	10.1%
7-day maximum	2582	0.7717	2757	0.726	6.8%	5.9%	3200	0.6937	23.9%	10.1%
30-day maximum	1010	0.6812	1063	0.6497	5.2%	4.6%	1106	0.6611	9.5%	3.0%
90-day maximum	615.3	0.6854	626.6	0.6593	1.8%	3.8%	646.4	0.6836	5.1%	0.3%
				Average	44.4%	129.6%		Average	47.3%	122.9%
				Score	2	2		Score	2	2
Parameter Group #3										
Date of minimum	219.7	0.2805	158.6	0.2571	27.8%	8.3%	95.23	0.2202	56.7%	21.5%
Date of maximum	176.7	0.1774	158	0.158	10.6%	10.9%	155.1	0.178	12.2%	0.3%
				Average	19.2%	9.6%		Average	34.4%	10.9%
				Score	0	0		Score	1	0
Parameter Group #4										
Low pulse count	8.5	1.295	9	0.5676	5.9%	56.2%	10.5	0.4406	23.5%	66.0%
Low pulse duration	16.86	1.271	9.979	0.2988	40.8%	76.5%	8.443	0.3157	49.9%	75.2%
High pulse count	5.136	0.739	5.227	0.6678	1.8%	9.6%	8.318	0.6526	62.0%	11.7%
High pulse duration	1.829	0.3289	3.553	0.5056	94.3%	53.7%	2.493	0.3996	36.3%	21.5%
				Average	35.7%	49.0%		Average	42.9%	43.6%
				Score	1	1		Score	1	1
Parameter Group #5										
Rise rate	263.3	0.8774	288.3	0.6094	9.5%	30.5%	630.7	0.5039	139.5%	42.6%
Fall rate	-116.4	-0.6698	-70.66	-0.5388	39.3%	19.6%	-135.7	-0.5045	16.6%	24.7%
Number of reversals	141.5	0.3499	63.55	0.1713	55.1%	51.0%	75.23	0.1805	46.8%	48.4%
				Average	34.6%	33.7%		Average	67.7%	38.6%
				Score	1	1		Score	3	1
				Total Point	8		Total Point	11		
				Classification	3		Classification	4		
				note:	Moderate risk of impact		note:	High risk of impact		

The amounts and frequencies of low flow regimes of both MDNF and DDNF are slightly different from the USGS flows at the control point CP11 as tabulated in Table 6.50.

Table 6.50 Evaluation Sheet by DHRAM Method for Control Point CP11

Control Point:	CP11		Period:	1934-1960						
IHA statistics group	USGS		DCFM		Absolute Chages		DCFD		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	70.43	1.492	69.5	1.513	1.3%	1.4%	69.52	1.512	1.3%	1.3%
February	79.73	1.358	78.76	1.375	1.2%	1.3%	78.73	1.375	1.3%	1.3%
March	75.12	1.666	74.24	1.686	1.2%	1.2%	74.24	1.686	1.2%	1.2%
April	218.4	1.348	217.4	1.354	0.5%	0.4%	217.4	1.354	0.5%	0.4%
May	107.4	1.367	106.5	1.378	0.8%	0.8%	106.5	1.379	0.8%	0.9%
June	133.7	1.341	132.8	1.351	0.7%	0.7%	132.8	1.35	0.7%	0.7%
July	120.4	3.27	119.6	3.292	0.7%	0.7%	119.6	3.292	0.7%	0.7%
August	35.45	3.117	34.69	3.183	2.1%	2.1%	34.7	3.182	2.1%	2.1%
September	67.19	1.882	66.29	1.907	1.3%	1.3%	66.32	1.906	1.3%	1.3%
October	105.9	2.44	105.1	2.457	0.8%	0.7%	105.1	2.457	0.8%	0.7%
November	66.21	1.755	65.39	1.776	1.2%	1.2%	65.38	1.776	1.3%	1.2%
December	71.87	1.638	70.97	1.659	1.3%	1.3%	70.98	1.658	1.2%	1.2%
					Average	1.1%		Average	1.1%	1.1%
					Score	0		Score	0	0
Parameter Group #2										
1-day minimum	1.778	1.096	0	0	100.0%	100.0%	0	0	100.0%	100.0%
3-day minimum	1.914	1.085	0	0	100.0%	100.0%	0	0	100.0%	100.0%
7-day minimum	2.074	1.055	0	0	100.0%	100.0%	0	0	100.0%	100.0%
30-day minimum	2.736	1.013	0.7988	1.779	70.8%	75.6%	0.3963	2.121	85.5%	109.4%
90-day minimum	10.66	1.5	9.986	1.858	6.3%	23.9%	10.61	1.953	0.5%	30.2%
1-day maximum	7077	1.196	4872	1.059	31.2%	11.5%	5363	0.9858	24.2%	17.6%
3-day maximum	3565	0.9818	2930	0.9104	17.8%	7.3%	2966	0.8759	16.8%	10.8%
7-day maximum	1781	0.9028	1732	0.8129	2.8%	10.0%	1693	0.7848	4.9%	13.1%
30-day maximum	540.8	0.8354	561.1	0.8269	3.8%	1.0%	557.8	0.8234	3.1%	1.4%
90-day maximum	242.6	0.7835	240.7	0.7818	0.8%	0.2%	240.6	0.7772	0.8%	0.8%
					Average	43.3%		Average	43.6%	48.3%
					Score	1		Score	1	1
Parameter Group #3										
Date of minimum	196.3	0.1566	61	0.1858	68.9%	18.6%	19.11	0.05699	90.3%	63.6%
Date of maximum	166.1	0.2462	153.4	0.2139	7.6%	13.1%	155.6	0.2298	6.3%	6.7%
					Average	38.3%		Average	48.3%	35.1%
					Score	1		Score	2	1
Parameter Group #4										
Low pulse count	5.481	1.001	0	0	100.0%	100.0%	0	0	100.0%	100.0%
Low pulse duration	16.32	0.6613	0	0	100.0%	100.0%	0	0	100.0%	100.0%
High pulse count	4.593	0.7716	3.852	0.7081	16.1%	8.2%	4.185	0.744	8.9%	3.6%
High pulse duration	1.928	0.4008	3.26	0.3425	69.1%	14.5%	3.063	0.3461	58.9%	13.6%
					Average	71.3%		Average	66.9%	54.3%
					Score	3		Score	2	1
Parameter Group #5										
Rise rate	255.3	0.6601	231.4	0.8451	9.4%	28.0%	315.9	0.7496	23.7%	13.6%
Fall rate	-127.7	-0.6357	-56.51	-0.7951	55.7%	25.1%	-69.38	-0.7144	45.7%	12.4%
Number of reversals	70.89	0.2635	59.15	0.2731	16.6%	3.6%	60.63	0.2774	14.5%	5.3%
					Average	27.2%		Average	28.0%	10.4%
					Score	1		Score	1	0
Total Point					8		Total Point			
Classification					3		Classification			
note:					Moderate risk of impact		note:			

Amounts of monthly flows and daily minimum flows of both MDNF and DDNF are underestimated than the USGS flows. The Flow frequencies of both MDNF and DDNF also slightly different from the USGS flows at the control point CP15 as tabulated in Table 6.51. These are attributed to effluent from spring water sources within the basin.

Table 6.51 Evaluation Sheet by DHRAM Method for Control Point CP15

Control Point:	CP15		Period:		1934-1960						
IHA statistics group	USGS		DCFM		Absolute Chages		DCFD		Absolute Chages		
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)	
Parameter Group #1											
January	1468	1.144	1207	0.8742	17.8%	23.6%	1207	0.8742	17.8%	23.6%	
February	1365	0.8685	1462	1.129	7.1%	30.0%	1462	1.129	7.1%	30.0%	
March	1251	0.8689	1380	0.8469	10.3%	2.5%	1380	0.8469	10.3%	2.5%	
April	1556	1.093	1770	0.8974	13.8%	17.9%	1770	0.8974	13.8%	17.9%	
May	1389	0.8571	2567	1.109	84.8%	29.4%	2567	1.109	84.8%	29.4%	
June	2111	1.001	1857	1.028	12.0%	2.7%	1857	1.028	12.0%	2.7%	
July	2468	1.023	1870	1.957	24.2%	91.3%	1870	1.958	24.2%	91.4%	
August	2130	0.9649	580.7	0.7362	72.7%	23.7%	580.7	0.7362	72.7%	23.7%	
September	1568	1.703	1363	1.133	13.1%	33.5%	1363	1.133	13.1%	33.5%	
October	798.1	0.7083	1697	1.338	112.6%	88.9%	1697	1.338	112.6%	88.9%	
November	1503	1.092	1418	1.323	5.7%	21.2%	1418	1.323	5.7%	21.2%	
December	1796	1.302	1232	0.9331	31.4%	28.3%	1232	0.9331	31.4%	28.3%	
				Average Score	33.8%	32.7%		Average Score	33.8%	32.8%	
					1	1			1	1	
Parameter Group #2											
1-day minimum	358.7	0.6177	68.74	1.107	80.8%	79.2%	4.185	2.463	98.8%	298.7%	
3-day minimum	385.1	0.6021	76.32	1.073	80.2%	78.2%	6.877	2.404	98.2%	299.3%	
7-day minimum	403.3	0.6004	90.42	1.054	77.6%	75.5%	16.03	2.336	96.0%	289.1%	
30-day minimum	465.4	0.6071	238	0.7826	48.9%	28.9%	147.7	0.8664	68.3%	42.7%	
90-day minimum	617.6	0.6149	509.3	0.6684	17.5%	8.7%	486.2	0.6935	21.3%	12.8%	
1-day maximum	25110	1.043	12540	0.9193	50.1%	11.9%	15870	0.9086	36.8%	12.9%	
3-day maximum	20650	1.042	11670	0.904	43.5%	13.2%	14420	0.9256	30.2%	11.2%	
7-day maximum	14280	0.9476	10410	0.8888	27.1%	6.2%	12490	0.9297	12.5%	1.9%	
30-day maximum	5650	0.797	5395	0.8532	4.5%	7.1%	5624	0.8434	0.5%	5.8%	
90-day maximum	3126	0.7162	3120	0.758	0.2%	5.8%	3169	0.7486	1.4%	4.5%	
				Average Score	43.0%	31.5%		Average Score	46.4%	97.9%	
					1	1			2	2	
Parameter Group #3											
Date of minimum	309.4	0.1777	192.8	0.2344	37.7%	31.9%	155.4	0.2659	49.8%	49.6%	
Date of maximum	210.3	0.2703	159.9	0.2031	24.0%	24.9%	157.3	0.2119	25.2%	21.6%	
				Average Score	30.8%	28.4%		Average Score	37.5%	35.6%	
					1	0			1	1	
Parameter Group #4											
Low pulse count	5.481	1.189	4.889	0.5739	10.8%	51.7%	8.741	0.3894	59.5%	67.2%	
Low pulse duration	17.39	0.7503	18.16	1.029	4.4%	37.1%	9.595	0.5909	44.8%	21.2%	
High pulse count	4.185	0.8768	3	0.8165	28.3%	6.9%	4.333	0.8321	3.5%	5.1%	
High pulse duration	4.287	0.4809	8.526	0.621	98.9%	29.1%	6.129	0.3641	43.0%	24.3%	
				Average Score	35.6%	31.2%		Average Score	37.7%	29.5%	
					1	1			1	1	
Parameter Group #5											
Rise rate	538.9	0.8099	305.6	0.7324	43.3%	9.6%	503.9	0.6751	6.5%	16.6%	
Fall rate	-369.5	-0.7123	-143.6	-0.6956	61.1%	2.3%	-247.2	-0.6234	33.1%	12.5%	
Number of reversals	152.1	0.1857	52	0.1402	65.8%	24.5%	62.93	0.1175	58.6%	36.7%	
				Average Score	56.7%	12.1%		Average Score	32.7%	22.0%	
					2	0			1	0	
Total Point					9		Total Point				
Classification					3		Classification				
note:					Moderate risk of impact		note:				

Hydrological characteristics of both MDNF and DDNF are totally different from the USGS flows at the control point CP18 as tabulated in Table 6.52. The control point CP18 is located on San Antonio River at San Antonio city. These differences may be attributed to various water uses in San Antonio city.

Table 6.52 Evaluation Sheet by DHRAM Method for Control Point CP18

Control Point:	CP18		Period:	1939-1960								
IHA statistics group	USGS		DCFM		Absolute Chages		DCFD		Absolute Chages			
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)		
Parameter Group #1												
January	43.65	0.9154	32.81	0.8811	24.8%	3.7%	32.83	0.8804	24.8%	3.8%		
February	47.5	0.8501	36.27	0.8138	23.6%	4.3%	36.3	0.8131	23.6%	4.4%		
March	45.81	0.8384	36.85	0.7262	19.6%	13.4%	36.87	0.7261	19.5%	13.4%		
April	51.96	0.8076	44.61	0.7576	14.1%	6.2%	44.64	0.7574	14.1%	6.2%		
May	50.65	0.7464	43.33	0.6792	14.5%	9.0%	43.38	0.6779	14.4%	9.2%		
June	41.65	0.7527	35.56	0.6754	14.6%	10.3%	35.59	0.6752	14.5%	10.3%		
July	30.93	0.7331	26.15	0.6695	15.5%	8.7%	26.17	0.6696	15.4%	8.7%		
August	27.33	0.5282	21.87	0.5534	20.0%	4.8%	21.9	0.5533	19.9%	4.8%		
September	43.39	1.123	38.33	1.134	11.7%	1.0%	38.37	1.133	11.6%	0.9%		
October	48.74	0.9988	41.73	0.9836	14.4%	1.5%	41.77	0.9831	14.3%	1.6%		
November	44.04	1	34.66	0.992	21.3%	0.8%	34.69	0.9917	21.2%	0.8%		
December	44.96	0.9032	34.89	0.8693	22.4%	3.8%	34.92	0.8682	22.3%	3.9%		
					Average	18.0%	5.6%			Average	18.0%	5.7%
					Score	0	0			Score	0	0
Parameter Group #2												
1-day minimum	12.09	0.598	0	0	100.0%	100.0%	0	0	100.0%	100.0%		
3-day minimum	13.77	0.5207	0	0	100.0%	100.0%	0	0	100.0%	100.0%		
7-day minimum	14.75	0.4858	0	0	100.0%	100.0%	0	0	100.0%	100.0%		
30-day minimum	17.3	0.4497	1.23	1.771	92.9%	293.8%	1.686	1.713	90.3%	280.9%		
90-day minimum	23.46	0.5636	14.93	0.7231	36.4%	28.3%	15.04	0.709	35.9%	25.8%		
1-day maximum	502.9	1.059	1469	0.5749	192.1%	45.7%	1057	0.6394	110.2%	39.6%		
3-day maximum	276.1	1.065	618.2	0.6348	123.9%	40.4%	540	0.6326	95.6%	40.6%		
7-day maximum	161.9	0.9241	305.8	0.6953	88.9%	24.8%	292.3	0.7025	80.5%	24.0%		
30-day maximum	92.55	0.77	106.7	0.6314	15.3%	18.0%	105.9	0.6489	14.4%	15.7%		
90-day maximum	72.72	0.7446	67.32	0.6496	7.4%	12.8%	67.65	0.6536	7.0%	12.2%		
					Average	85.7%	76.4%			Average	73.4%	73.9%
					Score	3	1			Score	3	1
Parameter Group #3												
Date of minimum	186.5	0.345	4.273	0.01753	97.7%	94.9%	5.591	0.02164	97.0%	93.7%		
Date of maximum	203.5	0.2472	208.6	0.2364	2.5%	4.4%	200.1	0.2673	1.7%	8.1%		
					Average	50.1%	49.6%			Average	49.3%	50.9%
					Score	2	1			Score	2	1
Parameter Group #4												
Low pulse count	10.82	1.159	0	0	100.0%	100.0%	0	0	100.0%	100.0%		
Low pulse duration	6.025	0.78			100.0%	100.0%			100.0%	100.0%		
High pulse count	5.773	0.7483	10.95	0.4268	89.7%	43.0%	10.73	0.4171	85.9%	44.3%		
High pulse duration	4.895	1.626	1.298	0.2279	73.5%	86.0%	1.669	0.3232	65.9%	80.1%		
					Average	90.8%	82.2%			Average	87.9%	81.1%
					Score	3	1			Score	3	1
Parameter Group #5												
Rise rate	15.58	0.6559	192.1	0.486	1133.0%	25.9%	148.1	0.5124	850.6%	21.9%		
Fall rate	-12.49	-0.5248	-55.18	-0.4824	341.8%	8.1%	-36.89	-0.4925	195.4%	6.2%		
Number of reversals	159.8	0.09928	64.32	0.189	59.7%	90.4%	64.27	0.187	59.8%	88.4%		
					Average	511.5%	41.5%			Average	368.6%	38.8%
					Score	3	1			Score	3	1
Total Point					15		Total Point					
Classification					4		Classification					
note:					High risk of impact		note:					

Monthly flows of both MDNF and DDNF are greater than the USGS flows at the control point CP28. The daily minimum flows of both MDNF and DDNF are smaller, and the daily maximum flows of both are bigger than the USGS flows at the control point as tabulated in Table 6.53. The control point CP 28 is located downstream of Median Lake. Thus, these differences are attributed to the operation of Medina Lake.

Table 6.53 Evaluation Sheet by DHRAM Method for Control Point CP28

Control Point:	CP28		Period:		1939-1960							
IHA statistics group	USGS		DCFM		Absolute Chages		DCFD		Absolute Chages			
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)		
Parameter Group #1												
January	76.5	0.7758	126.8	1.103	65.8%	42.2%	125.9	1.115	64.6%	43.7%		
February	105.8	0.8571	183	1.217	73.0%	42.0%	182.1	1.227	72.1%	43.2%		
March	67.26	0.5605	150.2	1.06	123.3%	89.1%	149.4	1.071	122.1%	91.1%		
April	108.3	1.099	225.8	1.232	108.5%	12.1%	225	1.24	107.8%	12.8%		
May	110.2	1.051	246.7	1.071	123.9%	1.9%	245.9	1.078	123.1%	2.6%		
June	98.64	0.9636	203.4	1.078	106.2%	11.9%	202.6	1.086	105.4%	12.7%		
July	66.62	0.8886	93.78	0.8432	40.8%	5.1%	92.89	0.8587	39.4%	3.4%		
August	85.65	1.996	114.4	1.618	33.6%	18.9%	114.4	1.618	33.6%	18.9%		
September	171.7	1.539	217.5	1.394	26.7%	9.4%	217.5	1.394	26.7%	9.4%		
October	143.1	1.311	215.1	1.096	50.3%	16.4%	215.2	1.096	50.4%	16.4%		
November	98.37	1.567	137.2	1.236	39.5%	21.1%	137.2	1.236	39.5%	21.1%		
December	84.74	0.7497	127.7	0.8431	50.7%	12.5%	127.7	0.8434	50.7%	12.5%		
					Average Score	70.2%	23.6%			Average Score	69.6%	24.0%
						3	0				3	0
Parameter Group #2												
1-day minimum	28.55	0.8148	14	0.9682	51.0%	18.8%	0.2273	2.325	99.2%	185.3%		
3-day minimum	29.5	0.802	15.29	0.9309	48.2%	16.1%	0.303	1.793	99.0%	123.6%		
7-day minimum	30.97	0.7782	17.43	0.9218	43.7%	18.5%	0.6948	2.352	97.8%	202.2%		
30-day minimum	36.12	0.6979	33.46	0.7622	7.4%	9.2%	10.23	1.631	71.7%	133.7%		
90-day minimum	48.36	0.747	70.1	1.071	45.0%	43.4%	64.9	1.165	34.2%	56.0%		
1-day maximum	2904	1.29	3267	1.057	12.5%	18.1%	4032	1.009	38.8%	21.8%		
3-day maximum	1608	1.153	2156	0.9362	34.1%	18.8%	2653	0.8456	65.0%	26.7%		
7-day maximum	892.2	1.105	1329	0.8811	49.0%	20.3%	1621	0.8339	81.7%	24.5%		
30-day maximum	330.6	1.083	527.2	0.854	59.5%	21.1%	586.7	0.8379	77.5%	22.6%		
90-day maximum	201.1	1.024	324.1	0.8545	61.2%	16.6%	336.6	0.8515	67.4%	16.8%		
					Average Score	41.1%	20.1%			Average Score	73.2%	81.3%
						1	0				3	1
Parameter Group #3												
Date of minimum	160.2	0.2979	184	0.2522	14.9%	15.3%	82.09	0.2495	48.8%	16.2%		
Date of maximum	272.5	0.2651	172.8	0.2576	36.6%	2.8%	180.5	0.2549	33.8%	3.8%		
					Average Score	25.7%	9.1%			Average Score	41.3%	10.0%
						1	0				1	0
Parameter Group #4												
Low pulse count	5.182	1.382	4.318	0.8254	16.7%	40.3%	9.545	0.4943	84.2%	64.2%		
Low pulse duration	15.03	0.6441	19.46	0.8618	29.5%	33.8%	8.287	0.4431	44.9%	31.2%		
High pulse count	3.455	0.8223	4.955	0.8775	43.4%	6.7%	6.318	0.7987	82.9%	2.9%		
High pulse duration	2.208	0.7815	3.091	0.6221	40.0%	20.4%	2.564	0.4578	16.1%	41.4%		
					Average Score	32.4%	25.3%			Average Score	57.0%	34.9%
						1	0				2	1
Parameter Group #5												
Rise rate	83.77	1.061	184.2	1.066	119.9%	0.5%	245.2	0.8168	192.7%	23.0%		
Fall rate	-46.33	-0.9528	-40.32	-0.857	13.0%	10.1%	-74.57	-0.8258	61.0%	13.3%		
Number of reversals	107.6	0.1877	66.09	0.3783	38.6%	101.5%	76.27	0.3275	29.1%	74.5%		
					Average Score	57.1%	37.4%			Average Score	94.3%	36.9%
						2	1				3	1
					Total Point Classification	9 3				Total Point Classification	15 4	
					note:	Moderate risk of impact				note:	High risk of impact	

Both MDNF and DDNF have underestimated minimum flows, and have different flow frequency characteristics than the USGS flows at the control points CP32 and CP37 as tabulated in Table 6.54 and 6.55. These are also attributed to the influence of Median Lake operation even though the two control points are far away from the Lake.

Table 6.54 Evaluation Sheet by DHRAM Method for Control Point CP32

Control Point:	CP32		Period:	1934-1960						
IHA statistics group	USGS		DCFM		Absolute Chages		DCFD		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	274.2	0.6211	267.9	0.838	2.3%	34.9%	267.9	0.838	2.3%	34.9%
February	298.4	0.7345	312.4	0.9713	4.7%	32.2%	312.4	0.9713	4.7%	32.2%
March	244.1	0.4548	258.7	0.6947	6.0%	52.7%	258.7	0.6948	6.0%	52.8%
April	358.7	0.7867	389.5	0.9733	8.6%	23.7%	389.5	0.9733	8.6%	23.7%
May	440.4	0.9502	464	0.9322	5.4%	1.9%	464	0.9322	5.4%	1.9%
June	487.5	1.417	470.8	1.191	3.4%	15.9%	470.8	1.191	3.4%	15.9%
July	342	1.477	306.8	1.434	10.3%	2.9%	306.8	1.434	10.3%	2.9%
August	211.1	0.7619	188.5	0.94	10.7%	23.4%	188.5	0.94	10.7%	23.4%
September	519.9	1.6	581.4	1.678	11.8%	4.9%	581.4	1.678	11.8%	4.9%
October	402.6	1.022	403.3	1.081	0.2%	5.8%	403.3	1.081	0.2%	5.8%
November	291.3	0.9003	275.1	1.014	5.6%	12.6%	275.1	1.014	5.6%	12.6%
December	275.1	0.5618	252.9	0.7089	8.1%	26.2%	252.9	0.7087	8.1%	26.1%
				Average Score	6.4%	19.8%		Average Score	6.4%	19.8%
					0	0			0	0
Parameter Group #2										
1-day minimum	95.78	0.5125	31.94	1.086	66.7%	111.9%	1.148	2.886	98.8%	463.1%
3-day minimum	99.21	0.507	33.78	1.054	66.0%	107.9%	1.864	2.479	98.1%	389.0%
7-day minimum	103.6	0.4914	38.46	0.9652	62.9%	96.4%	3.376	2.274	96.7%	362.8%
30-day minimum	119.8	0.4467	73.67	0.7839	38.5%	75.5%	36.96	1.076	69.1%	140.9%
90-day minimum	160.6	0.4566	137.6	0.7121	14.3%	56.0%	127.7	0.7427	20.5%	62.7%
1-day maximum	6512	1.307	3233	1.021	50.4%	21.9%	4715	0.9742	27.6%	25.5%
3-day maximum	4873	1.225	2957	1.018	39.3%	16.9%	4090	0.9675	16.1%	21.0%
7-day maximum	2932	1.083	2372	0.964	19.1%	11.0%	2941	0.9363	0.3%	13.5%
30-day maximum	1077	0.9371	1096	0.9192	1.8%	1.9%	1172	0.8798	8.8%	6.1%
90-day maximum	652.1	0.8277	670.3	0.8346	2.8%	0.8%	692.7	0.8327	6.2%	0.6%
				Average Score	36.2%	50.0%		Average Score	44.2%	148.5%
					1	1			2	2
Parameter Group #3										
Date of minimum	205.1	0.2227	211.8	0.2752	3.3%	23.6%	79.37	0.2453	61.3%	10.1%
Date of maximum	184	0.2285	177.9	0.2683	3.3%	17.4%	180.6	0.2851	1.8%	24.8%
				Average Score	3.3%	20.5%		Average Score	31.6%	17.5%
					0	0			1	0
Parameter Group #4										
Low pulse count	7.407	1.062	4.963	0.6764	33.0%	36.3%	9.407	0.4224	27.0%	60.2%
Low pulse duration	10.26	0.7058	15.91	0.8606	55.1%	21.9%	8.534	0.5786	16.8%	18.0%
High pulse count	4	0.7783	2.815	0.9453	29.6%	21.5%	4.296	0.8705	7.4%	11.8%
High pulse duration	2.95	0.7193	6.16	0.4557	108.8%	36.6%	4.379	0.4425	48.4%	38.5%
				Average Score	56.6%	29.1%		Average Score	24.9%	32.1%
					2	0			1	1
Parameter Group #5										
Rise rate	184.9	0.8854	113	0.8866	38.9%	0.1%	188.3	0.7739	1.8%	12.6%
Fall rate	-96.28	-0.8028	-39.12	-0.8458	59.4%	5.4%	-83.34	-0.7149	13.4%	10.9%
Number of reversals	106.5	0.1648	56.78	0.2333	46.7%	41.6%	64.26	0.2019	39.7%	22.5%
				Average Score	48.3%	15.7%		Average Score	18.3%	15.4%
					2	0			0	0
Total Point					6	Total Point				
Classification					3	Classification				
note:					Moderate risk of impact	note:				

Table 6.55 Evaluation Sheet by DHRAM Method for Control Point CP35

Control Point:	CP35	Period:	1934-1960							
IHA statistics group	USGS		DCFM		Absolute Chages		DCFD		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1										
January	67.37	1.295	67.42	1.293	0.1%	0.2%	67.42	1.293	0.1%	0.2%
February	82.28	2.1	82.35	2.098	0.1%	0.1%	82.35	2.098	0.1%	0.1%
March	53.36	0.9074	53.47	0.9045	0.2%	0.3%	53.48	0.9045	0.2%	0.3%
April	189.7	1.538	189.8	1.536	0.1%	0.1%	189.8	1.536	0.1%	0.1%
May	215.8	1.377	216	1.375	0.1%	0.1%	216	1.375	0.1%	0.1%
June	186.3	1.671	186.6	1.668	0.2%	0.2%	186.6	1.668	0.2%	0.2%
July	120.7	2.498	120.8	2.493	0.1%	0.2%	120.8	2.493	0.1%	0.2%
August	63.4	2.716	63.63	2.705	0.4%	0.4%	63.64	2.705	0.4%	0.4%
September	184.5	1.889	184.6	1.887	0.1%	0.1%	184.6	1.887	0.1%	0.1%
October	124.4	1.566	124.5	1.565	0.1%	0.1%	124.5	1.565	0.1%	0.1%
November	62.61	1.748	62.63	1.747	0.0%	0.1%	62.63	1.747	0.0%	0.1%
December	76.22	1.488	76.24	1.487	0.0%	0.1%	76.25	1.487	0.0%	0.1%
					Average Score	0.1% 0.2%			Average Score	0.1% 0.2%
						00				00
Parameter Group #2										
1-day minimum	9.556	0.5235	1.491	1.043	84.4%	99.2%	0	0	100.0%	100.0%
3-day minimum	9.84	0.5131	1.644	1.017	83.3%	98.2%	0	0	100.0%	100.0%
7-day minimum	10.31	0.4764	2.207	0.8931	78.6%	87.5%	0.005291	5.196	99.9%	990.7%
30-day minimum	12.01	0.4835	7.248	0.5655	39.7%	17.0%	3.59	1.198	70.1%	147.8%
90-day minimum	19.92	0.5905	21.49	0.6273	7.9%	6.2%	19.77	0.724	0.8%	22.6%
1-day maximum	6658	0.8417	2596	0.9676	61.0%	15.0%	3549	0.9052	46.7%	7.5%
3-day maximum	3868	0.8548	2211	0.9827	42.8%	15.0%	2873	0.9148	25.7%	7.0%
7-day maximum	2099	0.7933	1527	0.864	27.3%	8.9%	1844	0.8061	12.1%	1.6%
30-day maximum	631.5	0.7991	607.6	0.7907	3.8%	1.1%	631.7	0.7728	0.0%	3.3%
90-day maximum	315.1	0.8857	310.1	0.8842	1.6%	0.2%	316.9	0.8782	0.6%	0.8%
					Average Score	43.0% 34.8%			Average Score	45.6% 138.1%
						11				22
Parameter Group #3										
Date of minimum	212.8	0.1583	192.4	0.2563	9.6%	61.9%	43.96	0.134	79.3%	15.4%
Date of maximum	181.3	0.1916	175.4	0.2433	3.3%	27.0%	176.6	0.2369	2.6%	23.6%
					Average Score	6.4% 44.4%			Average Score	41.0% 19.5%
						01				10
Parameter Group #4										
Low pulse count	6.333	0.9792	9.407	0.4366	48.5%	55.4%	11.37	0.3228	79.5%	67.0%
Low pulse duration	12.16	0.5765	9.611	0.3454	21.0%	40.1%	7.434	0.2257	38.9%	60.8%
High pulse count	4.296	0.7298	2.741	0.8681	36.2%	19.0%	2.963	0.8293	31.0%	13.6%
High pulse duration	1.976	0.4097	6.047	0.6312	206.0%	54.1%	4.604	0.4146	133.0%	1.2%
					Average Score	77.9% 42.1%			Average Score	70.6% 35.7%
						31				31
Parameter Group #5										
Rise rate	266	0.8631	79.62	0.8872	70.1%	2.8%	121.4	0.7779	54.4%	9.9%
Fall rate	-109.1	-0.7646	-23.5	-0.8918	78.5%	16.6%	-43.93	-0.7715	59.7%	0.9%
Number of reversals	80.96	0.1669	70.07	0.1513	13.5%	9.3%	71.52	0.1203	11.7%	27.9%
					Average Score	54.0% 9.6%			Average Score	41.9% 12.9%
						20				10
					Total Point	9			Total Point	10
					Classification	3			Classification	3
					note:	Moderate risk of impact			note:	Moderate risk of impact

Both MDNF and DDNF have underestimated minimum flows, and have different durations of low and high pulses than the USGS flows at the control points CP35 as tabulated in Table 6.56. These are also attributed to the influence of the return flows from ground water uses within the basin.

Table 6.56 Evaluation Sheet by DHRAM Method for Control Point CP37

Control Point:	CP37	Period:	1939-1960									
IHA statistics group	USGS		DCFM		Absolute Chages		DCFD		Absolute Chages			
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)	Mean(cfs)	CV	Mean(%)	CV(%)		
Parameter Group #1												
January	374.9	0.9253	359.9	1.102	4.0%	19.1%	372.9	1.072	0.5%	15.9%		
February	469	1.253	470.4	1.404	0.3%	12.1%	483.3	1.366	3.0%	9.0%		
March	332.8	0.5539	338.1	0.757	1.6%	36.7%	338.1	0.757	1.6%	36.7%		
April	601.1	1.115	635.2	1.198	5.7%	7.4%	635.2	1.198	5.7%	7.4%		
May	885.4	1.065	933.3	1.126	5.4%	5.7%	933.3	1.126	5.4%	5.7%		
June	598.2	0.9392	626.4	0.9546	4.7%	1.6%	626.4	0.9545	4.7%	1.6%		
July	535.5	1.637	511.9	1.721	4.4%	5.1%	511.9	1.721	4.4%	5.1%		
August	300	0.6999	271.8	0.8438	9.4%	20.6%	271.8	0.8437	9.4%	20.5%		
September	992.1	1.413	969.8	1.469	2.2%	4.0%	969.8	1.469	2.2%	4.0%		
October	815.5	1.523	811.1	1.557	0.5%	2.2%	811.1	1.557	0.5%	2.2%		
November	547.1	1.187	517.4	1.284	5.4%	8.2%	517.4	1.284	5.4%	8.2%		
December	388	0.8919	362.3	1.049	6.6%	17.6%	362.3	1.049	6.6%	17.6%		
Average Score					4.2%	11.7%	Average Score					
					0	0						
Parameter Group #2												
1-day minimum	120.2	0.5493	49	0.7918	59.2%	44.1%	3.182	3.51	97.4%	539.0%		
3-day minimum	122.8	0.5434	51.64	0.7752	57.9%	42.7%	3.621	3.527	97.1%	549.1%		
7-day minimum	127.3	0.536	57.8	0.7425	54.6%	38.5%	5.981	2.832	95.3%	428.4%		
30-day minimum	147.1	0.4955	104.2	0.665	29.2%	34.2%	56.22	1.256	61.8%	153.5%		
90-day minimum	210.6	0.5333	185.3	0.651	12.0%	22.1%	173.2	0.7436	17.8%	39.4%		
1-day maximum	10140	0.8336	6040	0.908	40.4%	8.9%	8116	0.9082	20.0%	8.9%		
3-day maximum	8537	0.857	5296	0.8863	38.0%	3.4%	7006	0.8818	17.9%	2.9%		
7-day maximum	5875	0.8394	4334	0.8795	26.2%	4.8%	5551	0.8337	5.5%	0.7%		
30-day maximum	2137	0.8196	2166	0.8381	1.4%	2.3%	2327	0.8371	8.9%	2.1%		
90-day maximum	1218	0.8486	1236	0.853	1.5%	0.5%	1280	0.8347	5.1%	1.6%		
Average Score					32.0%	20.2%	Average Score					
					1	0						
Parameter Group #3												
Date of minimum	218.3	0.1787	209.6	0.2618	4.0%	46.5%	81.18	0.2653	62.8%	48.5%		
Date of maximum	183.3	0.1899	197.7	0.2202	7.9%	16.0%	208.6	0.2679	13.8%	41.1%		
Average Score					5.9%	31.2%	Average Score					
					0	1						
Parameter Group #4												
Low pulse count	6.318	1.057	4.409	0.659	30.2%	37.7%	7.364	0.5056	16.6%	52.2%		
Low pulse duration	13.94	0.4568	17.36	0.6526	24.5%	42.9%	11.51	0.4545	17.4%	0.5%		
High pulse count	4.091	0.948	2.227	1.082	45.6%	14.1%	3.227	0.8496	21.1%	10.4%		
High pulse duration	3.37	0.3921	9.942	0.7695	195.0%	96.3%	5.81	0.4433	72.4%	13.1%		
Average Score					73.8%	47.7%	Average Score					
					3	1						
Parameter Group #5												
Rise rate	318.7	0.8789	130.2	0.929	59.1%	5.7%	210.4	0.7697	34.0%	12.4%		
Fall rate	-166.4	-0.8241	-59.52	-0.8701	64.2%	5.6%	-116.9	-0.7226	29.7%	12.3%		
Number of reversals	105.2	0.1729	42.41	0.1562	59.7%	9.7%	48.45	0.1577	53.9%	8.8%		
Average Score					61.0%	7.0%	Average Score					
					2	0						
Total Point Classification					8	3	Total Point Classification					
note:					Moderate risk of impact			note:				

Table 6.57 summarizes the comparative scores, made through the qualitative comparison of both annual median flow duration curves at 12 control points. Annual flow duration curves of each flow at all the 12 control points as shown in Figures 6.17 to 6.28, respectively.

Table 6.57 Comparative Evaluation of Annual Median Flow Duration Curve
for the GSA WAM

CP	Data Period	Comparative Score	
		USGS vs. MDNF	USGS vs. DDNF
<u>Total</u>		<u>19</u>	<u>5</u>
CP01	06/01/1939-12/31/1960	2	0
CP02	01/01/1934-12/31/1960	2	0
CP04	01/01/1934-12/31/1960	2	0
CP08	01/01/1934-12/31/1960	1	1
CP10	05/01/1939-12/31/1960	2	0
CP11	01/01/1934-12/31/1960	2	0
CP15	01/01/1934-12/31/1960	1	1
CP18	03/01/1939-12/31/1960	1	1
CP28	08/01/1939-12/31/1960	0	2
CP32	01/01/1934-12/31/1960	2	0
CP35	01/01/1934-12/31/1960	2	0
CP37	03/01/1939-12/31/1960	2	0

The annual median curves of MDNF are mostly more similar to the USGS flows than the DDNF except for the control points CP08, 15, 18, and 28. Both annual median curves are closely similar to the USGS flows at the control points CP08 and CP15. The DDNF has more similar annual median curve to the USGS flows than the MDNF at only the control point CP28. At the control point CP18, both annual median curves of MDNF and DDNF are totally different from the USGS flows. Therefore, comparative evaluation is impossible at this control point. 1 point is given to both the disaggregated daily flows, respectively.

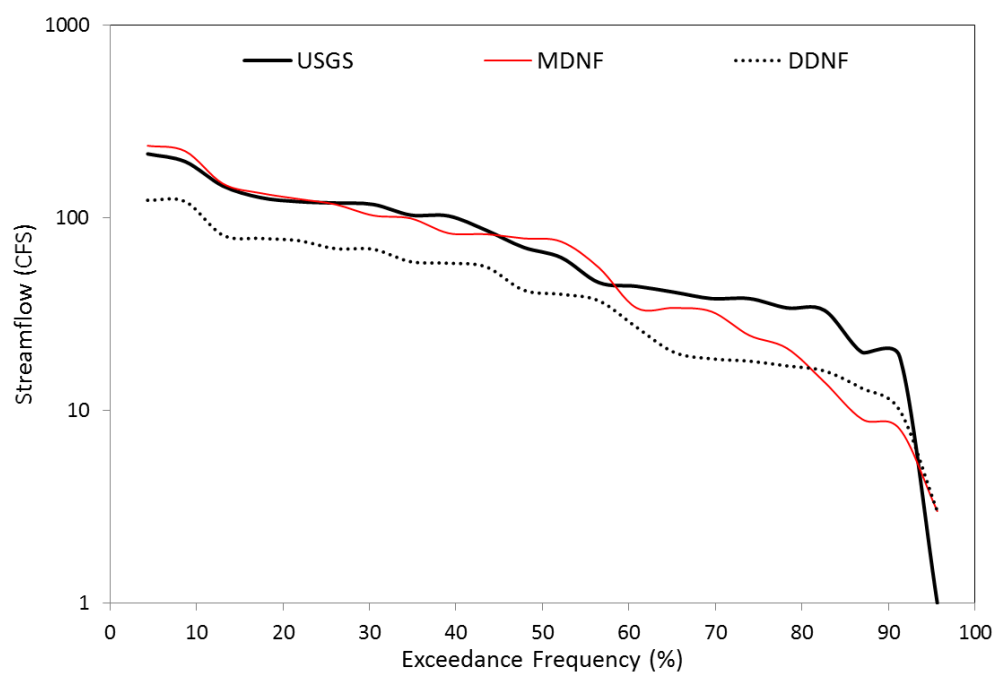


Figure 6.17 Annual Median Flow Duration Curves at CP01

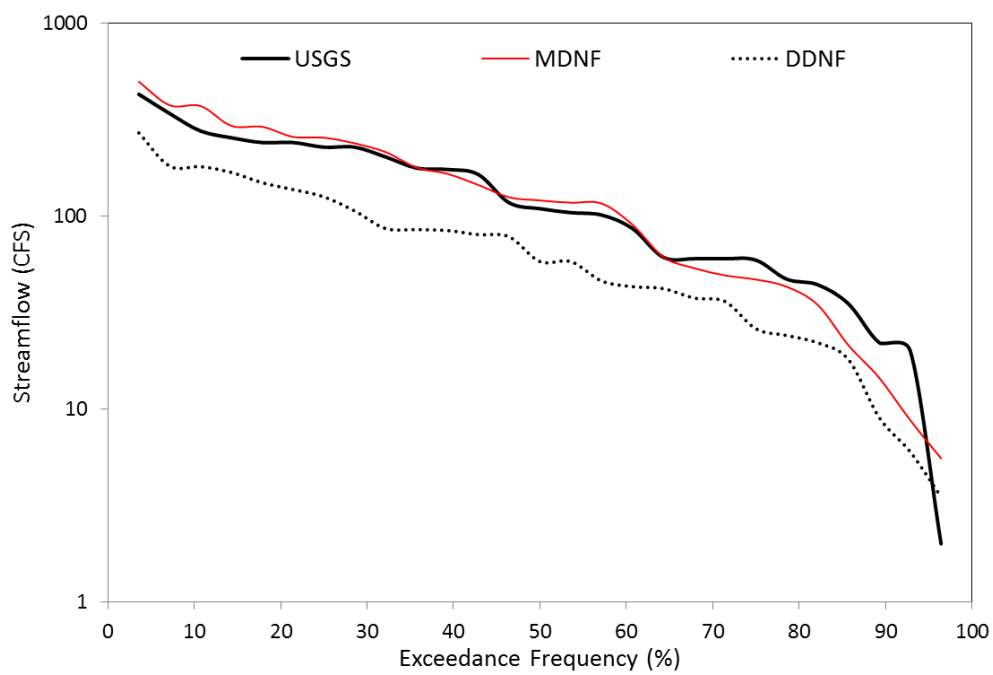


Figure 6.18 Annual Median Flow Duration Curves at CP02

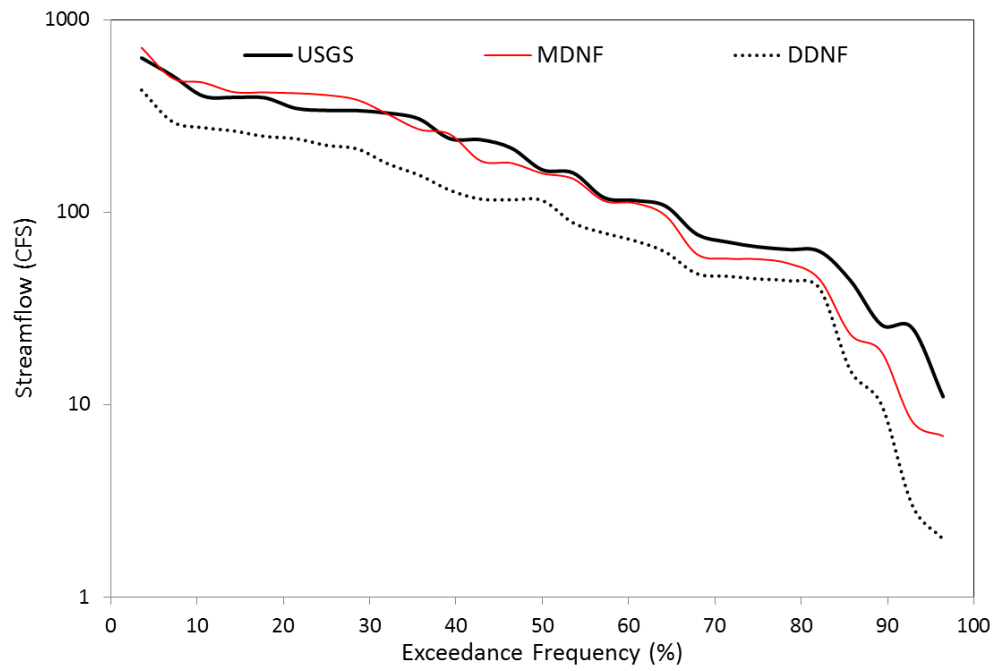


Figure 6.19 Annual Median Flow Duration Curves at CP04

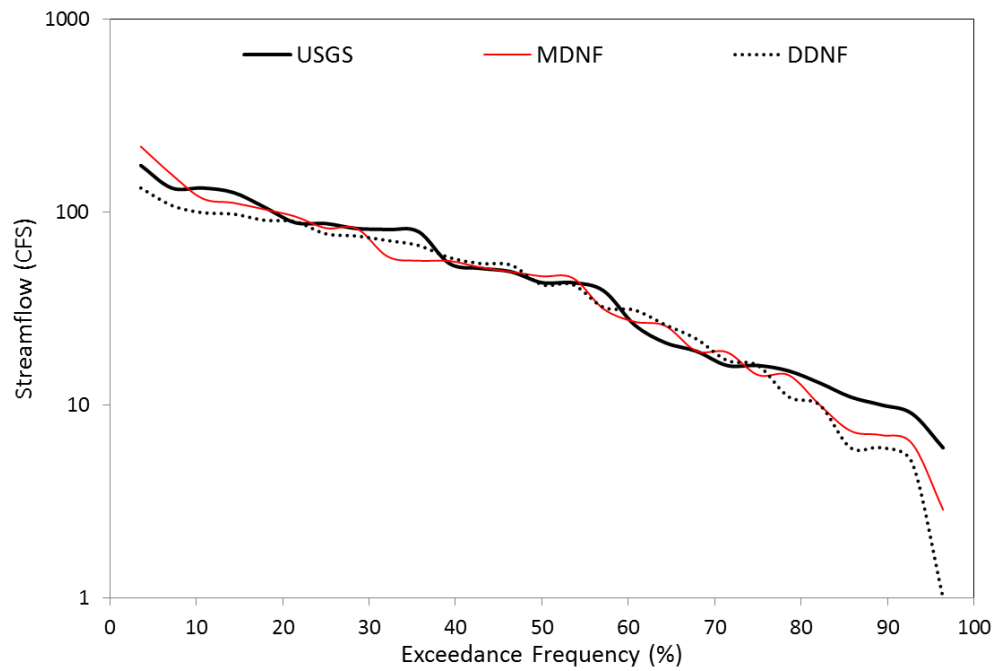


Figure 6.20 Annual Median Flow Duration Curves at CP08

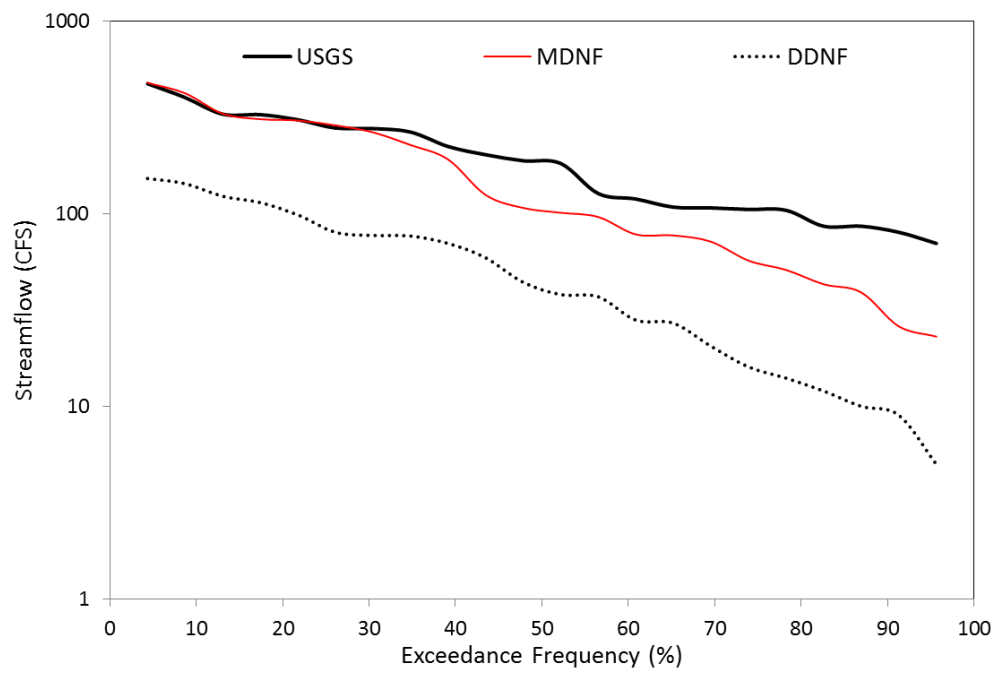


Figure 6.21 Annual Median Flow Duration Curves at CP10

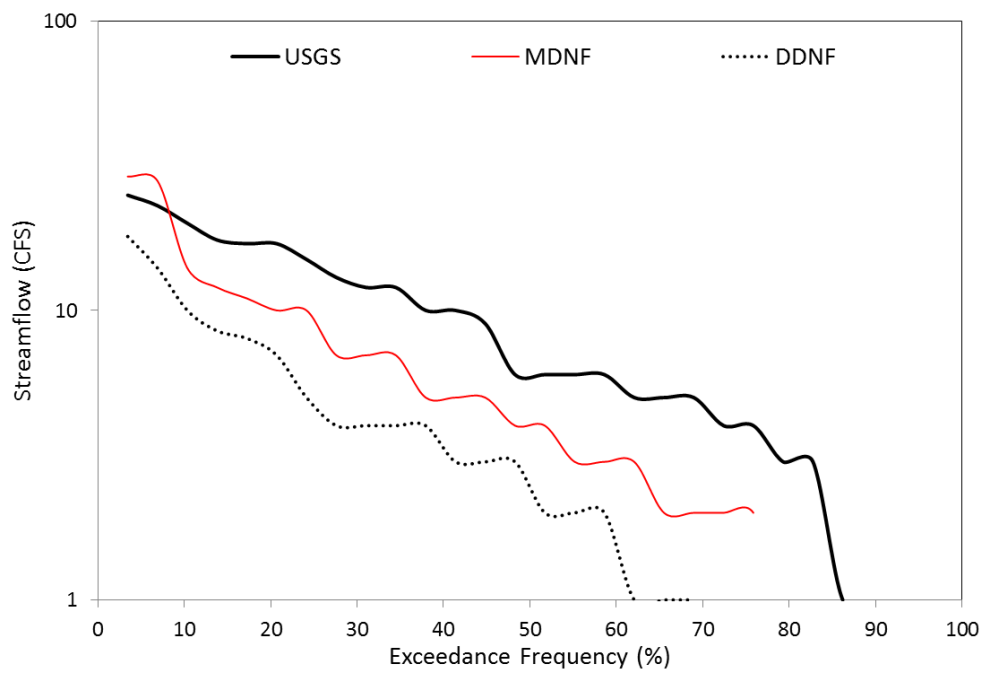


Figure 6.22 Annual Median Flow Duration Curves at CP11

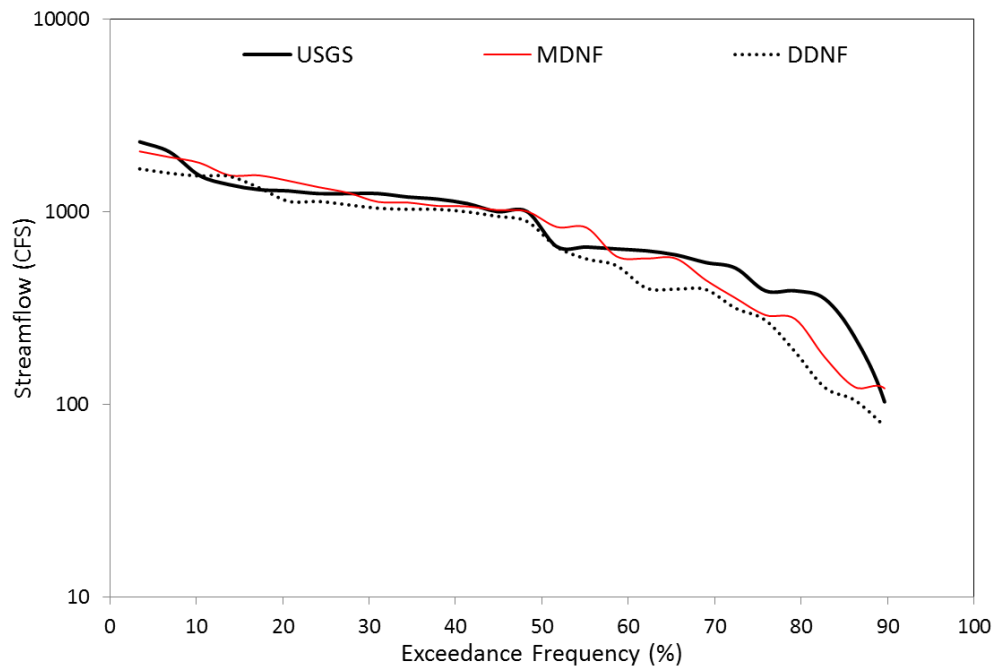


Figure 6.23 Annual Median Flow Duration Curves at CP15

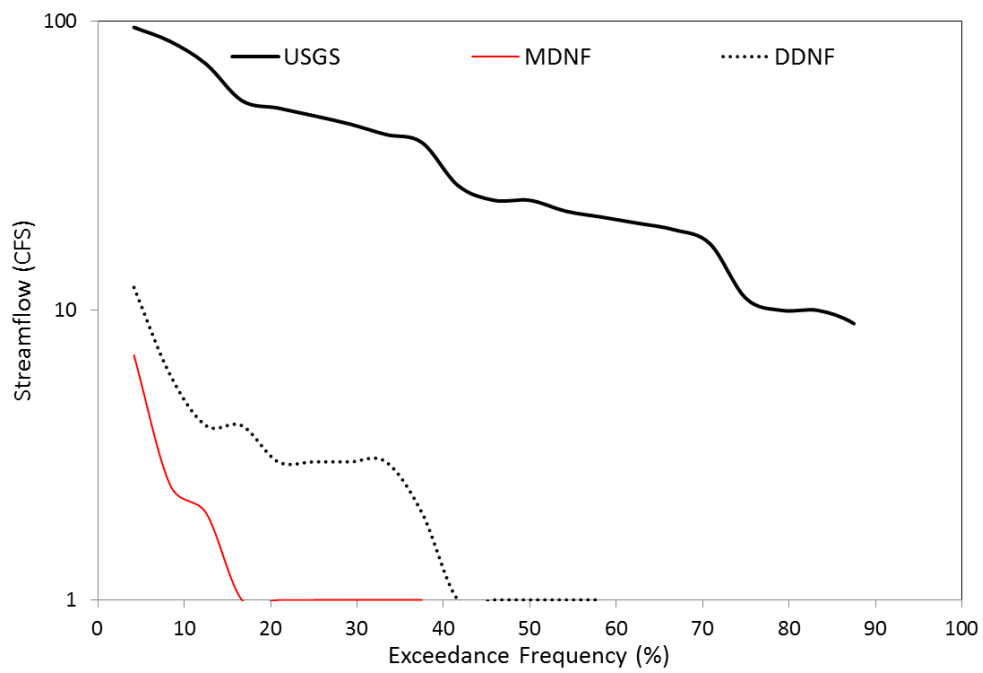


Figure 6.24 Annual Median Flow Duration Curves at CP18

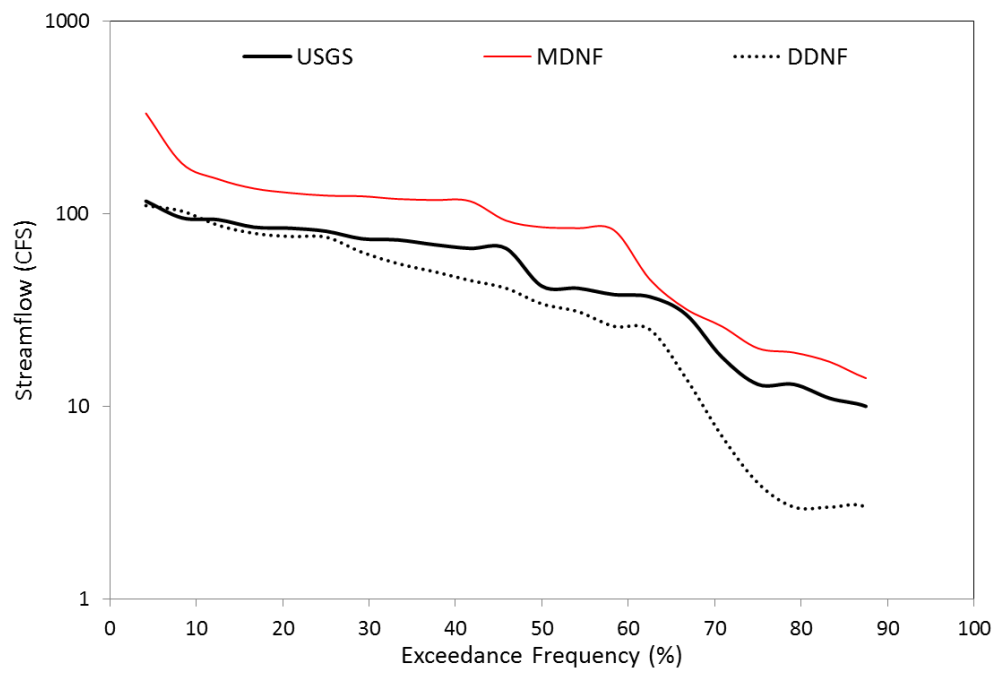


Figure 6.25 Annual Median Flow Duration Curves at CP28

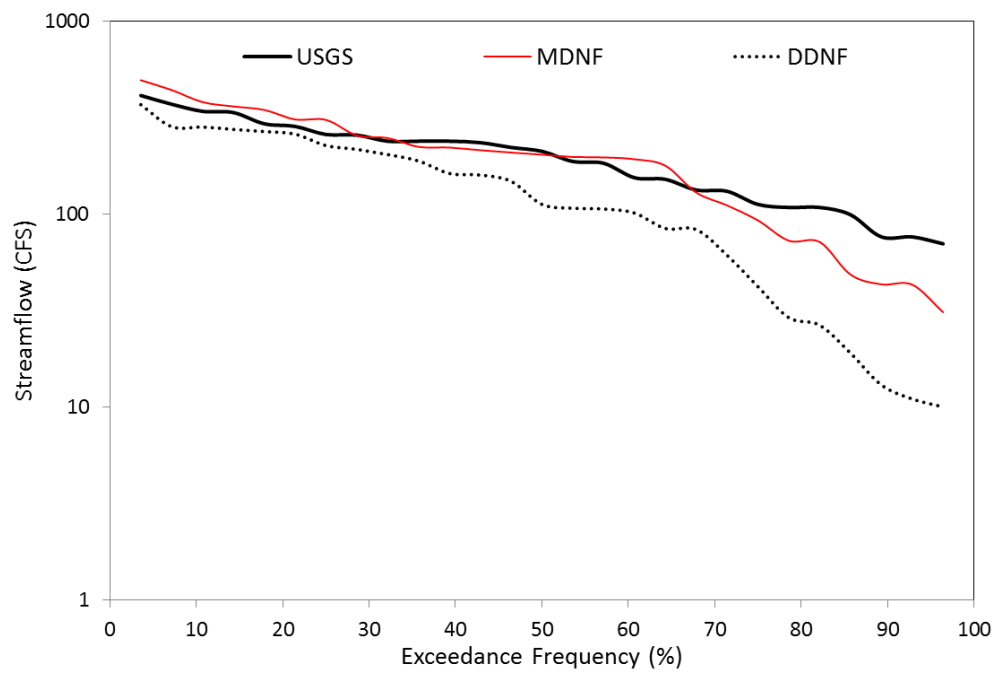


Figure 6.26 Annual Median Flow Duration Curves at CP32

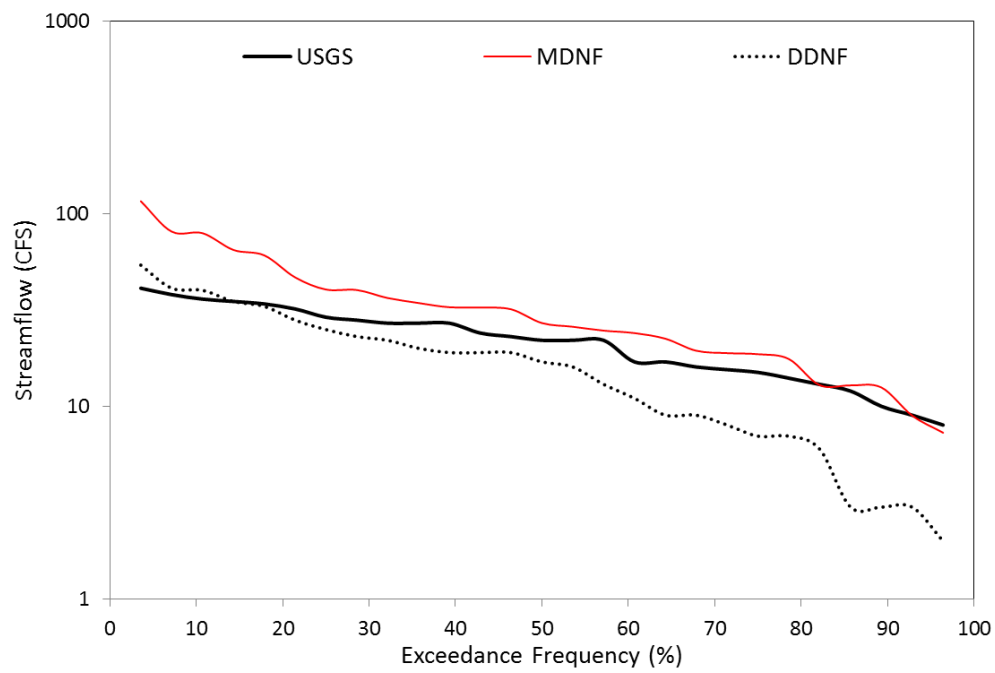


Figure 6.27 Annual Median Flow Duration Curves at CP35

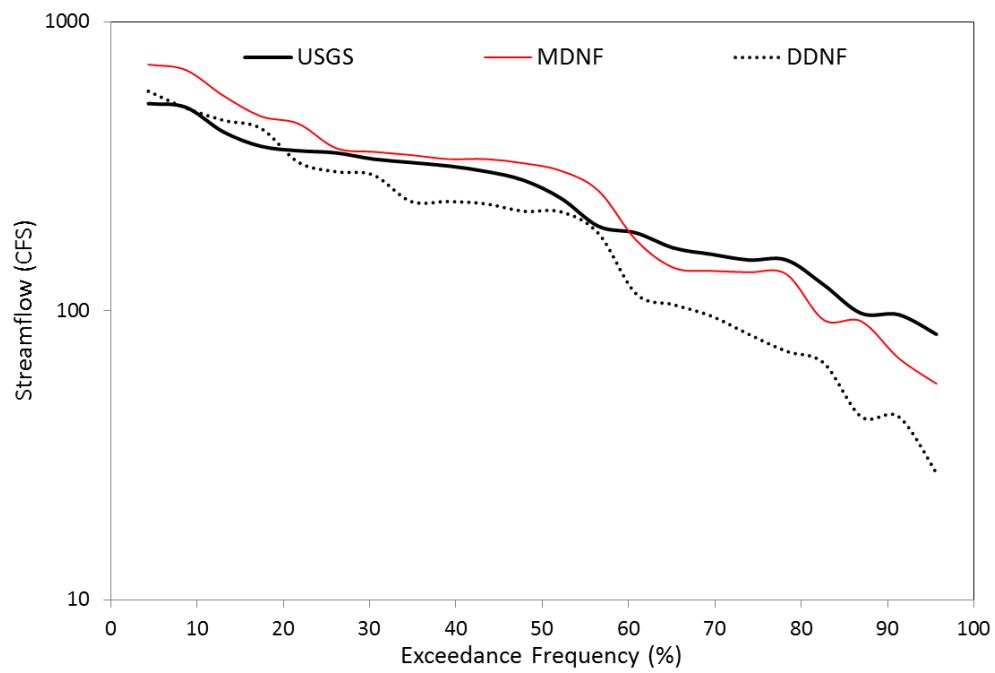


Figure 6.28 Annual Median Flow Duration Curves at CP37

The total scores for both the disaggregated flows are tabulated in Table 6.58. MDNF has better performances than DDNF on all evaluation methods. Both the disaggregated flows would have slightly different flow regimes from the USGS daily recorded flows at some control points. These are attributed to the reasons that various human impacts, such as dam operations and effluent from spring water sources influenced on USGS daily recorded flows for periods-of-comparison. However, the total score of the comparative evaluation indicates that MDNF has relatively more similar naturalized flow regimes than DDNF.

Accordingly, the calibration strategy with monthly naturalized flows from the WAM datasets for the daily SWAT is selected for the daily GSA WAM as tabulated in Table 6.58.

Table 6.58 Selection of Calibration Strategy for the GSA WAM

Methods	Evaluation purposes	Calibration Strategy	
		Monthly WAM datasets	Daily USGS recorded data
Total Score		8	0
Statistic Evaluation (NSE)	Streamflow timing	2	0
Flow Frequency Metric	Streamflow regime	2	0
DHRAM (IHA)	Hydrologic characteristics alteration	2	0
Median Annual Flow Duration Curve	Overall hydrologic state of a river	2	0

CHAPTER VII

MODELING SB3 ENVIRONMENTAL FLOW STANDARDS

The process created by the 1997 Senate Bill 3 (SB3) for establishing environmental flow requirements is explained in detail by Wurbs and Hoffpauir (2013b) and briefly described in Chapters 1 and 2 of this dissertation. SB3 standards have been established for the case study river basins at the USGS gauging stations noted in this chapter. These standards have been incorporated in the Sabine, Neches, and GSA WAMs as described in this chapter. Simulation results from the daily WAMs are summarized in the present Chapter VII from the perspective of analyzing the SB3 environmental flow requirements. Further statistical analyses of flows are presented in Chapter VIII.

7.1 Senate Bill 3 Environmental Flow Standards for the Basins

7.1.1 Sabine River Basin

The Recommendation Report for the Sabine and Neches Rivers was submitted by the Bay and Basin Expert Science Team (BBEST) to the Basin and Bay Area Stakeholder Committee (BBASC) and TCEQ in November 2009. The TCEQ received the Recommendation Report from the Bay and Basin Area Stakeholder Committee (BBASC) in May 2010. The TCEQ finalized the standards for the Sabine and Neches Rivers effective May 15, 2011.

The environmental flow standards for surface water for the Sabine and Neches Rivers are published in Texas Administrative Code Title 30, Part 1, Chapter 298, Subchapter C. The Code includes environmental flow standards at ten USGS gaging stations, five sites in the Sabine River Basin and five sites in the Neches River Basin. The Sabine WAM four primary and one secondary control points corresponding to the five USGS gage sites are listed with descriptive information in Table 6.2. New adjacent control points BSBSE, SRGWE, SRBEE, 29500E, and SRRLE for the environmental flow standards were added to the daily WAM (Wurbs *et al.*, 2014a).

The environmental flow standards include seasonal subsistence flows, base flows, and high flow pulses depending on four seasons, listed in Table 7.1.

Table 7.1 Months Included in Each Season for the Sabine River Basin

Season	Months
Winter	January, February, March
Spring	April, May, June
Summer	July, August, September
Fall	October, November, December

Subsistence and base flow standards

Table 7.2 tabulates the subsistence flow standards for the four control points in the Sabine River Basin. If the flow at a control point is less than the applicable subsistence flow standard, then water right holders may not make diversions from the river (Wurbs *et al.*, 2014a). If the flow is greater than the subsistence flow standard and less than the applicable base flow standard, water right holders may make diversions as long as the flow does not drop below the subsistence flow (Wurbs *et al.*, 2014a). Table 7.3 tabulates base flow standards. If the flow at a control point is greater than the applicable base flow standard and less than the applicable pulse flow trigger level, then water right holders may make diversions as long as the flow does not drop below the base flow standard (Wurbs *et al.*, 2014a).

Table 7.2 Subsistence Flow Standards (cfs)
for the Sabine River Basin

WAM CP ID	Winter	Spring	Summer	Fall
BSBSE	20	9	8	8
SRGWE	45	22	14	17
SRBEE	66	28	22	22
29500E	28	20	20	20
SRRLE	949	436	396	396

Table 7.3 Base Flow Standards (cfs) for the Sabine River Basin

WAM CP ID	Winter	Spring	Summer	Fall
BSBSE	73	33	15	22
SRGWE	305	131	37	54
SRBEE	482	255	56	83
29500E	62	42	31	40
SRRLE	1672	1329	737	809

High flow pulse standards

The high flow pulse standards are activated when flow at a control point goes beyond the applicable high flow pulse trigger level. Water right holders may not be allowed to divert water until either the applicable volume or duration time has passed since engagement of the trigger flow level. However, diversions can be allowed before the volume or duration criteria are satisfied if the flow at the control point goes beyond the high flow pulse trigger level, as long as diversions do not lead to the flow to go below the high flow pulse trigger level. Winter and Summer seasons have one pulse per season, and Spring and Fall seasons have two pulses per season for all five control points according to the criteria specified in Table 7.4. The tracking of high flow pulse events each season is performed independently of preceding and subsequent seasons (Wurbs *et al.*, 2014a).

Water right permit conditions

The environmental flow standards will be differently applied for some water right permits issued after the effective date of the environmental flow standards depending on the conditions of the new permit. Specifically, water right permits with an authorization to divert 10,000 acre-feet or less per year are not required to protect the high flow pulse requirements (Wurbs *et al.*, 2014a). However, as discussed later in this chapter, the input records corresponding to the SB-3 requirements in the Sabine WAM DAT file were not configured to allow junior water right exemptions from honoring downstream senior instream flow requirements (Wurbs *et al.*, 2014a).

Table 7.4 High Flow Pulse Standards for the Sabine River Basin

WAM CP	Criteria	Winter	Spring	Summer	Fall
BSBSE	Trigger (cfs)	358	313	50	130
	Volume (ac-ft)	5,932	5,062	671	2,189
	Duration (days)	10	13	6	9
SRGWE	Trigger (cfs)	1,880	1,580	168	380
	Volume (ac-ft)	48,599	51,150	2,752	1098
	Duration (days)	15	16	7	11
SRBEE	Trigger (cfs):	2,900	2,160	285	628
	Volume (ac-ft)	84,998	72,092	5,436	7,245
	Duration (days)	15	15	6	9
29500E	Trigger (cfs)	693	350	109	322
	Volume (ac-ft)	4,911	2,545	873	2,232
	Duration (days)	8	7	5	7
SRRLE	Trigger (cfs)	1,600	3,250	3,380	2020
	Volume (ac-ft)	10,202	42,883	54,321	17,662
	Duration (days)	3	8	11	5

7.1.2 Neches River Basin

The environmental flow standards for surface water for the Sabine and Neches Rivers are provided in Texas Administrative Code Title 30, Part 1, Chapter 298, Subchapter C. Environmental flow standards are established at five USGS gaging stations in the Neches River Basin. Instream flow standards at the five Neches River Basin locations were incorporated into the daily Neches WAM using the modeling techniques (Wurbs *et al.*, 2014b). The Neches WAM primary control points corresponding to the five USGS gage sites are listed with descriptive information in Table 6.3.

Although not necessary, five new control points were added immediately downstream of the primary control points dedicated solely to the SB3 standards (Wurbs *et al.*, 2014b). The identifiers of the new control points are the same as original control points, with the letter E added to the end (Wurbs *et al.*, 2014b). The instream flow standards are established dividing into seasonal subsistence flows, base flows, and high flow pulses. Table 7.5 defines four seasons according to the months.

Table 7.5 Months Included in Each Season for the Neches River Basin

Season	Months
Winter	January, February, March
Spring	April, May, June
Summer	July, August, September
Fall	October, November, December

Table 7.6 tabulates the subsistence flow standards for the four control points in the Neches River Basin.

Table 7.6 Subsistence Flow Standards (cfs) for the Neches River Basin

WAM CP ID	Winter	Spring	Summer	Fall
NENEE	51	21	12	13
NEROE	67	29	21	21
ANALE	55	18	11	16
NEEVE	228	266	228	228
VIKOE	83	49	41	41

Table 7.7 also defines the base flow standards, and Table 7.8 specifies high flow pulse standards depending on four seasons for the Neches River Basins.

Table 7.7 Base Flow Standards (cfs) for the Neches River Basin

WAM CP ID	Winter	Spring	Summer	Fall
NENEE	196	96	46	80
NEROE	603	420	67	90
ANALE	277	90	40	52
NEEVE	1,925	1,804	580	512
VIKOE	264	117	77	98

Table 7.8 High Flow Pulse Standards for the Neches River Basin

WAM CP ID	Criteria	Winter	Spring	Summer	Fall
NENEE	Trigger (cfs):	833	820	113	345
	Volume (ac-ft):	19,104	20,405	1,339	5,391
	Duration (days):	10	12	4	8
NEROE	Trigger (cfs):	3,080	1,720	195	515
	Volume (ac-ft):	82,195	39,935	1,548	8,172
	Duration (days):	14	12	5	8
ANALE	Trigger (cfs):	1,620	1,100	146	588
	Volume (ac-ft):	37,114	24,117	2,632	12,038
	Duration (days):	13	14	8	12
NEEVE	Trigger (cfs):	2,020	3,830	1,540	1,570
	Volume (ac-ft):	20,920	68,784	21,605	17,815
	Duration (days):	6	12	9	7
VIKOE	Trigger (cfs):	2,010	1,380	341	712
	Volume (ac-ft):	36,927	23,093	6,159	11,426
	Duration (days):	13	13	8	9

7.1.3 GSA River Basins

The Recommendation Report for the Guadalupe, San Antonio, Mission, and Aransas Rivers and Mission, Copano, Aransas, and San Antonio Bays was submitted by The Bay and Basin Expert Science Team (BBEST) to the BBASC and TCEQ in March 2011. TCEQ received the Recommendation Report of The Bay and Basin Area Stakeholder Committee (BBASC) in September 2011 and a Work Plan in May 2012, respectively. TCEQ adopted environmental flow standards for the Guadalupe, San Antonio, Mission, and Aransas Rivers and Mission, Copano, Aransas, and San Antonio Bays on effective August 30, 2012.

The environmental flow standards for surface water for the Guadalupe, San Antonio, Mission, and Aransas Rivers and Mission, Copano, Aransas, and San Antonio Bays are documented in Texas Administrative Code Title 30, Part 1, Chapter 298, Subchapter E. GSA River Basins has flow standards at 16 control point locations, including 9 sites in the Guadalupe River Basin, 6 sites in the San Antonio River Basin, and 1 site in the Mission River Basin. Thus, the GSA WAM incorporate the flow standards for the 15 control points located in the Guadalupe and San Antonio River Basins. Table

6.5 lists the 15 control points with descriptive information. The priority date used for water availability modeling is March 1, 2011, the date the BBEST report was submitted (Wurbs *et al.*, 2014c).

The environmental flow standards have three different recommendations for subsistence flows, base flows, and high flow pulses. These are seasonally various and, in the San Antonio River Basin, according to hydrologic conditions. Each season includes three with the winter season beginning in January, as listed in Table 7.9.

Table 7.9 Months Included in Each Season for the GSA River Basins

Season	Months
Winter	January, February, March
Spring	April, May, June
Summer	July, August, September
Fall	October, November, December

In the San Antonio River Basin, three hydrologic conditions (dry, average, and wet) are decided depending on the amount of 12-month cumulative stream flows. The hydrologic conditions are evaluated once at the beginning of each season based on 12-month cumulative flows on the last day of the preceding season and applied for the entire season. Table 7.10 lists the cumulative stream flow limits for each hydrologic condition. These were determined by assessing the exceedance frequency curves for 12-month cumulative monthly naturalized flows from the GSA WAM, such that dry conditions occurred 25% of the time, average conditions occurred 50% of the time, and wet conditions occurred 25% of the time (Wurbs *et al.*, 2014c).

The cumulative stream flow limits were determined based on the existing naturalized flow values for standard period-of-analysis from 1934-1989. A separate set of cumulative stream flow limits was developed using the original naturalized flows for the period from 1934-1989 and *WRAP-HYD* extended naturalized flows for the period from 1990-2013 (Wurbs *et al.*, 2014c).

Table 7.10 12-Month Cumulative Naturalized Stream Flow Limits for Evaluating Hydrologic Conditions at Control Points in the San Antonio River Basin

Control Point	<u>Hydrologic Condition</u>		
	Dry	Average	Wet
<u>Original 1934-1989 Dataset</u>			
P382411	26,591	26,591 - 103,345	103,345
CP28	71,879	71,879 - 245,191	245,191
CP29	111,543	111,543 - 379,920	379,920
CP32	136,710	136,710 - 436,835	436,835
CP35	30,622	30,622 - 119,904	119,904
CP37	220,177	220,177 - 713,915	713,915
<u>Original 1934-1989 and WRAP-HYD Extended 1990-2012 Dataset</u>			
P382411	29,845	29,845 – 108,419	108,419
CP28	74,460	74,460 – 250,583	250,583
CP29	121,364	121,364 – 402,324	402,324
CP32	149,603	149,603 – 457,485	457,485
CP35	35,672	35,672 – 132,946	132,946
CP37	231,340	231,340 – 765,797	765,797

The subsistence flow standard is normally applied similarly in the two river basins. However, for control points located in the San Antonio River Basin, the subsistence flow standard is applicable during dry hydrologic conditions when measured stream flows fall below the subsistence flow level (Wurbs *et al.*, 2014c). Table 7.11 shows the subsistence flow levels vary seasonally.

In both basins, if the measured stream flow is above the subsistence flow level and below the base flow level, then junior water rights must pass the subsistence flow plus 50% of the difference between the measured stream flow and the subsistence flow (Wurbs *et al.*, 2014c). The based flow levels are seasonally various for the control points in the Guadalupe River Basin, as listed in Table 7.12, and the control points in the San Antonio River Basin have seasonally various the base flow levels and according to hydrologic conditions, as listed in Table 7.13.

Table 7.11 Subsistence Flow Standards (cfs) in the GSA River Basins

WAM CP ID	Winter	Spring	Summer	Fall
CP01E	31	18	2	25
CP02E	18	18	18	18
CP08E	10	13	8	10
CP10E	89	89	73	81
CP11E	3	2	1	1
C38461E	210	210	210	180
CP13E	4	1	1	2
CP14E	130	120	130	86
CP15E	160	130	150	110
P38241E	6	7	1	2
CP28E	14	12	8	13
CP29E	60	60	60	60
CP32E	60	60	60	60
CP35E	8	8	8	8
CP37E	60	60	60	60

Table 7.12 Base Flow Standards (cfs) in the Guadalupe River Basin

WAM CP ID	Winter	Spring	Summer	Fall
CP01E	110	100	75	110
CP02E	160	160	110	150
CP08E	52	64	56	64
CP10E	210	220	220	200
CP11E	12	10	5	8
C38461E	796	791	727	746
CP13E	12	9	4	9
CP14E	980	940	800	870
CP15E	975	945	795	865

Table 7.13 Base Flow Standards (cfs) in the San Antonio River Basin

WAM CP ID	Winter			Spring			Summer			Fall		
	Dry	Avg	Wet	Dry	Avg	Wet	Dry	Avg	Wet	Dry	Avg	Wet
P38241E	17	32	54	10	22	48	6	16	41	16	33	49
CP28E	20	53	71	37	62	77	33	57	72	27	60	74
CP29E	115	262	328	106	237	364	87	178	341	92	223	367
CP32E	152	292	424	137	264	467	113	199	430	117	246	479
CP35E	20	28	39	16	28	44	11	20	37	13	24	40
CP37E	200	329	469	174	313	502	139	237	481	167	280	584

Table 7.14 lists high flow pulse standard in the GSA River Basins. For control points in the Guadalupe River Basin and control points P38241E and CP28E in the San Antonio River Basin, two "small" and one "large" high flow pulse events per season are specified (Wurbs *et al.*, 2014c). For the remaining four control points in the San Antonio River Basin, one or two "small" and two or three "large" pulses per season are specified (Wurbs *et al.*, 2014c).

Table 7.14 also lists the months included in each season for the "large" pulse events differ from those for the other environmental flow components. Water right permits issued after the effective date of the environmental flow standards are not required to protect a high flow pulse event if the diversion rate for the water right is less than 20% of the trigger magnitude for the high flow pulse event (Wurbs *et al.*, 2014c).

Table 7.14 High Flow Pulse Standards for the GSA River Basins

WAM CP ID	Season	Pulse Flow Criteria	Pulse Flow	
			Small	Large
CP01E	Winter	Trigger (cfs)	140	350
		Volume (ac-ft)	1,030	3,390
		Duration (days)	11	20
		Frequency	2	1
	Spring	Trigger (cfs)	400	1,190
		Volume (ac-ft)	2,980	8,950
		Duration (days)	17	26
		Frequency	2	1
	Summer	Trigger (cfs)	160	570
		Volume (ac-ft)	1,130	4,110
		Duration (days)	12	19
		Frequency	2	1
	Fall	Trigger (cfs)	160	500
		Volume (ac-ft)	1,110	4,060
		Duration (days)	13	24
		Frequency	2	1

Table 7.14 (Continued)

WAM CP ID	Season	Pulse Flow Criteria	Pulse Flow	
			Small	Large
CP02E	Winter	Trigger (cfs)	210	570
		Volume (ac-ft)	1,520	5,150
		Duration (days)	11	19
		Frequency	2	1
	Spring	Trigger (cfs)	870	2,310
		Volume (ac-ft)	6,500	17,500
		Duration (days)	19	26
		Frequency	2	1
	Summer	Trigger (cfs)	240	870
		Volume (ac-ft)	1,520	5,970
		Duration (days)	11	19
		Frequency	2	1
	Fall	Trigger (cfs)	230	1,000
		Volume (ac-ft)	1,660	8,060
		Duration (days)	12	23
		Frequency	2	1
CP08E	Winter	Trigger (cfs)	54	380
		Volume (ac-ft)	360	3,840
		Duration (days)	10	28
		Frequency	2	1
	Spring	Trigger (cfs)	360	960
		Volume (ac-ft)	2,370	6,540
		Duration (days)	18	26
		Frequency	2	1
	Summer	Trigger (cfs)	74	190
		Volume (ac-ft)	410	1,130
		Duration (days)	9	13
		Frequency	2	1
	Fall	Trigger (cfs)	82	440
		Volume (ac-ft)	500	3,220
		Duration (days)	10	21
		Frequency	2	1

Table 7.14 (Continued)

WAM CP ID	Season	Pulse Flow Criteria	Pulse Flow	
			Small	Large
CP10E	Winter	Trigger (cfs)	340	1,330
		Volume (ac-ft)	1,800	11,400
		Duration (days)	8	23
		Frequency	2	1
	Spring	Trigger (cfs)	1,140	1,999
		Volume (ac-ft)	6,800	18,000
		Duration (days)	14	21
		Frequency	2	1
	Summer	Trigger (cfs)	240	500
		Volume (ac-ft)	1,090	2,670
		Duration (days)	6	9
		Frequency	2	1
	Fall	Trigger (cfs)	540	1,710
		Volume (ac-ft)	2,740	11,200
		Duration (days)	9	18
		Frequency	2	1
CP11E	Winter	Trigger (cfs)	350	1,470
		Volume (ac-ft)	1,800	6,870
		Duration (days)	17	23
		Frequency	2	1
	Spring	Trigger (cfs)	720	2,100
		Volume (ac-ft)	3,300	8,860
		Duration (days)	17	21
		Frequency	2	1
	Summer	Trigger (cfs)	48	230
		Volume (ac-ft)	230	1,080
		Duration (days)	10	15
		Frequency	2	1
	Fall	Trigger (cfs)	150	750
		Volume (ac-ft)	720	3,280
		Duration (days)	13	17
		Frequency	2	1

Table 7.14 (Continued)

WAM CP ID	Season	Pulse Flow Criteria	Pulse Flow	
			Small	Large
C38461E	Winter	Trigger (cfs)	1,150	4,140
		Volume (ac-ft)	9,640	48,300
		Duration (days)	13	29
		Frequency	2	1
	Spring	Trigger (cfs)	3,250	4,154
		Volume (ac-ft)	26,900	50,000
		Duration (days)	17	24
		Frequency	2	1
	Summer	Trigger (cfs)	950	1,760
		Volume (ac-ft)	7,060	14,800
		Duration (days)	10	14
		Frequency	2	1
	Fall	Trigger (cfs)	1,410	4,154
		Volume (ac-ft)	11,400	41,200
		Duration (days)	13	23
		Frequency	2	1
CP13E	Winter	Trigger (cfs)	300	770
		Volume (ac-ft)	1,880	4,840
		Duration (days)	16	21
		Frequency	2	1
	Spring	Trigger (cfs)	440	770
		Volume (ac-ft)	2,710	4,840
		Duration (days)	18	21
		Frequency	2	1
	Summer	Trigger (cfs)	59	250
		Volume (ac-ft)	330	1,430
		Duration (days)	11	16
		Frequency	2	1
	Fall	Trigger (cfs)	150	570
		Volume (ac-ft)	960	3,650
		Duration (days)	14	18
		Frequency	2	1

Table 7.14 (Continued)

WAM CP ID	Season	Pulse Flow Criteria	Pulse Flow	
			Small	Large
CP14E	Winter	Trigger (cfs)	1,610	4,610
		Volume (ac-ft)	14,100	55,300
		Duration (days)	13	26
		Frequency	2	1
	Spring	Trigger (cfs)	3,370	8,870
		Volume (ac-ft)	31,800	100,000
		Duration (days)	18	30
		Frequency	2	1
	Summer	Trigger (cfs)	1,050	2,110
		Volume (ac-ft)	8,300	19,300
		Duration (days)	12	17
		Frequency	2	1
	Fall	Trigger (cfs)	1,730	5,200
		Volume (ac-ft)	14,100	54,700
		Duration (days)	13	23
		Frequency	2	1
CP15E	Winter	Trigger (cfs)	1,690	3,240
		Volume (ac-ft)	14,400	33,000
		Duration (days)	13	18
		Frequency	2	1
	Spring	Trigger (cfs)	3,240	3,240
		Volume (ac-ft)	33,000	43,500
		Duration (days)	18	25
		Frequency	2	1
	Summer	Trigger (cfs)	1,040	2,060
		Volume (ac-ft)	8,570	19,200
		Duration (days)	11	16
		Frequency	2	1
	Fall	Trigger (cfs)	1,880	3,240
		Volume (ac-ft)	15,600	35,500
		Duration (days)	13	23
		Frequency	2	1

Table 7.14 (Continued)

WAM CP ID	Season	Pulse Flow Criteria	Pulse Flow	
			Small	Large
P38241E	Winter	Trigger (cfs)	53	110
		Volume (ac-ft)	400	960
		Duration (days)	12	17
		Frequency	2	1
	Spring	Trigger (cfs)	110	480
		Volume (ac-ft)	900	4,190
		Duration (days)	17	28
		Frequency	2	1
	Summer	Trigger (cfs)	94	340
		Volume (ac-ft)	670	2,310
		Duration (days)	14	21
		Frequency	2	1
	Fall	Trigger (cfs)	68	220
		Volume (ac-ft)	500	1,930
		Duration (days)	14	24
		Frequency	2	1
CP28E	Winter	Trigger (cfs)	120	350
		Volume (ac-ft)	970	3,570
		Duration (days)	15	27
		Frequency	2	1
	Spring	Trigger (cfs)	380	1,000
		Volume (ac-ft)	2,680	7,950
		Duration (days)	17	27
		Frequency	2	1
	Summer	Trigger (cfs)	140	440
		Volume (ac-ft)	860	3,050
		Duration (days)	12	21
		Frequency	2	1
	Fall	Trigger (cfs)	130	450
		Volume (ac-ft)	930	3,890
		Duration (days)	14	28
		Frequency	2	1

Table 7.14 (Continued)

WAM CP ID	Season	Pulse Flow Criteria	Pulse Flow	
			Small	Large
CP29E	Winter	Trigger (cfs)	830	
		Volume (ac-ft)	6,210	
		Duration (days)	14	
		Frequency	1	
	Spring	Trigger (cfs)	1,560	
		Volume (ac-ft)	10,700	
		Duration (days)	16	
		Frequency	1	
	Summer	Trigger (cfs)	1,110	
		Volume (ac-ft)	6,460	
		Duration (days)	12	
		Frequency	1	
	Fall	Trigger (cfs)	1,010	
		Volume (ac-ft)	6,570	
		Duration (days)	13	
		Frequency	1	
	April through June	Trigger (cfs)		3,000
		Volume (ac-ft)		6,000
		Duration (days)		2
		Frequency		3
	May through June	Trigger (cfs)		4,000
		Volume (ac-ft)		8,000
		Duration (days)		2
		Frequency		2
	July through November	Trigger (cfs)		4,000
		Volume (ac-ft)		8,000
		Duration (days)		2
		Frequency		2

Table 7.14 (Continued)

WAM CP ID	Season	Pulse Flow Criteria	Pulse Flow	
			Small	Large
CP32E	Winter	Trigger (cfs)	830	
		Volume (ac-ft)	6,330	
		Duration (days)	16	
		Frequency	1	
	Spring	Trigger (cfs)	1,670	
		Volume (ac-ft)	12,300	
		Duration (days)	19	
		Frequency	1	
	Summer	Trigger (cfs)	1,030	
		Volume (ac-ft)	6,440	
		Duration (days)	14	
		Frequency	1	
	Fall	Trigger (cfs)	850	
		Volume (ac-ft)	5,690	
		Duration (days)	14	
		Frequency	1	
	April through June	Trigger (cfs)		4,000
		Volume (ac-ft)		8,000
		Duration (days)		2
		Frequency		3
	February through April	Trigger (cfs)		4,000
		Volume (ac-ft)		8,000
		Duration (days)		2
		Frequency		2
	July through November	Trigger (cfs)		6,500
		Volume (ac-ft)		13,000
		Duration (days)		2
		Frequency		2

Table 7.14 (Continued)

WAM CP ID	Season	Pulse Flow Criteria	Pulse Flow	
			Small	Large
CP35E	Winter	Trigger (cfs)	570	
		Volume (ac-ft)	3,200	
		Duration (days)	20	
		Frequency	1	
	Spring	Trigger (cfs)	N/A	
		Volume (ac-ft)	N/A	
		Duration (days)	N/A	
		Frequency	N/A	
	Summer	Trigger (cfs)	390	
		Volume (ac-ft)	1,990	
		Duration (days)	15	
		Frequency	1	
	Fall	Trigger (cfs)	190	
		Volume (ac-ft)	1,000	
		Duration (days)	13	
		Frequency	2	
	April through June	Trigger (cfs)		1,000
		Volume (ac-ft)		2,000
		Duration (days)		2
		Frequency		3
	July through October	Trigger (cfs)		1,000
		Volume (ac-ft)		2,000
		Duration (days)		2
		Frequency		2
	July through November	Trigger (cfs)		2,500
		Volume (ac-ft)		5,000
		Duration (days)		2
		Frequency		2

Table 7.14 (Continued)

WAM CP ID	Season	Pulse Flow Criteria	Pulse Flow	
			Small	Large
CP37E	Winter	Trigger (cfs)	1,520	
		Volume (ac-ft)	12,800	
		Duration (days)	19	
		Frequency	1	
	Spring	Trigger (cfs)	1,570	
		Volume (ac-ft)	11,300	
		Duration (days)	16	
		Frequency	2	
	Summer	Trigger (cfs)	1,640	
		Volume (ac-ft)	11,200	
		Duration (days)	16	
		Frequency	1	
	Fall	Trigger (cfs)	2,320	
		Volume (ac-ft)	17,600	
		Duration (days)	19	
		Frequency	1	
	April through June	Trigger (cfs)		4,000
		Volume (ac-ft)		8,000
		Duration (days)		2
		Frequency		3
	February through April	Trigger (cfs)		4,000
		Volume (ac-ft)		8,000
		Duration (days)		2
		Frequency		2
	July through November	Trigger (cfs)		8,000
		Volume (ac-ft)		16,000
		Duration (days)		2
		Frequency		2

7.2 Modeling Environmental Flow Standards

Wurbs and Hoffpauir (2013b) outline capabilities for incorporating environmental flow standards into water availability modeling using the new daily WRAP/WAM modeling system. This section briefly introduces the methodologies that were recently employed to model the SB3 environmental flow standards in the three daily WAM reports (Wurbs *et al.*, 2014a, 2014b and 2014c). Thus, detailed explanations for each record, used for the SB3 modeling for the three daily WAMs can be referred to the three report of “Daily Water Availability Model for the Sabine, Neches, GSA River Basins”, respectively. The evaluations of environmental flow standards for the basins are performed through the modification of the options based on the three daily WAM systems.

7.2.1 Sabine WAM

The method for modeling of environmental flow standards in the Sabine WAM is illustrated by the input records used to model the instream flow requirements for control point BSBSE, as an example of modeling. The same modeling methodology was used for all five control points as follows:

- Target setting water right *WR* records along with flow switch *FS*, target options *TO*, daily data *DW*, and daily options *DO* records explained in the WRAP user manuals were used for the modeling of subsistence and base flow standards.
- A target setting water right *WR* record in combination with pulse flow *PF* records and pulse flow options *PO* records was used for the modeling of pulse flow standards.
- An instream flow *IF* record with a target equal to the maximum of the targets set by the target setting water right records was used for setting the instream flow target.
- A priority number of 50000000 was assigned to the water rights modeling the SB3 instream flow standards to make them junior to all other water rights in the WAM.

The flow standards have a priority date of November 30, 2009. However, there water rights related to the sharing of Toledo Bend by the states of Texas and Louisiana that are modeled with a priority of 30001231 to designate them as being junior to other

rights in the Sabine WAM includes. Therefore, the SB3 environmental flow standards are assigned a priority of 50000000 to make them the most junior rights in the WAM (Wurbs *et al.*, 2014a). Table 7.15 summarizes the environmental flow standards for control point BSBSE, Big Sandy Creek near Big Sandy, that consist of seasonal subsistence flow, base flow, and high flow pulse requirements.

Table 7.15 Environmental Flow Standards for Control Point BSBSE

Season	Subsistence (cfs)	Base (cfs)	Pulse	
Winter	20	73	Trigger (cfs):	358
			Volume (ac-ft):	5,932
			Duration (days):	10
			Frequency	1
Spring	9	33	Trigger (cfs):	313
			Volume (ac-ft):	5,062
			Duration (days):	13
			Frequency	2
Summer	8	15	Trigger (cfs):	50
			Volume (ac-ft):	671
			Duration (days):	6
			Frequency	1
Fall	8	22	Trigger (cfs):	130
			Volume (ac-ft):	2,189
			Duration (days):	9
			Frequency	2

Table 7.16 reproduces the input records used for modeling the environmental flow standards for control point BSBSE. These are categorized into four sections as follows (Wurbs *et al.*, 2014a):

- Section 1. Use coefficient *UC* records are used to identify the months in each season.
- Section 2. Daily subsistence and base flow targets are set. Eight target setting water rights are implemented corresponding to subsistence and base flow targets for four seasons.

Section 3. The daily high flow pulse target is set using a target setting water right and a series of *PF* and *PO* records. A target of zero is set if no pulse events are triggered.

Section 4. The final daily instream flow target is set. An instream flow *IF* record adopts the maximum target set by the target setting water rights from Sections 2 and 3.

Table 7.16 Input Records for Environmental Flow Standards
for Control Point BSBSE

```

**
** Section 1 - Use coefficient records used to identify seasons
**
UCWINTER      1      1      1      0      0      0
UC             0      0      0      0      0      0
UCSPRING      0      0      0      1      1      1
UC             0      0      0      0      0      0
UCSUMMER      0      0      0      0      0      0
UC             1      1      1      0      0      0
UC FALL       0      0      0      0      0      0
UC             0      0      0      1      1      1
**
** Section 2 - Subsistence and base flow targets
**
** Subsistence flows
WR BSBSE 9999999 WINTER50000000 8      BSBSE_SUB_WIN
TO 15 39.669 MIN
DO 16
WR BSBSE 9999999 SPRING50000000 8      BSBSE_SUB_SPR
TO 15 17.851 MIN
DO 16
WR BSBSE 9999999 SUMMER50000000 8      BSBSE_SUB_SMR
TO 15 15.868 MIN
DO 16
WR BSBSE 9999999 FALL50000000 8      BSBSE_SUB_FAL
TO 15 15.868 MIN
DO 16
** Base flows
WR BSBSE 434.38 WINTER50000000 8      BSBSE_BASE_WIN
FS 1      0.0      1.0      144.79 1      0      1
DO 19
DW 2
WR BSBSE 196.36 SPRING50000000 8      BSBSE_BASE_SPR
FS 1      0.0      1.0      65.45 1      0      1
DO 19
DW 2
WR BSBSE 89.26 SUMMER50000000 8      BSBSE_BASE_SMR

```

Table 7.16 (Continued)

FS	1		0.0	1.0		29.75	1		0		1
DO			19								
DW		2									
WR BSBSE		130.91	FALL50000000	8					BSBSE_BASE_FAL		
FS	1		0.0	1.0		43.64	1		0		1
DO			19								
DW		2									
WR BSBSE		0	50000000	8					BSBSE_BASEFLOW		
TO	13		MAX						BSBSE_SUB_WIN	CONT	
TO	13		MAX						BSBSE_SUB_SPR	CONT	
TO	13		MAX						BSBSE_SUB_SMR	CONT	
TO	13		MAX						BSBSE_SUB_FAL	CONT	
TO	13		MAX						BSBSE_BASE_WIN	CONT	
TO	13		MAX						BSBSE_BASE_SPR	CONT	
TO	13		MAX						BSBSE_BASE_SMR	CONT	
TO	13		MAX						BSBSE_BASE_FAL		
DO		16									
** Section 3 - Pulse flow targets											
** Pulse flows											
WR BSBSE		0	50000000	8					BSBSE_PULSE		
PF	0	BSBSE	710.083	5932	10	1	1	3	2	4	
BSBSE_WIN											
PO		2									
PF	0	BSBSE	620.826	5062	13	2	4	6	2	4	
BSBSE_SPR											
PO		2									
PF	0	BSBSE	99.174	671	6	1	7	9	2	4	
BSBSE_SMR											
PO		2									
PF	0	BSBSE	257.851	2189	9	2	10	12	2	4	
BSBSE_FAL											
PO		2									
** Section 4 - Final instream flow target											
IF BSBSE		0	50000000	2			IF-BSBSE				
TO	13		MAX						BSBSE_BASEFLOW	CONT	
TO	13		MAX						BSBSE_PULSE		
DO		16									
**											

7.2.2 Neches WAM

Table 7.17 reproduces the input records used to model the instream flow requirements for control point NENEE for demonstration purposes. The same modeling methodology was also used for all five control points. The SB3 environmental flow standards for the Neches WAM are the same as the Sabine WAM expect for a priority

number of 20091130 that was used for all instream flow *IF* and water right *WR* records, corresponding to a priority date of November 30, 2009.

Table 7.17 Input Records for Environmental Flow Standards
for Control Point NENEE

```

** Section 1 - Use coefficient records used to identify seasons
UCWINTER      1      1      1      0      0      0
UC             0      0      0      0      0      0
UCSPRING      0      0      0      1      1      1
UC             0      0      0      0      0      0
UCSUMMER      0      0      0      0      0      0
UC             1      1      1      0      0      0
UC  FALL      0      0      0      0      0      0
UC             0      0      0      1      1      1
**
** CP: NENEE, 08032000, Neches River at Neches
** Section 2 - Subsistence and Base flow Targets
** - Subsistence flows
WR NENEE 9999999 WINTER20091130 8
NENEE_SUB_WIN
TO 15 101.157 MIN
DO 16
WR NENEE 9999999 SPRING20091130 8
NENEE_SUB_SPR
TO 15 41.653 MIN
DO 16
WR NENEE 9999999 SUMMER20091130 8
NENEE_SUB_SMR
TO 15 23.802 MIN
DO 16
WR NENEE 9999999 FALL20091130 8
NENEE_SUB_FAL
TO 15 25.785 MIN
DO 16
**- Base flows
WR NENEE 1166.28 WINTER20091130 8
NENEE_BASE_WIN
FS 1 0.0 1.0 388.76 1 0
1
DO 19
DW 2
WR NENEE 571.24 SPRING20091130 8
NENEE_BASE_SPR
FS 1 0.0 1.0 190.41 1 0
1
DO 19
DW 2
WR NENEE 273.72 SUMMER20091130 8
NENEE_BASE_SMR

```

Table 7.17 (Continued)

FS	1	0.0	1.0	91.24	1	0
1						
DO		19				
DW	2					
WR NENEE	476.03	FALL20091130	8			
NENEE_BASE_FAL						
FS	1	0.0	1.0	158.68	1	0
1						
DO		19				
DW	2					
WR NENEE	0	20091130	8			
NENEE_BASEFLOW						
TO	13	MAX				NENEE_SUB_WIN
CONT						
TO	13	MAX				NENEE_SUB_SPR
CONT						
TO	13	MAX				NENEE_SUB_SMR
CONT						
TO	13	MAX				NENEE_SUB_FAL
CONT						
TO	13	MAX				NENEE_BASE_WIN
CONT						
TO	13	MAX				NENEE_BASE_SPR
CONT						
TO	13	MAX				NENEE_BASE_SMR
CONT						
TO	13	MAX				NENEE_BASE_FAL
DO	16					
** Section 3 - Pulse flow targets						
WR NENEE	0	20091130	8			NENEE_PULSE
PF	0	NENEE1652.231	19104 10 1	1 3	2 4	
NENEE_WIN						
PO	2					
PF	0	NENEE1626.446	20405 12 2	4 6	2 4	
NENEE_SPR						
PO	2					
PF	0	NENEE 224.132	1339 4 1	7 9	2 4	
NENEE_SMR						
PO	2					
PF	0	NENEE 684.298	5391 8 2	10 12	2 4	
NENEE_FAL						
PO	2					
** Section 4 - Final instream flow target						
IF NENEE	0	20091130	2	IF-NENEE		
TO	13	MAX				NENEE_BASEFLOW
CONT						
TO	13	MAX				NENEE_PULSE
DO	16					
**						

The environmental flow standards for control point NENEE also have seasonal subsistence flow, base flow, and high flow pulse requirements as shown in Table 7.18. The category of the input records is also the same as the Sabine WAM. Detailed

Table 7.18 Environmental Flow Standards for Control Point NENEE

Season	Subsistence (cfs)	Base (cfs)	Pulse	
Winter	51	196	Trigger (cfs):	833
			Volume (ac-ft):	19,104
			Duration (days):	10
			Frequency	1
Spring	21	96	Trigger (cfs):	820
			Volume (ac-ft):	20,405
			Duration (days):	12
			Frequency	2
Summer	12	46	Trigger (cfs):	113
			Volume (ac-ft):	1,339
			Duration (days):	4
			Frequency	1
Fall	13	80	Trigger (cfs):	345
			Volume (ac-ft):	5,391
			Duration (days):	8
			Frequency	2

7.2.3 GSA WAM

Two sets of input records are included in this section to illustrate the alternate methodologies employed for modeling environmental flow standards at control points located in the Guadalupe River Basin and control points in the San Antonio River Basin for which different approaches were used to define hydrologic conditions (Wurbs *et al.*, 2014c). Even though the modeling methodology was more complicated for control points in the San Antonio River Basin, a similar paradigm was implemented in both basins. The same modeling methodology was used for all fifteen control points as follows:

- Subsistence and base flow standards were modeled using target setting water right *WR* records in combination with flow switch *FS*, target options *TO*, daily data *DW*, and daily options *DO* records.
- Pulse flow standards were modeled using a target setting water right *WR* record in combination with pulse flow *PF* records and pulse flow options *PO* records.
- The instream flow target was set using an instream flow *IF* record with a target equal to the maximum of the targets set by the target setting water right records.
- A priority of 88888888 was used for the instream flow rights modeling SB3 environmental flow standards to make them junior to all other water rights. Three existing rights have priorities of 88888801, 88888802, and 88888803, which are junior to the actual priority date of March 1, 2011 for the SB3 flow standards.

Control points in the Guadalupe River Basin

Control point CP14E represents the control points in the Guadalupe River Basin. Table 7.19 tabulates The environmental flow standards for this location consist of subsistence and base flows that vary seasonally, a two-per-season high flow pulse, and a one-per-season high flow pulse. The input records for modeling the winter subsistence, base, and pulse flow requirements are representatively reproduced as listed Table 7.20. Similar records for other seasons have been omitted for brevity. The input records used for modeling the environmental flow standards for control point CP14E for the winter season are categorized as follows (Wurbs *et al.*, 2014c):

- Section 1. A use coefficient *UC* record is used to identify the months in the winter season.
- Section 2. Daily subsistence and base flow targets for the winter season are set. Three target setting water rights are used corresponding to the winter subsistence flow, the winter base flow, and intermediate flows between the winter subsistence and base flows.

- Section 3. Daily high flow pulse targets for the winter season are set. A target setting water right adopts the maximum target set by a series of *PF* and *PO* records. A target of zero is set if no high flow pulse events are triggered.
- Section 4. The daily instream flow target for the winter season is set. An *IF* record adopts the maximum target set by the target setting water rights from Sections 2 and 3.

Table 7.19 Environmental Flow Standards for Control Point CP14E

Season	Subsistence (cfs)	Base (cfs)		Small Seasonal Pulse Events	Large Seasonal Pulse Events
Winter	130	980	Trigger (af/day)	1,610	4,610
			Volume (ac-ft)	14,100	55,300
			Duration (days)	13	26
			Frequency	2	1
Spring	120	940	Trigger (cfs)	3,370	8,870
			Volume (ac-ft)	31,800	100,000
			Duration (days)	18	30
			Frequency	2	1
Summer	130	800	Trigger (cfs)	1,050	2,110
			Volume (ac-ft)	8,300	19,300
			Duration (days)	12	17
			Frequency	2	1
Fall	86	870	Trigger (cfs)	1,730	5,200
			Volume (ac-ft)	14,100	54,700
			Duration (days)	13	23
			Frequency	2	1

**Table 7.20 Input Records for Environmental Flow Standards
at Control Point CP14E for the Winter Season**

```

**
** Section 1 - Use Type Records for the Winter Season
**
UC   WIN      1      1      1      0      0      0      0      0      0
0      0      0
**
** Section 2 - Subsistence and Base Flow Targets for the Winter Season
**
WR CP14E      3.0      WIN88888888  8      CP14E_WIN
DW      2
WR CP14E 773.55      WIN88888888  8      CP14E_SUB_WIN
FS      0.0      1.0 257.85      1      0
DO      19
DW      2
WR CP14E 773.55      WIN88888888  8      CP14E_SUBBAS_WIN
TO      2      ADD      CONT
TO      15      0.5      MUL      CONT
TO      13      MUL      CP14E_WIN
FS      1.0      0.0 257.85 1943.80 1      0
DO      16      19
DW      2
WR CP14E 5831.40      WIN88888888  8      CP14E_BAS_WIN
FS      0.0      1.0      1943.80 1      0
DO      19
DW      2
**
** Section 3 - High Flow Pulse Event Targets for the Winter Season
**
WR CP14E      0      88888888  8      CP14E_PULSE
PF      0      3193.39 14100 13 2      1 3      2 4
CP14E_WINTER_S
PO      2
PF      0      9143.80 55300 26 1      1 3      2 4
CP14E_WINTER_L
PO      2
**
** Section 4 - Final Daily Instream Flow Target for the Winter Season
**
IF CP14E      88888888  2      IF-CP14E
TO      13      MAX      CP14E_SUB_WIN  CONT
TO      13      MAX      CP14E_SUBBAS_WINCONT
TO      13      MAX      CP14E_BAS_WIN  CONT
TO      13      MAX      CP14E_PULSE
DO      16

```

Control points in the San Antonio River Basin

Control point CP37E (USGS Gage 08188500) represents the control points in the San Antonio River. This control point also has the environmental flow standards that consist of subsistence and base flows that vary seasonally and according to hydrologic conditions, a set of small seasonal high flow pulses, and three large inter-seasonal high flow pulses, as listed in Table 7.21. The three levels of hydrologic conditions (dry, average, and wet) are evaluated using the hydrologic index series (HIS) input file (Wurbs *et al.*, 2014c).

Table 7.21 Environmental Flow Standards for Control Point CP37E

Season	Hydrologic Condition	Subsistence (cfs)	Base (cfs)	Small Seasonal Pulse Events	
Winter	Dry	60	200	Trigger (cfs)	1,520
	Average	N/A	329	Volume (ac-ft)	12,800
	Wet	N/A	469	Duration (days)	19
				Frequency	1
Spring	Dry	60	174	Trigger (cfs)	1,570
	Average	N/A	313	Volume (ac-ft)	11,300
	Wet	N/A	502	Duration (days)	16
				Frequency	2
Summer	Dry	60	139	Trigger (cfs)	1,640
	Average	N/A	237	Volume (ac-ft)	11,200
	Wet	N/A	481	Duration (days)	16
				Frequency	1
Fall	Dry	60	167	Trigger (cfs)	2,320
	Average	N/A	280	Volume (ac-ft)	17,600
	Wet	N/A	584	Duration (days)	19
				Frequency	1

Table 7.21 (Continued)

Season	Hydrologic Condition	Subsistence (cfs)	Base (cfs)	Small Seasonal Pulse Events	
Large Pulse Events					
April through June				Trigger (cfs)	4,000
				Volume (ac-ft)	15,867
				Duration (days)	2
				Frequency	3
February through April				Trigger (cfs)	4,000
				Volume (ac-ft)	15,867
				Duration (days)	2
				Frequency	2
July through November				Trigger (cfs)	8,000
				Volume (ac-ft)	31,735
				Duration (days)	2
				Frequency	2

Table 7.22 describes the input records for modeling the subsistence, base flow, and high flow pulse event requirements for the winter season at control point CP37E (Wurbs *et al.*, 2014c). These are the same as the input records for the spring, summer, and fall seasons. The input records are divided into the five sections as follows (Wurbs *et al.*, 2014c):

- Section 1. Use coefficient *UC* records and a target setting water right *WR* record are used to set subsistence and base flow targets for the winter season.
- Section 2. Hydrologic conditions are evaluated using information from the hydrologic index series (HIS) input file. The hydrologic conditions are evaluated once per season based on conditions on the last day of the preceding season.
- Section 3. Daily subsistence and base flow targets for the winter season are set. Five target setting water rights are implemented, corresponding to the winter subsistence flow, the winter base flows for three hydrologic

conditions, and intermediate flows between the winter subsistence and base flows.

Section 4. Daily high flow pulse event targets are set. A target setting water right adopts the maximum target set by a series of *PF* and *PO* records. A target of zero is set if no high flow pulse events are triggered.

Section 5. The final daily instream flow target for the winter season is set. An instream flow *IF* record adopts the maximum target set by the target setting water rights from Sections 3 and 4.

Table 7.22 Input Records for Environmental Flow Standards
at Control Point CP37E for the Winter Season

```

**
** Section 1 - Use Coefficients and Water Right Target for the Winter Season
**
UC  WIN      1      1      1      0      0      0      0      0      0      0      0      0
UCBEGWIN    1      0      0      0      0      0      0      0      0      0      0      0
**
WR CP37E    3.0    WIN88888888  8                      CP37E_WIN
DW          2
**
** Section 2 - Evaluation of Hydrologic Conditions for the Winter Season
**
WR CP37E          88888888  8                      CP37E_HYDCOND
TO  -16
DO          16
WR CP37E    1.0      88888888  8                      CP37E_ANREG_DRY
FS  10          1.0      0.0      0.9      1.1      1                      CP37E_HYDCOND
DO          19
DW          1
WR CP37E    1.0      88888888  8                      CP37E_ANREG_AVG
FS  10          1.0      0.0      1.9      2.1      1                      CP37E_HYDCOND
DO          19
DW          1
WR CP37E    1.0      88888888  8                      CP37E_ANREG_WET
FS  10          1.0      0.0      2.9      3.1      1                      CP37E_HYDCOND
DO          19
DW          1
WR CP37E    1.0  BEGWIN88888888  8                      CP37E_WIN_DRY
TO  13          MJL                      CP37E_ANREG_DRY
DO          16
DW          2  1
WR CP37E    1.0  BEGWIN88888888  8                      CP37E_WIN_AVG
TO  13          MJL                      CP37E_ANREG_AVG
DO          16
DW          2  1
WR CP37E    1.0  BEGWIN88888888  8                      CP37E_WIN_WET
TO  13          MJL                      CP37E_ANREG_WET
DO          16

```

Table 7.22 (Continued)

DW 2 1

**

** Section 3 - Subsistence and Base Flow Targets for the Winter Season

**

WR CP37E	357.02	WIN88888888	8				CP37E_SUB_WIN	
FS 10		1.0	0.0	1.0	1	92		CP37E_WIN_DRY
FS 1		0.0	1.0	119.01	1	0		
DO	19							
DW	2							
WR CP37E	357.02	WIN88888888	8				CP37E_SUBBAS_WIN	
TO 2		ADD						CONT
TO 15	0.5	MUL						CONT
TO 13		MUL				CP37E_WIN		
FS 10		1.0	0.0	1.0	1	92		CP37E_WIN_DRY
FS 1		1.0	0.0	119.01	396.69	1	0	
DO	16	19						
DW	2							
WR CP37E	1190.08	WIN88888888	8				CP37E_BASED_WIN	
FS 10		1.0	0.0	1.0	1	92		CP37E_WIN_DRY
FS 1		0.0	1.0		396.69	1	0	
DO	19							
DW	2							
WR CP37E	1957.69	WIN88888888	8				CP37E_BASEFA_WIN	
FS 10		1.0	0.0	1.0	1	92		CP37E_WIN_AVG
DO	19							
DW	2							
WR CP37E	2790.74	WIN88888888	8				CP37E_BASEW_WIN	
FS 10		1.0	0.0	1.0	1	92		CP37E_WIN_WET
DO	19							
DW	2							

**

** Section 4 - High Flow Pulse Event Targets for the Winter Season

**

WR CP37E	0	88888888	8				CP37E_PULSE	
PF 0		3014.88	12800	19	1	3	2	4
PF 0		3114.05	11300	16	2	4	2	4
PF 0		3252.89	11200	16	1	7	2	4
PF 0		4601.65	17600	19	1	10	2	4
PF 0		7933.88	15867	2	3	4	2	4
PO	2							
PF 0		7933.88	15867	2	2	2	2	4
PO	2							
PF 0		15867.77	31735	2	2	7	2	4
PO	2							

**

** Section 5 - Final Daily Instream Flow Target for the Winter Season

**

IF CP37E		88888888	2		IF-CP37E
TO 13	MAX				CP37E_SUB_WIN CONT
TO 13	MAX				CP37E_SUBBAS_WINCONT
TO 13	MAX				CP37E_BASED_WIN CONT
TO 13	MAX				CP37E_BASEFA_WIN CONT
TO 13	MAX				CP37E_BASEW_WIN CONT
TO 13	MAX				CP37E_PULSE
DO	16				

7.3 SB3 Environmental Flow Target

The monthly naturalized datasets, daily flow patterns, water use scenarios, and environmental flow standards in the Sabine, Neches, and GSA River Basins are incorporated in each daily WRAP model for river system simulations. In this section, daily naturalized flows, regulated flows, environmental flow targets and associated shortages in meeting the flow targets at the control points, where the environmental flow standards have been established, are developed by each WAM model for investigating the flow characteristics and alterations. The letter “E” is added to the control point identifiers. The “E” control points have the only water rights that are IF record rights modeling the SB3 environmental flow standards at the same locations of the control points.

Daily naturalized flow sequences are disaggregated based on daily flow patterns that are developed by the calibrated daily SWAT model, described in Chapter VI. A daily WRAP/WAM model develops regulated flows, influencing the naturalized flows by various human impacts such as reservoir storage, water diversions etc. based on water use scenarios using naturalized flow datasets. The environmental flow target adopted each day is the maximum of the high pulse flows target, which is often zero, and the combined subsistence and base flow target.

7.3.1 Sabine WAM

The daily WRAP model for the Sabine WAM develops four different daily flow sequences, mentioned above at the five control points, listed in Table 6.2. The simulation covers the period-of-analysis (1940-2013), and a full authorized use scenario is adopted. The flow frequency metrics are developed using the four different daily flow sequences for examining each flow sequence at the five control points.

Table 7.23 presents the flow frequency metrics for naturalized flow, regulated flow, SB3 environmental flow target and shortage in meeting the SB3 environmental target at the control point BSBSE. There is the Winnsboro Lake upstream of the control point, but the flow frequency metrics of both naturalized and regulated flows indicate that the hydrologic alterations are insignificant at the control point.

Table 7.23 Flow Frequency Metrics (acre-feet/day) for Control Point BSBSE

	Naturalized Flow	Regulated Flow	<u>Subsidence and Base</u>		<u>Total Including Pulse</u>	
			IF Target	IF Shortage	IF Target	IF Shortage
Mean	368.8	359.0	57.6	1.0	79.6	1.0
Stan Dev	731.8	730.1	43.2	3.9	107.7	3.9
Minimum	0.0	0.0	15.9	0.0	15.9	0.0
99.50%	0.0	0.0	15.9	0.0	15.9	0.0
99%	0.1	0.3	15.9	0.0	15.9	0.0
98%	2.2	2.2	15.9	0.0	15.9	0.0
95%	10.9	10.2	15.9	0.0	15.9	0.0
90%	21.3	19.5	15.9	0.0	15.9	0.0
85%	30.4	27.5	17.9	0.0	17.9	0.0
80%	41.5	36.4	29.8	0.0	29.8	0.0
75%	53.1	46.8	29.8	0.0	29.8	0.0
70%	65.8	57.9	29.8	0.0	29.8	0.0
60%	98.1	87.2	39.7	0.0	39.7	0.0
50%	141.2	129.0	43.6	0.0	43.6	0.0
40%	200.1	186.1	65.5	0.0	65.5	0.0
30%	297.0	283.5	65.5	0.0	65.5	0.0
25%	370.8	356.5	65.5	0.0	65.5	0.0
20%	467.4	453.4	144.8	0.0	144.8	0.0
15%	608.8	596.5	144.8	0.0	144.8	0.0
10%	869.2	855.0	144.8	0.0	144.8	0.0
5%	1,452	1,436	144.8	8.9	249.8	8.9
2%	2,585	2,571	144.8	15.5	620.8	15.5
1%	3,690	3,689	144.8	17.3	620.8	17.3
0.50%	5,071	5,064	144.8	25.2	710.1	25.2
Maximum	16,928	16,956	144.8	39.7	710.1	39.7

The regulated flow has totally different flow frequency metrics than the naturalized flow at the control point SRGWE due to water uses and flow controls by the two large dams, as listed in Table 7.24. The regulated flow can meet the environmental flow target engaged by the WRAP model for 90 % of the period-of-analysis at the control point.

Table 7.24 Flow Frequency Metrics (acre-feet/day) for Control Point SRGWE

	Naturalized Flow	Regulated Flow	<u>Subsidence and Base</u>		<u>Total Including Pulse</u>	
			IF Target	IF Shortage	IF Target	IF Shortage
Mean	4,157	2,777	221.0	2.8	396.1	2.8
Stan Dev	6,958	5,109	207.4	10.5	724.6	10.5
Minimum	2.9	0.0	27.8	0.0	27.8	0.0
99.50%	11.0	0.0	27.8	0.0	27.8	0.0
99%	15.9	0.0	27.8	0.0	27.8	0.0
98%	25.1	0.0	27.8	0.0	27.8	0.0
95%	49.0	0.0	27.8	0.0	27.8	0.0
90%	109.4	33.5	33.7	0.0	33.7	0.0
85%	194.9	81.0	43.6	0.0	43.6	0.0
80%	297.4	138.2	73.4	0.0	73.4	0.0
75%	410.5	210.5	73.4	0.0	73.4	0.0
70%	544.2	292.5	73.4	0.0	73.4	0.0
60%	939.6	532.9	107.1	0.0	107.1	0.0
50%	1,563	932.2	107.1	0.0	107.1	0.0
40%	2,553	1,565	259.8	0.0	259.8	0.0
30%	4,034	2,551	259.8	0.0	259.8	0.0
25%	4,953	3,239	605.0	0.0	605.0	0.0
20%	6,347	4,096	605.0	0.0	605.0	0.0
15%	8,196	5,387	605.0	0.0	605.0	0.0
10%	10,976	7,391	605.0	0.1	605.0	0.1
5%	16,233	11,280	605.0	27.8	2,612	27.8
2%	25,261	17,716	605.0	33.7	3,134	33.7
1%	34,659	24,407	605.0	43.6	3,729	43.6
0.50%	43,550	31,092	605.0	89.3	3,729	89.3
Maximum	109,712	108,543	605.0	89.3	3,729	89.3

The regulated flow decreases considerably relative to the naturalized flow at control point SRBEE due to upstream water uses and flow controls by several dams, as listed in Table 7.25. The shortage of the environmental flow target is 2.0 acre-feet/day in average, and 130.9 acre-feet/day in maximum at the control points.

Table 7.25 Flow Frequency Metrics (acre-feet/day) for Control Point SRBEE

	Naturalized Flow	Regulated Flow	Subsidence and Base		Total Including Pulse	
			IF Target	IF Shortage	IF Target	IF Shortage
Mean	5,516	3,845	356.2	2.0	621.7	2.0
Stan Dev	8,016	6,176	332.0	10.2	1,064	10.2
Minimum	2.1	0.0	43.6	0.0	43.6	0.0
99.50%	22.0	0.0	43.6	0.0	43.6	0.0
99%	31.8	0.0	43.6	0.0	43.6	0.0
98%	47.3	3.5	43.6	0.0	43.6	0.0
95%	128.8	35.8	43.6	0.0	43.6	0.0
90%	250.2	107.7	43.6	0.0	43.6	0.0
85%	390.5	183.9	111.1	0.0	111.1	0.0
80%	535.6	269.3	111.1	0.0	111.1	0.0
75%	719.8	324.3	111.1	0.0	111.1	0.0
70%	927.7	449.7	111.1	0.0	111.1	0.0
60%	1,558	851.6	164.6	0.0	164.6	0.0
50%	2,485	1,493	164.6	0.0	164.6	0.0
40%	3,925	2,456	446.3	0.0	446.3	0.0
30%	5,803	3,781	446.3	0.0	446.3	0.0
25%	7,113	4,775	956.0	0.0	956.0	0.0
20%	8,900	6,060	956.0	0.0	956.0	0.0
15%	11,301	7,937	956.0	0.0	956.0	0.0
10%	14,624	10,684	956.0	0.0	956.0	0.0
5%	20,146	15,269	956.0	9.9	3,843	9.9
2%	28,296	21,624	956.0	41.1	4,284	41.1
1%	36,802	27,921	956.0	43.6	5,752	43.6
0.50%	47,837	36,448	956.0	55.5	5,752	55.5
Maximum	102,609	101,136	956.0	130.9	5,752	130.9

Both naturalized and regulated flows have almost identical flow frequency metrics at the control point 29500E, as presented in Table 7.26. This means that there are no water uses and flow controllers upstream of the control point. The exceedance frequency of the environmental flow shortage is 15% similar to the control point SRRLE. This is slightly higher than other control points in the Sabine River Basin.

Table 7.26 Flow Frequency Metrics (acre-feet/day) for Control Point 29500E

	Naturalized Flow	Regulated Flow	Subsidence and Base		Total Including Pulse	
			IF Target	IF Shortage	IF Target	IF Shortage
Mean	382.6	382.6	75.3	4.0	95.0	4.0
Stan Dev	781.9	781.9	28.8	10.7	122.1	10.7
Minimum	0.0	0.0	39.7	0.0	39.7	0.0
99.50%	0.0	0.0	39.7	0.0	39.7	0.0
99%	0.0	0.0	39.7	0.0	39.7	0.0
98%	0.0	0.0	39.7	0.0	39.7	0.0
95%	6.7	6.7	39.7	0.0	39.7	0.0
90%	23.1	23.1	39.7	0.0	39.7	0.0
85%	36.3	36.3	39.7	0.0	39.7	0.0
80%	50.3	50.3	39.7	0.0	39.7	0.0
75%	65.5	65.5	55.5	0.0	55.5	0.0
70%	84.0	84.0	61.5	0.0	61.5	0.0
60%	131.1	131.1	61.5	0.0	61.5	0.0
50%	184.5	184.5	79.3	0.0	79.3	0.0
40%	256.6	256.6	83.3	0.0	83.3	0.0
30%	356.3	356.3	83.3	0.0	83.3	0.0
25%	425.6	425.6	83.3	0.0	83.3	0.0
20%	516.1	516.1	123.0	0.0	123.0	0.0
15%	650.0	650.0	123.0	5.4	123.0	5.4
10%	854.2	854.2	123.0	17.9	123.0	17.9
5%	1,312	1,312	123.0	34.2	123.0	34.2
2%	2,172	2,172	123.0	39.7	638.7	39.7
1%	3,117	3,117	123.0	39.7	694.2	39.7
0.50%	4,486	4,486	123.0	55.5	868.8	55.5
Maximum	31,932	31,932	123.0	55.5	1,375	55.5

Table 7.27 presents the flow frequency metrics at control point SRRLE. There are different statistic parameters and flow frequencies between the naturalized and regulated flows at the control point. Environmental flows are engaged as 785.5 acre-feet/day in minimum, and 6,704 acre-feet/day in maximum.

Table 7.27 Flow Frequency Metrics (acre-feet/day) for Control Point SRRLE

	Naturalized Flow	Regulated Flow	Subsidence and Base		Total Including Pulse	
			IF Target	IF Shortage	IF Target	IF Shortage
Mean	17,153	11,000	1,918	95.0	2,045	95.0
Stan Dev	19,677	16,777	922	278.5	1,154	278.5
Minimum	120.1	0.0	785.5	0.0	785.5	0.0
99.50%	359.5	0.0	785.5	0.0	785.5	0.0
99%	491.6	0.0	785.5	0.0	785.5	0.0
98%	645.9	0.0	785.5	0.0	785.5	0.0
95%	1,068	206.7	785.5	0.0	785.5	0.0
90%	1,723	441.8	785.5	0.0	785.5	0.0
85%	2,299	766.1	785.5	0.0	785.5	0.0
80%	2,958	1,111	864.8	0.0	864.8	0.0
75%	3,723	1,459	864.8	0.0	864.8	0.0
70%	4,556	1,792	1,462	0.0	1,462	0.0
60%	6,764	2,737	1,605	0.0	1,605	0.0
50%	9,798	4,066	1,605	0.0	1,605	0.0
40%	14,324	6,207	2,636	0.0	2,636	0.0
30%	20,609	10,342	2,636	0.0	2,636	0.0
25%	24,337	13,414	2,636	0.0	2,636	0.0
20%	28,975	17,770	3,316	0.0	3,316	0.0
15%	34,545	23,679	3,316	101	3,316	101
10%	41,968	31,665	3,316	417	3,316	417
5%	54,257	44,148	3,316	668	3,316	668
2%	73,701	61,468	3,316	865	6,223	865
1%	90,381	79,070	3,316	1,656	6,446	1,656
0.50%	106,170	92,610	3,316	1,882	6,667	1,882
Maximum	278,369	254,852	3,316	1,882	6,704	1,882

7.3.2 Neches WAM

The SIMD and TABLES programs of the WRAP modeling system are applied with the Neches WAM to develop four different daily flow sequences and their flow frequency metrics, respectively at the five control points, listed in Table 6.3. The simulation covers the 1940-2013 period-of-analysis, and a full authorized use scenario is adopted.

Table 7.28 presents the flow frequency metrics for naturalized flow, regulated flow, SB3 environmental flow target and shortage in meeting the SB3 environmental target at the control point NENEE. There is the significant difference between the mean values of the naturalized and regulated flows at the control points. This is attributed to the flow control of Palestine Lake and water uses. The minimum shortage of the environmental flow target occurs at 10 % of the exceedance frequency.

Table 7.28 Flow Frequency Metrics (acre-feet/day) for Control Point NENEE

	Naturalized Flow	Regulated Flow	<u>Subsidence and Base</u>		<u>Total Including Pulse</u>	
			IF Target	IF Shortage	IF Target	IF Shortage
Mean	1,509	822.0	137.5	4.7	197.6	4.7
Stan Dev	2,754	1,900	114.6	16.2	295.9	16.2
Minimum	0.0	0.0	23.8	0.0	23.8	0.0
99.50%	0.0	0.0	23.8	0.0	23.8	0.0
99%	0.0	0.0	23.8	0.0	23.8	0.0
98%	0.0	0.0	23.8	0.0	23.8	0.0
95%	0.0	17.1	23.8	0.0	23.8	0.0
90%	11.0	45.0	25.8	0.0	25.8	0.0
85%	40.6	55.5	25.8	0.0	25.8	0.0
80%	78.2	65.4	25.8	0.0	25.8	0.0
75%	125.2	79.8	41.7	0.0	41.7	0.0
70%	175.1	87.6	41.7	0.0	41.7	0.0
60%	313.6	106.7	91.2	0.0	91.2	0.0
50%	520.6	180.8	101.2	0.0	101.2	0.0
40%	823.5	307.5	158.7	0.0	158.7	0.0
30%	1,291	525.3	190.4	0.0	190.4	0.0
25%	1,644	679.8	190.4	0.0	190.4	0.0
20%	2,163	935.4	190.4	0.0	190.4	0.0
15%	2,900	1,339	388.8	0.0	388.8	0.0
10%	4,094	2,093	388.8	9.0	388.8	9.0
5%	6,443	3,847	388.8	45.5	684.3	45.5
2%	10,063	6,930	388.8	67.8	1,626	67.8
1%	12,508	9,403	388.8	98.5	1,626	98.5
0.50%	16,534	11,604	388.8	101.2	1,652	101.2
Maximum	52,115	50,281	388.8	101.2	1,652	101.2

The regulated flow decreases slightly compared to the naturalized flow at the control point NEROE, as shown in Table 7.29. The shortage of the environmental flow target is 7.0 acre-feet/day in average, and 132.9 acre-feet/day in maximum at the control point.

Table 7.29 Flow Frequency Metrics (acre-feet/day) for Control Point NEROE

	Naturalized Flow	Regulated Flow	<u>Subsidence and Base</u>		<u>Total Including Pulse</u>	
			IF Target	IF Shortage	IF Target	IF Shortage
Mean	4,809	4,021	453.3	7.0	641.5	7.0
Stan Dev	7,262	6,677	455.4	20.7	998.6	20.7
Minimum	0.0	0.0	41.7	0.0	41.7	0.0
99.50%	0.0	0.0	41.7	0.0	41.7	0.0
99%	0.0	0.0	41.7	0.0	41.7	0.0
98%	0.0	0.0	41.7	0.0	41.7	0.0
95%	37.9	0.2	41.7	0.0	41.7	0.0
90%	122.7	0.9	41.7	0.0	41.7	0.0
85%	231.8	58.0	41.7	0.0	41.7	0.0
80%	363.7	133.0	57.5	0.0	57.5	0.0
75%	519.0	235.0	132.9	0.0	132.9	0.0
70%	695.0	362.0	132.9	0.0	132.9	0.0
60%	1,213	738.9	132.9	0.0	132.9	0.0
50%	2,039	1,370	178.5	0.0	178.5	0.0
40%	3,189	2,347	386.8	0.0	386.8	0.0
30%	4,877	3,841	833.1	0.0	833.1	0.0
25%	6,124	4,958	833.1	0.0	833.1	0.0
20%	7,693	6,422	1,196	0.0	1,196	0.0
15%	9,916	8,399	1,196	0.0	1,196	0.0
10%	12,767	11,337	1,196	40.8	1,196	40.8
5%	18,859	16,861	1,196	41.7	2,318	41.7
2%	27,297	24,583	1,196	57.0	3,969	57.0
1%	33,726	30,890	1,196	132.6	6,109	132.6
0.50%	42,461	39,342	1,196	132.8	6,109	132.8
Maximum	100,881	98,545	1,196	132.9	6,109	132.9

The regulated flow has slightly different statistic parameters and flow frequency metrics from the naturalized flow at the control point ANALE due to water uses and flow controls by the Tyler East and Tyler dams, as listed in Table 7.30. The regulated flow can meet the environmental flow target engaged by the WRAP model for 90% of the period-of-analysis at the control point.

Table 7.30 Flow Frequency Metrics (acre-feet/day) for Control Point ANALE

	Naturalized Flow	Regulated Flow	<u>Subsidence and Base</u>		<u>Total Including Pulse</u>	
			IF Target	IF Shortage	IF Target	IF Shortage
Mean	1,874	1,420	170.8	3.1	276.6	3.1
Stan Dev	3,232	2,739	179.8	11.0	492.2	11.0
Minimum	0.0	0.0	21.8	0.0	21.8	0.0
99.50%	0.0	0.0	21.8	0.0	21.8	0.0
99%	0.0	0.0	21.8	0.0	21.8	0.0
98%	1.8	0.0	21.8	0.0	21.8	0.0
95%	15.3	0.0	21.8	0.0	21.8	0.0
90%	41.6	18.8	21.8	0.0	21.8	0.0
85%	72.2	40.3	31.7	0.0	31.7	0.0
80%	111.1	61.0	35.7	0.0	35.7	0.0
75%	159.4	86.5	35.7	0.0	35.7	0.0
70%	216.2	119.4	79.3	0.0	79.3	0.0
60%	363.1	208.7	103.1	0.0	103.1	0.0
50%	608.4	363.2	103.1	0.0	103.1	0.0
40%	1,010	630.9	109.1	0.0	109.1	0.0
30%	1,667	1,102	178.5	0.0	178.5	0.0
25%	2,153	1,467	178.5	0.0	178.5	0.0
20%	2,803	1,987	549.4	0.0	549.4	0.0
15%	3,733	2,787	549.4	0.0	549.4	0.0
10%	5,213	4,004	549.4	10.7	549.4	10.7
5%	8,097	6,541	549.4	21.8	1,166	21.8
2%	12,055	10,340	549.4	35.2	2,182	35.2
1%	15,226	13,092	549.4	35.7	3,213	35.7
0.50%	18,479	16,459	549.4	90.1	3,213	90.1
Maximum	55,090	54,732	549.4	109.1	3,213	109.1

Table 7.31 presents the flow frequency metrics at control point NEEVE. There are considerably different statistic parameters and flow frequencies between the naturalized and regulated flows at the control point. The minimum shortage of the environmental flow target occurs at 25% of exceedance frequency.

Table 7.31 Flow Frequency Metrics (acre-feet/day) for Control Point NEEVE

	Naturalized Flow	Regulated Flow	<u>Subsidence and Base</u>		<u>Total Including Pulse</u>	
			IF Target	IF Shortage	IF Target	IF Shortage
Mean	12,349	8,511	1,551	101.7	1,675	101.7
Stan Dev	16,977	14,378	1,409	290.3	1,622	290.3
Minimum	0.0	0.0	452.2	0.0	452.2	0.0
99.50%	31.3	0.0	452.2	0.0	452.2	0.0
99%	64.3	0.0	452.2	0.0	452.2	0.0
98%	107.7	0.0	452.2	0.0	452.2	0.0
95%	185.5	0.5	452.2	0.0	452.2	0.0
90%	392.9	0.9	452.2	0.0	452.2	0.0
85%	675.8	119.0	452.2	0.0	452.2	0.0
80%	1,121	246.7	452.2	0.0	452.2	0.0
75%	1,599	414.2	452.2	0.0	452.2	0.0
70%	2,148	605.2	452.2	0.0	452.2	0.0
60%	3,671	1,102	527.6	0.0	527.6	0.0
50%	5,711	1,792	1,016	0.0	1,016	0.0
40%	8,873	3,440	1,150	0.0	1,150	0.0
30%	13,561	7,406	3,114	0.0	3,114	0.0
25%	16,417	10,121	3,578	52.2	3,578	52.2
20%	20,341	13,614	3,578	218.3	3,578	218.3
15%	25,663	18,570	3,818	339.6	3,818	339.6
10%	33,442	27,967	3,818	451.3	3,818	451.3
5%	45,905	40,591	3,818	452.2	3,818	452.2
2%	63,594	58,140	3,818	527.1	5,392	527.1
1%	77,558	65,429	3,818	527.3	7,597	527.3
0.50%	91,127	70,820	3,818	1,015.5	7,597	1,015.5
Maximum	274,941	196,728	3,818	7,596.7	7,597	7,596.7

Both naturalized and regulated flows have nearly identical flow frequency metrics in the control point VIKOE, as presented in Table 7.32. This means that there are no water uses and flow controllers upstream of the control point. The exceedance frequency that the environmental flow shortage occurs is below 15%.

Table 7.32 Flow Frequency Metrics (acre-feet/day) for Control Point VIKOE

	Naturalized Flow	Regulated Flow	<u>Subsidence and Base</u>		<u>Total Including Pulse</u>	
			IF Target	IF Shortage	IF Target	IF Shortage
Mean	1,749	1,748	219.3	13.7	347.8	13.7
Stan Dev	3,396	3,396	147.3	30.7	576.2	30.7
Minimum	0.0	0.0	81.3	0.0	81.3	0.0
99.50%	0.0	0.0	81.3	0.0	81.3	0.0
99%	0.0	0.0	81.3	0.0	81.3	0.0
98%	0.0	0.0	81.3	0.0	81.3	0.0
95%	2.8	2.7	81.3	0.0	81.3	0.0
90%	23.2	23.0	81.3	0.0	81.3	0.0
85%	50.4	49.9	81.3	0.0	81.3	0.0
80%	81.2	80.6	97.2	0.0	97.2	0.0
75%	124.9	124.3	97.2	0.0	97.2	0.0
70%	174.5	173.4	152.7	0.0	152.7	0.0
60%	313.6	312.8	152.7	0.0	152.7	0.0
50%	537.5	536.2	194.4	0.0	194.4	0.0
40%	864.2	863.0	194.4	0.0	194.4	0.0
30%	1,397	1,395	232.1	0.0	232.1	0.0
25%	1,824	1,822	232.1	0.0	232.1	0.0
20%	2,447	2,445	523.6	13.5	523.6	13.5
15%	3,304	3,300	523.6	44.6	523.6	44.6
10%	4,798	4,798	523.6	70.4	523.6	70.4
5%	7,743	7,743	523.6	81.3	1,412	81.3
2%	11,529	11,528	523.6	97.2	2,737	97.2
1%	15,968	15,966	523.6	135.3	3,987	135.3
0.50%	21,698	21,673	523.6	153.5	3,987	153.5
Maximum	64,692	64,700	523.6	164.6	3,987	164.6

7.3.3 GSA WAM

The WRAP model for the GSA WAM computes the regulated flows based on naturalized datasets and engages the environmental flow target based on the regulated flow and hydrologic index (HIS file) at 15 control points. Flow frequency metrics are developed as presented in Tables 7.33 to 7.47. The simulation covers the period-of-analysis (1934-2013), and a full authorized use scenario is adopted.

Table 7.33 Flow Frequency Metrics (acre-feet/day) for Control Point CP01E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	412.4	382.1	136.2	6.9
Stan Dev	1,585	1,568	164.7	15.3
Minimum	0.0	0.0	4.0	0.0
99.50%	0.0	0.0	4.0	0.0
99%	0.0	0.0	4.0	0.0
98%	0.3	0.1	4.0	0.0
95%	1.5	0.4	4.4	0.0
90%	5.7	2.8	23.1	0.0
85%	12.2	8.1	35.7	0.0
80%	23.0	16.9	40.4	0.0
75%	36.3	29.7	49.6	0.0
70%	54.2	46.3	54.7	0.0
60%	101.6	89.7	63.3	0.0
50%	164.3	141.6	96.0	0.0
40%	246.9	210.0	148.8	0.0
30%	367.5	319.9	198.3	0.0
25%	446.7	392.7	198.3	0.1
20%	545.6	491.9	218.2	3.9
15%	687.5	626.7	218.2	22.5
10%	914.0	850.0	218.2	34.7
5%	1,381	1,313	277.7	46.3
2%	2,255	2,180	694.2	58.1
1%	3,335	3,226	793.4	60.9
0.50%	5,186	5,098	991.7	61.3
Maximum	182,609	181,764	2,360	61.5

The regulated flows have almost similar statistical parameters but slightly different flow frequency metrics at the 10 control points CP01E, CP02E, CP08E, CP10E, CP11E, C38461E, CP14E, and CP15 in the Guadalupe River Basin. The major water rights in the Guadalupe River Basin are hydroelectric generations. The flows, utilized for generating electricity, must fully return to the river without losses. For these reasons, the amounts of both flows are almost similar, but flow timing and regime are changed slightly.

Table 7.34 Flow Frequency Metrics (acre-feet/day) for Control Point CP02E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	709.0	681.4	218.1	4.2
Stan Dev	1,895	1,878	300.4	10.4
Minimum	0.0	0.0	35.7	0.0
99.50%	0.0	0.0	35.7	0.0
99%	0.0	0.0	35.7	0.0
98%	0.0	0.0	35.7	0.0
95%	2.2	0.9	35.7	0.0
90%	14.0	11.1	35.7	0.0
85%	33.9	29.0	35.7	0.0
80%	57.9	52.0	43.9	0.0
75%	85.0	78.4	57.1	0.0
70%	118.8	109.0	72.5	0.0
60%	197.3	183.0	109.6	0.0
50%	298.1	276.1	168.6	0.0
40%	443.2	411.2	218.2	0.0
30%	654.7	614.4	297.5	0.0
25%	789.7	744.3	317.4	0.0
20%	968.5	914.8	317.4	0.0
15%	1,208	1,156	317.4	6.7
10%	1,592	1,536	317.4	24.6
5%	2,444	2,382	416.5	34.8
2%	4,012	3,930	1,072	35.7
1%	5,975	5,846	1,726	35.7
0.50%	8,721	8,681	1,726	35.7
Maximum	164,943	164,498	4,582	35.7

Table 7.35 Flow Frequency Metrics (acre-feet/day) for Control Point CP08E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	280.8	278.1	86.5	1.7
Stan Dev	806.7	802.8	124.6	4.8
Minimum	0.0	0.0	15.9	0.0
99.50%	0.0	0.0	15.9	0.0
99%	0.0	0.0	15.9	0.0
98%	0.0	0.0	15.9	0.0
95%	5.8	5.4	18.2	0.0
90%	12.6	12.3	19.8	0.0
85%	19.3	18.8	21.1	0.0
80%	26.6	25.9	25.8	0.0
75%	35.2	34.3	27.2	0.0
70%	44.9	44.0	32.0	0.0
60%	67.7	66.2	43.3	0.0
50%	99.9	98.4	59.5	0.0
40%	148.3	145.4	103.1	0.0
30%	225.8	223.1	111.1	0.0
25%	284.4	280.6	126.9	0.0
20%	366.1	361.7	126.9	0.0
15%	474.6	470.0	126.9	0.9
10%	662.6	656.1	126.9	7.5
5%	1,043	1,036	126.9	14.8
2%	1,736	1,733	376.9	19.7
1%	2,611	2,608	714.0	21.9
0.50%	3,510	3,468	753.7	25.8
Maximum	73,319	73,029	1,904	25.8

The variances of both naturalized and regulated flows are bigger than the means at all the control points in the Guadalupe River Basin. That means that the flow variability in the Guadalupe River Basin is much higher than other basins like the Sabine and Neches River Basins in this research. The exceedance frequencies of the environmental flow target shortage at the 9 control points are also relatively higher than two other basins.

Table 7.36 Flow Frequency Metrics (acre-feet/day) for Control Point CP10E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	758.0	740.2	338.3	36.6
Stan Dev	1,682	1,665	366.0	59.8
Minimum	0.0	0.0	144.8	0.0
99.50%	0.0	0.0	144.8	0.0
99%	0.0	0.0	144.8	0.0
98%	0.0	0.0	144.8	0.0
95%	1.8	0.3	144.8	0.0
90%	14.9	10.6	144.8	0.0
85%	34.9	30.9	160.7	0.0
80%	59.9	57.2	167.3	0.0
75%	88.9	91.7	176.5	0.0
70%	123.7	134.4	176.5	0.0
60%	212.4	213.5	188.7	0.0
50%	330.2	315.8	240.3	0.0
40%	482.3	461.5	396.7	0.0
30%	698.6	669.8	416.5	28.1
25%	844.2	811.4	416.5	69.4
20%	1,039	1,003	436.4	104.2
15%	1,314	1,273	436.4	130.8
10%	1,740	1,700	436.4	144.8
5%	2,654	2,613	436.4	163.6
2%	4,533	4,480	1,083	176.5
1%	6,705	6,665	2,386	176.5
0.50%	9,937	9,863	3,317	176.5
Maximum	758.0	101,154	3,965	176.5

Table 7.37 Flow Frequency Metrics (acre-feet/day) for Control Point CP11E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	220.4	219.1	33.0	1.0
Stan Dev	1,140	1,136	210.6	1.6
Minimum	0.0	0.0	2.0	0.0
99.50%	0.0	0.0	2.0	0.0
99%	0.0	0.0	2.0	0.0
98%	0.0	0.0	2.0	0.0
95%	0.0	0.0	2.0	0.0
90%	0.0	0.0	2.0	0.0
85%	0.1	0.1	2.0	0.0
80%	0.4	0.4	2.0	0.0
75%	0.9	0.8	3.0	0.0
70%	1.5	1.4	4.0	0.0
60%	3.7	3.6	5.3	0.0
50%	8.2	8.0	6.0	0.0
40%	17.7	17.4	9.9	0.0
30%	41.0	40.5	15.9	1.5
25%	63.9	63.0	15.9	1.9
20%	106.8	105.6	19.8	2.0
15%	193.3	191.5	19.8	2.0
10%	388.8	384.5	23.8	3.6
5%	978.2	973.4	23.8	4.5
2%	2,420	2,395	297.5	5.8
1%	4,037	4,020	694.2	6.0
0.50%	5,846	5,845	1,428	6.0
Maximum	72,919	72,920	4,165	6.0

Table 7.38 Flow Frequency Metrics (acre-feet/day) for Control Point C38461E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	2,576	2,256	1,194	35.7
Stan Dev	4,120	3,491	1,037	93.0
Minimum	0.0	0.0	357.0	0.0
99.50%	0.0	0.0	357.0	0.0
99%	0.0	2.4	357.0	0.0
98%	9.8	37.2	357.0	0.0
95%	70.2	120.7	416.5	0.0
90%	178.4	230.3	416.5	0.0
85%	305.8	348.2	416.5	0.0
80%	436.3	486.8	444.7	0.0
75%	572.8	638.3	520.7	0.0
70%	719.7	790.9	597.7	0.0
60%	1,056	1,137	769.5	0.0
50%	1,454	1,499	976.9	0.0
40%	1,934	1,671	1,480	0.0
30%	2,569	1,724	1,480	0.0
25%	2,992	1,757	1,569	0.0
20%	3,537	2,311	1,569	0.0
15%	4,324	2,877	1,579	50.9
10%	5,658	5,303	1,579	177.8
5%	8,646	10,435	1,884	289.4
2%	14,079	12,339	4,384	364.6
1%	19,289	14,186	8,212	412.3
0.50%	24,984	18,319	8,239	416.5
Maximum	111,820	96,140	8,239	416.5

Table 7.39 Flow Frequency Metrics (acre-feet/day) for Control Point CP13E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	260.8	259.2	30.6	0.6
Stan Dev	1,329	1,323	131.2	1.5
Minimum	0.0	0.0	2.0	0.0
99.50%	0.0	0.0	2.0	0.0
99%	0.0	0.0	2.0	0.0
98%	0.0	0.0	2.0	0.0
95%	0.2	0.2	2.0	0.0
90%	0.7	0.7	2.0	0.0
85%	1.5	1.5	2.8	0.0
80%	2.5	2.5	4.0	0.0
75%	3.8	3.7	4.3	0.0
70%	5.4	5.3	6.4	0.0
60%	10.6	10.5	7.9	0.0
50%	19.7	19.6	7.9	0.0
40%	36.4	36.1	17.9	0.0
30%	68.7	68.2	17.9	0.0
25%	98.4	97.5	17.9	0.0
20%	144.2	142.8	17.9	0.9
15%	228.7	226.4	23.8	1.7
10%	434.5	431.7	23.8	2.0
5%	1,067	1,057	23.8	4.0
2%	2,711	2,690	297.5	6.7
1%	4,689	4,680	740.1	7.5
0.50%	7,392	7,316	1,131	7.8
Maximum	95,545	95,114	1,527	7.9

Table 7.40 Flow Frequency Metrics (acre-feet/day) for Control Point CP14E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	3,390	3,105	1,378	13.6
Stan Dev	6,179	5,468	1,470	46.1
Minimum	0.0	0.0	170.6	0.0
99.50%	1.1	0.0	170.6	0.0
99%	4.1	3.9	170.6	0.0
98%	13.3	34.2	206.2	0.0
95%	72.6	110.9	240.1	0.0
90%	171.5	216.5	257.9	0.0
85%	292.7	339.3	285.8	0.0
80%	445.2	478.7	354.0	0.0
75%	611.3	626.7	429.0	0.0
70%	807.6	791.4	513.2	0.0
60%	1,217	1,472	849.2	0.0
50%	1,692	1,947	1,587	0.0
40%	2,236	2,219	1,726	0.0
30%	3,045	2,317	1,865	0.0
25%	3,628	2,581	1,865	0.0
20%	4,424	3,167	1,865	0.0
15%	5,572	4,693	1,944	0.0
10%	7,691	7,366	1,944	19.9
5%	12,309	11,837	2,083	130.7
2%	20,480	18,491	5,172	202.7
1%	28,272	24,955	8,232	241.6
0.50%	39,400	33,744	10,314	257.9
Maximum	137,129	130,576	17,593	257.9

Table 7.41 Flow Frequency Metrics (acre-feet/day) for Control Point CP15EE

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	3,508	3,187	1,346	20.0
Stan Dev	6,110	5,352	982.0	59.6
Minimum	0.0	0.0	218.2	0.0
99.50%	0.0	0.0	218.2	0.0
99%	6.6	3.3	218.2	0.0
98%	29.1	31.2	219.9	0.0
95%	96.3	100.8	257.9	0.0
90%	213.8	197.6	297.5	0.0
85%	355.3	307.0	317.4	0.0
80%	519.8	433.5	352.8	0.0
75%	690.8	570.9	423.1	0.0
70%	894.0	743.0	509.8	0.0
60%	1,321	1,648	969.0	0.0
50%	1,789	2,235	1,577	0.0
40%	2,352	2,623	1,716	0.0
30%	3,201	2,737	1,874	0.0
25%	3,794	2,883	1,874	0.0
20%	4,580	3,297	1,874	0.0
15%	5,826	4,239	1,934	0.0
10%	7,964	7,074	1,934	79.8
5%	12,482	11,551	2,063	180.3
2%	20,548	18,298	3,818	247.3
1%	28,546	24,800	6,427	288.0
0.50%	40,964	35,504	6,427	297.5
Maximum	106,650	89,492	6,427	317.4

The regulated flows have almost similar statistical parameters and flow frequency metrics to the naturalized flows at the 2 control points P38241E and CP35E in the San Antonio River Basin. This is because the control point P38241E is located upstream of Medina Lake, and CP35E is located at a tributary.

Table 7.42 Flow Frequency Metrics (acre-feet/day) for Control Point P38241E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	210.7	210.4	62.9	9.5
Stan Dev	839.5	837.3	69.3	19.7
Minimum	0.0	0.0	0.0	0.0
99.50%	0.2	0.2	2.0	0.0
99%	0.6	0.6	2.2	0.0
98%	1.4	1.4	3.7	0.0
95%	3.7	3.7	7.3	0.0
90%	9.1	9.1	11.9	0.0
85%	15.5	15.5	19.8	0.0
80%	22.1	22.1	31.7	0.0
75%	28.7	28.7	31.7	0.0
70%	35.4	35.4	31.7	0.0
60%	51.4	51.4	43.6	0.0
50%	73.0	72.9	63.5	0.0
40%	104.2	104.1	63.5	0.0
30%	150.1	150.0	65.5	1.8
25%	182.6	182.6	81.3	8.0
20%	224.6	224.4	95.2	15.8
15%	286.9	286.4	97.2	26.2
10%	400.7	400.3	105.1	38.1
5%	640.2	639.5	107.1	58.5
2%	1,200	1,199	218.2	75.7
1%	2,192	2,188	243.5	85.0
0.50%	3,934	3,934	603.2	91.1
Maximum	47,589	47,109	952.1	107.1

The regulated flows have considerably different statistical parameters and flow frequency metrics from the naturalized flows at the control points, CP28E, CP29E, CP32E, and CP37E due to water uses and flow controls by Medina Lake in the San Antonio River.

Table 7.43 Flow Frequency Metrics (acre-feet/day) for Control Point CP28E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	505.7	308.0	125.9	17.6
Stan Dev	1,379	1,030	130.1	31.2
Minimum	1.2	0.0	0.0	0.0
99.50%	4.1	0.0	15.9	0.0
99%	7.0	2.1	15.9	0.0
98%	10.5	7.3	20.7	0.0
95%	18.1	16.5	25.8	0.0
90%	35.2	28.9	32.8	0.0
85%	55.1	39.5	39.7	0.0
80%	74.2	49.9	65.5	0.0
75%	94.4	59.4	105.1	0.0
70%	114.9	72.3	105.1	0.0
60%	160.2	96.9	113.1	0.0
50%	207.6	122.8	119.0	0.0
40%	271.7	154.2	123.0	0.0
30%	374.5	195.9	123.0	12.4
25%	445.2	230.7	140.8	23.3
20%	549.6	283.7	142.8	37.1
15%	708.8	364.1	146.8	53.9
10%	986.0	533.4	152.7	70.2
5%	1,656	975.2	234.9	90.4
2%	3,357	2,100	494.3	110.0
1%	5,578	3,714	753.7	120.9
0.50%	8,408	5,932	892.6	131.5
Maximum	45,662	44,078	1,984	152.7

The variances of both naturalized and regulated flows are also greater than the means at all the 6 control points in the San Antonio River Basin. That means that the flow variability in the San Antonio River Basin is higher than 2 other basins like the Guadalupe River Basin. The exceedance frequencies of the environmental flow target shortage at the 6 control points are also relatively higher than two other basins as well.

Table 7.44 Flow Frequency Metrics (acre-feet/day) for Control Point CP29E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	770.5	487.9	473.5	230.3
Stan Dev	2,003	1,721	526.5	204.0
Minimum	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	119.0	0.0
99%	0.0	0.0	119.0	0.0
98%	0.0	0.0	119.0	0.0
95%	1.1	1.9	119.0	0.0
90%	27.6	8.8	119.0	0.0
85%	60.9	17.7	119.0	0.0
80%	92.4	26.4	160.4	0.0
75%	122.9	33.3	228.1	22.7
70%	154.1	46.6	353.1	74.6
60%	232.1	90.6	442.3	116.8
50%	325.9	144.4	470.1	192.2
40%	445.6	214.6	470.1	300.2
30%	611.9	303.6	519.7	350.8
25%	724.6	366.8	650.6	389.1
20%	881.6	459.0	650.6	426.4
15%	1,115	608.6	676.4	461.3
10%	1,505	891.7	722.0	514.8
5%	2,542	1,690	727.9	616.3
2%	5,212	3,819	727.9	658.8
1%	8,587	6,820	2,003	695.9
0.50%	13,114	10,502	3,666	709.6
Maximum	57,202	56,177	7,934	726.1

Table 7.45 Flow Frequency Metrics (acre-feet/day) for Control Point CP32E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	905.4	618.4	546.7	218.0
Stan Dev	1,822	1,489	554.8	240.4
Minimum	0.0	2.2	0.0	0.0
99.50%	9.0	2.3	119.0	0.0
99%	14.4	6.0	119.0	0.0
98%	25.3	19.7	119.0	0.0
95%	49.4	35.9	119.0	0.0
90%	85.0	54.8	119.0	0.0
85%	121.8	73.7	135.2	0.0
80%	157.3	95.4	224.1	0.0
75%	191.8	118.2	301.5	0.0
70%	230.6	141.9	394.7	0.0
60%	321.8	192.6	487.9	55.8
50%	432.5	249.3	523.6	116.7
40%	569.3	328.7	523.6	246.5
30%	763.2	445.1	579.2	330.8
25%	900.2	530.0	841.0	369.2
20%	1,091	657.4	841.0	425.8
15%	1,384	877.4	852.9	487.8
10%	1,868	1,263	926.3	599.3
5%	3,104	2,244	950.1	719.2
2%	5,771	4,458	950.1	797.8
1%	8,767	6,811	1,686	843.7
0.50%	12,329	9,593	3,015	868.1
Maximum	47,547	45,832	12,893	915.1

Table 7.46 Flow Frequency Metrics (acre-feet/day) for Control Point CP35E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	254.4	250.2	70.4	12.6
Stan Dev	861.7	860.2	212.0	19.4
Minimum	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	15.9	0.0
99%	0.0	0.0	15.9	0.0
98%	2.1	0.8	15.9	0.0
95%	5.3	3.9	15.9	0.0
90%	9.6	8.3	17.3	0.0
85%	13.4	12.2	22.2	0.0
80%	17.5	16.1	31.7	0.0
75%	22.4	20.7	39.7	0.0
70%	27.5	25.4	39.7	0.0
60%	40.1	37.7	47.6	0.0
50%	57.0	53.1	55.5	0.0
40%	81.8	76.9	55.5	5.1
30%	124.0	118.2	55.5	15.1
25%	159.9	153.4	73.4	21.7
20%	218.0	211.5	77.4	28.8
15%	309.7	302.7	77.4	35.2
10%	500.4	492.0	79.3	42.4
5%	1,024	1,016	87.3	55.5
2%	2,228	2,214	87.3	69.0
1%	3,714	3,701	621.8	74.6
0.50%	5,407	5,380	1,984	78.2
Maximum	33,996	33,990	4,959	86.0

Table 7.47 Flow Frequency Metrics (acre-feet/day) for Control Point CP37E

	Naturalized Flow	Regulated Flow	IF Target	IF Shortage
Mean	1,433	1,159	671.7	183.0
Stan Dev	3,102	2,848	778.0	250.4
Minimum	4.6	0.0	0.0	0.0
99.50%	17.1	11.6	119.0	0.0
99%	32.2	25.5	119.0	0.0
98%	44.3	31.3	119.0	0.0
95%	77.1	57.7	119.0	0.0
90%	128.8	95.7	143.8	0.0
85%	176.4	131.0	217.5	0.0
80%	225.9	169.9	331.2	0.0
75%	283.3	208.1	396.7	0.0
70%	347.6	247.6	470.1	0.0
60%	482.5	342.5	555.4	0.0
50%	633.2	445.9	620.8	27.0
40%	829.0	584.9	620.8	127.5
30%	1,128	797.8	652.6	274.9
25%	1,356	980.2	930.2	336.5
20%	1,688	1,251	954.0	394.0
15%	2,203	1,674	954.0	459.2
10%	3,043	2,486	995.7	556.7
5%	5,021	4,258	1,158	724.9
2%	9,326	8,161	1,158	879.4
1%	14,588	12,635	3,114	954.7
0.50%	20,076	18,196	4,602	1,012
Maximum	102,858	101,526	15,868	1,130

CHAPTER VIII

FURTHER STATISTICAL ANALYSES OF STREAM FLOWS

Sequences of daily observed flows at USGS gauging stations and naturalized regulated, and unappropriated flows from the daily WAMs for the Sabine, Neches, and Guadalupe and San Antonio (GSA) River Basins are investigated in Chapters 7, 8, and 9 from the perspective of the environmental flow standards recently established pursuant to the Senate Bill (SB3) process. The following types of comparative analyses are performed for daily flows at gauge sites in the case study river basins for which SB3 environmental flow standards have been adopted:

1. analyses of statistical metrics of observed gauged flows during historical periods prior to significant water resources development versus during more recent periods reflecting development (Chapter 8)
2. analyses of statistical metrics for naturalized versus regulated flows from the WRAP/WAM simulations (Chapters 7 and 8)
3. analyses of unappropriated flows from the WRAM/WAM simulations with versus without the environmental flow standards (Chapter 9)

Long-term changes in river flow characteristics are important in considering environmental flows. The first two sets of analyses listed above deal with quantifying changes in flow characteristics. Two alternative approaches are adopted in Chapter 8 for quantifying long-term changes in river flow characteristics: (a) the Dundee Hydrological Regime Alteration Method (DHRAM) and (b) comparison of WAM naturalized and regulated flows.

The third set of analyses explores the effects of the environmental flow standards on flows available for water right permit applications. The SB3 process specifies that environmental flow standards be assigned priorities that are junior to all existing water right permits. The WAM simulation analyses presented in Chapter 9 compare unappropriated flows and water supply reliabilities with the SB3 environmental flow standards assigned priorities alternatively junior versus senior to all existing water rights.

8.1 Hydrological Regime Alteration on River Flow

The hydrological differences of daily river flow sequences, divided into impacted and un-impacted periods, are quantified using the DHRAM applied to observed daily flows at selected USGS gauging stations and WAM naturalized and regulated flows at these sites. As noted in Chapter 1, The Dundee Hydrological Regime Alteration Method (DHRAM) was developed by Black *et al.* (2005) based on the Indicator of Hydrologic Alteration (IHA) methodology developed by Richter *et al.* (1996).

The selected USGS gauging stations have SB3 environmental flow standards and daily recorded data for a long period-of-record. These USGS recorded flows are divided into the two periods before and after a major dam construction and accompanying reservoir storage. Dam construction is considered most significant point-specific human influence for the DHRAM analysis. In the simulations of daily WRAP models, naturalized flows are literally un-impacted flows, and regulated flows are theoretically impacted flows. The DHRAM analyses can be performed at all control points under these assumptions. However, the analyses based on both the flows from the simulation results are only performed at the same control points where the analyses are available based on the USGS recorded flows for the comparison of both results of the DHRAM analyses.

8.1.1 Assessment of USGS Recorded Flow before and after Human Influences

Sabine River Basin

There are five control points that have environmental flow standards. Four of these control points have daily recorded flows for relatively long term period. Control point 29500 (USGS gauge number 08029500) does not and is therefore excluded for the DHRAM analysis.

USGS gauging station 08019500(WAM control point BSBS), located on the Big Sandy Creek near Big Sandy, has daily recorded flow data for the period 1939 to present. Winnsboro Lake, located upstream of the gauging station, was initially impounded in 1962. The period-of-record is divided into two periods based on 1962. The un-impacted period is from 1940 to 1961, and impacted period is from 1962 to 2013. Table 8.1 presents the result of the analysis. Monthly average flows for

each month, minimum and maximum daily flows, and flow timings of both the periods are almost similar. The analysis yields a total of 2 impact points and a corresponding DHRAM class of 2.

The USGS gauging station 08020000 (control point SRGW), located on the Sabine River near Gladewater, has daily recorded flow data for the period 1932 to present. Ten dams are located upstream of the gauging station, but two large dams, Lake Tawakoni and Lake Fork, most significantly influence the flow regime at the gauging station. Lake Tawakoni and Lake Fork were initially impounded in 1960 and 1979, respectively. The period-of-record is divided into two periods on the basis of 1960. Un-impacted period is from 1940 to 1959, and impacted period is from 1960 to 2013. Table 8.2 presents the result of the analysis. Monthly average flows for each month decrease during the impacted period. The alterations of minimum and maximum daily flows are ignorable, but flow timings of both the periods are totally different. The analysis yields a total of 5 impact points and a corresponding DHRAM class of 3.

USGS gauging station 08022040 (SRBE), located on the Sabine River near Beckville, and downstream of the control point SRGW, has the daily recorded flow data for the period 1938 to present. The period-of-record is also divided into two periods on the basis of 1960 like the control point SRGW. Table 8.3 shows the result of the analysis. Monthly average for each month and minimum and maximum daily flows for certain periods from 1 day to 90days relatively decrease during the impacted period than the un-impacted period. The average date of daily maximum for the impacted period is put forward about 80 days than the un-impacted period. The analysis yields a total of 3 impact points and a corresponding DHRAM class of 2.

USGS gauging station 08030500 (control point SRRL), located on the Sabine River near Ruliff and downstream of Toledo Bend Lake, has daily recorded flow data for the period 1924 to present. The lake has been operated since 1966. Thus, the period-of-record is also divided into two periods on the basis of 1966 like the control point SRGW. Table 8.4 provides the result of the analysis. Monthly averages for each month and minimum and maximum daily flows for certain periods from 1 day to 90days relatively decrease for the impacted period than the un-impacted period. The average date of daily maximum for the impacted period is put forward about 70 days than the un-impacted period. The analysis yields a total of 6 impact points and a corresponding HDRAM class of 3.

Table 8.1 DHRAM Results at USGS 08029500 (Control Point BSBS)

Control Point:	BSBS(USGS)		Period: 1940-2013			
IHA statistics group	1940-1961		1962-2013		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	273.4	0.7568	219.0	0.844	19.9%	11.5%
February	297.6	0.6789	281.7	0.726	5.3%	6.9%
March	330.2	1.037	333.9	0.795	1.1%	23.3%
April	367.7	0.8362	261.8	0.841	28.8%	0.6%
May	371.7	0.8503	247.9	0.960	33.3%	12.9%
June	211.6	1.038	139.9	0.970	33.9%	6.5%
July	79.4	0.8254	85.1	1.413	7.2%	71.2%
August	40.8	0.6684	31.3	1.022	23.3%	52.9%
September	45.9	0.6061	52.6	1.504	14.7%	148.1%
October	80.1	1.369	76.8	2.062	4.2%	50.6%
November	133.9	1.238	130.3	1.236	2.7%	0.2%
December	220.9	0.7337	226.3	0.997	2.4%	35.9%
Average Score					14.7%	35.1%
					0	1
Parameter Group #2						
1-day minimum	18.16	0.4232	11.98	0.5596	34.0%	32.2%
3-day minimum	18.45	0.4329	12.33	0.5499	33.2%	27.0%
7-day minimum	19.45	0.4391	12.99	0.5448	33.2%	24.1%
30-day minimum	23.85	0.4624	16.83	0.571	29.4%	23.5%
90-day minimum	42.52	0.6105	30.87	0.8227	27.4%	34.8%
1-day maximum	3981	1.01	2449	0.7052	38.5%	30.2%
3-day maximum	3021	0.8916	1957	0.6424	35.2%	27.9%
7-day maximum	1936	0.8138	1348	0.5455	30.4%	33.0%
30-day maximum	768.8	0.6372	635.4	0.5349	17.4%	16.1%
90-day maximum	436.5	0.5317	383.1	0.5146	12.2%	3.2%
Average Score					29.1%	25.2%
					0	0
Parameter Group #3						
Date of minimum	231.7	0.06215	240.3	0.08241	3.7%	32.6%
Date of maximum	121.5	0.2271	140.7	0.2987	15.8%	31.5%
Average Score					9.8%	32.1%
					1	0
Parameter Group #4						
Low pulse count	4.636	0.5093	5.519	0.3737	19.0%	26.6%
Low pulse duration	23.2	0.9133	21.49	0.5079	7.4%	44.4%
High pulse count	4.318	0.7874	4.019	0.7577	6.9%	3.8%
High pulse duration	4.652	0.4071	4.73	0.6026	1.7%	48.0%
Average Score					8.8%	30.7%
					0	1
Parameter Group #5						
Rise rate	88.6	0.7507	64.83	0.6987	26.8%	6.9%
Fall rate	-48.05	-0.7083	-35.21	-0.6589	26.7%	7.0%
Number of reversals	76.5	0.09232	86.23	0.117	12.7%	26.7%
Average Score					22.1%	13.5%
					0	0
Total Point Classification					3	2
note:					Low risk of impact	

Table 8.2 DHRAM Results at USGS 08020000 (Control Point SRGW)

Control Point:	SRBE(USGS)		Period: 1940-2013			
IHA statistics group	1940-1959		1960-2013		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	2920.0	0.8549	3385.0	0.972	15.9%	13.7%
February	3599.0	0.9182	3919.0	0.752	8.9%	18.1%
March	3836.0	0.9104	4503.0	0.858	17.4%	5.8%
April	4843.0	1.264	3646.0	0.860	24.7%	31.9%
May	7125.0	0.9237	3913.0	1.042	45.1%	12.8%
June	4323.0	1.093	2392.0	1.056	44.7%	3.4%
July	1078.0	1.359	1090.0	1.710	1.1%	25.8%
August	466.3	1.223	337.2	1.336	27.7%	9.2%
September	435.6	0.82	445.7	1.333	2.3%	62.6%
October	785.4	1.33	797.2	2.042	1.5%	53.5%
November	1941.0	1.553	1617.0	1.310	16.7%	15.6%
December	2161.0	1.015	2990.0	1.056	38.4%	4.0%
Average Score					20.4%	21.4%
					1	0
Parameter Group #2						
1-day minimum	79.89	0.6591	62.79	0.6443	21.4%	2.2%
3-day minimum	83.46	0.6797	66.12	0.6274	20.8%	7.7%
7-day minimum	90.75	0.7075	71.89	0.6155	20.8%	13.0%
30-day minimum	145.9	0.8675	107.2	0.6236	26.5%	28.1%
90-day minimum	378.8	0.9232	274.1	0.9777	27.6%	5.9%
1-day maximum	30450	0.9384	15210	0.6185	50.0%	34.1%
3-day maximum	28690	0.9349	14400	0.5959	49.8%	36.3%
7-day maximum	23640	0.8706	12900	0.5741	45.4%	34.1%
30-day maximum	11980	0.6614	8777	0.5506	26.7%	16.8%
90-day maximum	6539	0.6169	5684	0.5471	13.1%	11.3%
Average Score					30.2%	18.9%
					0	0
Parameter Group #3						
Date of minimum	257.8	0.0719	250.2	0.09071	2.9%	26.2%
Date of maximum	142.7	0.212	62.93	0.1872	55.9%	11.7%
Average Score					29.4%	18.9%
					2	0
Parameter Group #4						
Low pulse count	3.55	0.4702	5.074	0.5062	42.9%	7.7%
Low pulse duration	28.76	0.7992	25.34	0.7494	11.9%	6.2%
High pulse count	2.15	0.8024	2.389	0.8583	11.1%	7.0%
High pulse duration	12.29	0.5222	11.92	0.6441	3.0%	23.3%
Average Score					17.2%	11.0%
					0	0
Parameter Group #5						
Rise rate	469.2	0.6892	367.2	0.5129	21.7%	25.6%
Fall rate	-327.4	-0.6447	-213.6	-0.4756	34.8%	26.2%
Number of reversals	61.75	0.1373	85.41	0.2142	38.3%	56.0%
Average Score					31.6%	35.9%
					0	0
Total Point					3	
Classification					2	
note:					Low risk of impact	

Table 8.3 DHRAM results at USGS 08022040 (Control Point SRBE)

Control Point:	SRGW(USGS)		Period: 1940-2013			
IHA statistics group	1940-1959		1960-2013		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	2126.0	0.8537	2170.0	1.040	2.1%	21.8%
February	2888.0	1.092	2617.0	0.889	9.4%	18.6%
March	2872.0	0.9689	3327.0	0.918	15.8%	5.3%
April	4437.0	1.245	2505.0	0.932	43.5%	25.1%
May	5495.0	0.9335	3205.0	1.192	41.7%	27.7%
June	3734.0	1.181	1613.0	1.075	56.8%	9.0%
July	707.0	1.316	757.3	1.924	7.1%	46.2%
August	301.5	1.298	213.6	1.304	29.2%	0.5%
September	317.8	0.8499	304.6	1.430	4.2%	68.3%
October	683.5	1.39	630.5	2.408	7.8%	73.2%
November	1555.0	1.689	1206.0	1.455	22.4%	13.9%
December	1851.0	1.118	2274.0	1.225	22.9%	9.6%
Average Score					21.9%	26.6%
					1	0
Parameter Group #2						
1-day minimum	48.1	0.6842	47.99	0.5402	0.2%	21.0%
3-day minimum	50.5	0.6921	49.63	0.5402	1.7%	21.9%
7-day minimum	55.13	0.6873	53.19	0.5228	3.5%	23.9%
30-day minimum	95.09	0.9114	75.61	0.561	20.5%	38.4%
90-day minimum	264.8	0.9516	176.7	0.9283	33.3%	2.4%
1-day maximum	32490	0.931	15020	0.741	53.8%	20.4%
3-day maximum	30010	0.8761	14460	0.7311	51.8%	16.6%
7-day maximum	24250	0.8049	12730	0.6986	47.5%	13.2%
30-day maximum	10590	0.6477	7347	0.612	30.6%	5.5%
90-day maximum	5418	0.6327	4296	0.5885	20.7%	7.0%
Average Score					26.4%	17.0%
					0	0
Parameter Group #3						
Date of minimum	248.6	0.06424	248.9	0.09245	0.1%	43.9%
Date of maximum	146.8	0.2637	62.8	0.1787	57.2%	32.2%
Average Score					28.7%	38.1%
					2	1
Parameter Group #4						
Low pulse count	4.05	0.4199	4.87	0.5903	20.2%	40.6%
Low pulse duration	23.35	0.7018	24.93	0.5609	6.8%	20.1%
High pulse count	2.35	0.7843	1.37	0.9749	41.7%	24.3%
High pulse duration	9.719	0.3032	11.54	0.4799	18.7%	58.3%
Average Score					21.9%	35.8%
					0	1
Parameter Group #5						
Rise rate	478	0.715	268.5	0.5813	43.8%	18.7%
Fall rate	-366.1	-0.717	-169.4	-0.5416	53.7%	24.5%
Number of reversals	60.55	0.1603	67.85	0.137	12.1%	14.5%
Average Score					36.5%	19.2%
					0	0
Total Point Classification					5	3
note:					<u>Moderate risk of impact</u>	

Table 8.4 DHRAM results at USGS 08030500 (Control Point SRRL)

Control Point:	SRRL(USGS)		Period: 1940-2013			
IHA statistics group	1940-1965		1966-2013		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	13800.0	0.8227	10910.0	0.745	20.9%	9.4%
February	14740.0	0.5986	12470.0	0.629	15.4%	5.0%
March	13940.0	0.5539	13010.0	0.724	6.7%	30.8%
April	13260.0	0.7929	10820.0	0.727	18.4%	8.3%
May	15920.0	0.9414	9310.0	0.888	41.5%	5.6%
June	10570.0	1.013	7200.0	0.766	31.9%	24.4%
July	5182.0	0.898	5879.0	1.033	13.5%	15.0%
August	2878.0	1.062	4330.0	0.510	50.5%	52.0%
September	2709.0	1.079	4073.0	0.714	50.4%	33.9%
October	2458.0	1.149	2985.0	1.027	21.4%	10.6%
November	4749.0	1.212	4334.0	1.118	8.7%	7.8%
December	9158.0	1.106	8007.0	0.816	12.6%	26.2%
Average Score					24.3%	19.1%
					1	0
Parameter Group #2						
1-day minimum	857.4	0.521	870.8	0.3784	1.6%	27.4%
3-day minimum	870.3	0.5278	936.5	0.4217	7.6%	20.1%
7-day minimum	897.7	0.5322	1031	0.4773	14.8%	10.3%
30-day minimum	1116	0.57	1392	0.6109	24.7%	7.2%
90-day minimum	1896	0.6886	2436	0.6029	28.5%	12.4%
1-day maximum	44830	0.5375	40620	0.5783	9.4%	7.6%
3-day maximum	43520	0.548	38650	0.5822	11.2%	6.2%
7-day maximum	40000	0.568	33670	0.5759	15.8%	1.4%
30-day maximum	28320	0.5734	21950	0.497	22.5%	13.3%
90-day maximum	18900	0.5056	15060	0.4903	20.3%	3.0%
Average Score					15.6%	10.9%
					0	0
Parameter Group #3						
Date of minimum	275.5	0.07237	288.2	0.1136	4.6%	57.0%
Date of maximum	128.4	0.2649	52.29	0.2024	59.3%	23.6%
Average Score					31.9%	40.3%
					2	1
Parameter Group #4						
Low pulse count	3.4	0.5023	8.714	0.65	156.3%	29.4%
Low pulse duration	29.51	0.6863	13.51	0.9187	54.2%	33.9%
High pulse count	2.96	0.6577	2.939	0.8547	0.7%	30.0%
High pulse duration	14.23	0.758	8.544	0.5868	40.0%	22.6%
Average Score					62.8%	29.0%
					1	0
Parameter Group #5						
Rise rate	1068	0.4124	869.6	0.3499	18.6%	15.2%
Fall rate	-684.9	-0.4158	-768.9	-0.3791	12.3%	8.8%
Number of reversals	50.76	0.1176	93.8	0.2811	84.8%	139.0%
Average Score					38.5%	54.3%
					0	1
Total Point Classification					6	3
note:					<u>Moderate risk of impact</u>	

Neches River Basin

The Neches River Basin has five control points that have environmental flow standards. All the five control points have daily recorded flow for long term periods. However, control points ANAL (USGS gauge number 08036500) has a significant gap of missing data. This control point is, therefore, excluded for the DHRAM analysis.

The USGS gauging station (08032000, NENE) located on the Neches River at Neches, and downstream of Palestine Lake, has daily recorded flow data for the period 1939 to present. The Palestine Lake has been operated since 1962. The period-of-record is divided into two periods on the basis of 1962. Table 8.5 presents the result of the analysis. Monthly average flows for each month for the impacted period are smaller than the un-impacted period. Minimum and maximum daily flows for certain periods from 1 day to 90 days increase and decrease, respectively for the impacted period than the un-impacted period. The average date of daily maximum flow for the impacted period is put forward about 60 days than the un-impacted period. The analysis yields a total of 8 impact points and a corresponding DHRAM class of 3.

The USGS gauging station (08033500, NERO) located on the Neches River near Rockland, and downstream of the control point NENE, has daily recorded flow data for the period 1903 to present. The period-of-record is divided into two periods on the basis of 1962 like the control point NENE. Table 8.6 provides the result of the analysis. Monthly average flows for each month for the impacted period are also smaller than the un-impacted period, but the differences between both the periods are insignificant. Minimum and maximum daily flows for certain periods from 1 day to 90 days slightly increase and decrease, respectively, for the impacted period than the impacted period. However, flow timings for both periods are almost similar. The analysis yields a total of 0 impact points and a corresponding DHRAM class of 1.

The USGS gauging station (08041000, NEEV) located on the Neches River at Evadale, and downstream of Sam Rayburn and Town Bluff dams, has daily recorded flow data for the period 1904 to present. The Sam Rayburn was initially impounded in 1965. Thus, the period-of-record is divided into two periods on the basis of 1965. Table 8.7

provides the result of the analysis. Monthly average flows from July to September for the impacted period tremendously increase than the un-impacted period. Minimum daily flows for certain periods for the impacted period are significantly larger than the un-impacted period while maximum flows for certain period slightly decrease than the un-impacted period. The average date of daily maximum flow for the impacted period is put forward about 60 days than the un-impacted period. The analysis yields a total of 10 impact points and a corresponding DHRAM class of 3.

The USGS gauging station (08041500, VIKO) located on the Village Creek near Kountze has the daily recorded flow data for the period 1924 to present. There are no serious human influences upstream of the station. The period-of-record is divided into two periods on the basis of 1977 that is the middle year for the period-of-record. Table 8.8 provides the result of the analysis. Monthly average flows for the impacted period increase than the un-impacted period. The average date of daily maximum flow for the impacted period is put off about 140 days than the un-impacted period. The analysis yields a total of 4 impact points and a corresponding DHRAM class of 2.

GSA River Basins

The GSA River Basins have fifteen control points that have environmental flow standards. Of the 15 control points, only six control points, CP01, CP02, CP08, CP10, CP15, and CP35 do not have periods of missing data during the periods-of-record. These recorded data are therefore analyzed by the DHRAM method after dividing into un-impacted and impacted periods at these control points. The control points that have daily recorded flow without missing period along the San Antonio River are excluded in the DHRAM analysis because the flow regime of the San Antonio River has been totally influenced by the operation of Medina Lake since 1913.

The USGS gauging station (08167000, CP01) located on the Guadalupe River at Comfort, and upstream of Canyon Lake, has daily recorded flow data for the period 1939 to present. It is difficult to find the turning point the moment the flow regime obviously had been changed for the period-of-record at the gauging station. Thus, the period-of-

record is divided into two periods on the basis of 1976 that is the middle year for the period-of-record. Table 8.9 presents the results of the analysis. The monthly average flows and minimum and maximum flows for certain days for the impacted period significantly increase than the un-impacted period, but the flow timings are almost identical. The analysis yields a total of 9 impact points and a corresponding DHRAM class of 3.

The USGS gauging station (08167500, CP02) located on the Guadalupe River near Spring Branch, and upstream of Canyon Lake, has daily recorded flow data for the period 1934 to present. This gauge also doesn't have the obvious turning point for the period-of-record like the USGS gauge 08167000. The period-of-record is, therefore, divided into two periods on the basis of 1974 that is the middle year for the period-of-record. Table 8.10 presents the result of the analysis. The analysis results are almost similar to the control point CP01. The analysis yields a total of 7 impact points and a corresponding DHRAM class of 3.

The USGS gauging station (08171000, CP08) located on the Blanco River at Wimberley, has daily recorded flow data for the period 1934 to present. There are no obvious human influences on the flow regime at this gauge for the period-of-record. The period-of-record is, therefore, divided into two periods on the basis of 1974 that is the middle year for the period-of-record. Table 8.11 presents the result of the analysis. The analysis results are almost the same as the CP01 and CP02. The analysis yields a total of 6 impact points and a corresponding DHRAM class of 3.

The USGS gauging station (08172000, CP10) located on the San Marcos River at Luling has the daily recorded flow data for the period 1939 to present. There are also no obvious human influences on the flow regime at this gauge for the period-of-record. The period-of-record is, therefore, divided into two periods on the basis of 1976 that is the middle year for the period-of-record. Table 8.12 presents the result of the analysis. The analysis results are almost the same as the CP01, CP02, and CP08 even though the differences between both the periods are smaller than others. The analysis yields a total of 2 impact points and a corresponding DHRAM class of 2.

The USGS gauging station (08176500, CP15) located on the Guadalupe River at Vitoria, and downstream far from Canyon Lake, has daily recorded flow data for the period 1934 to present. Canyon Lake has been operated since 1964. The period-of-record is, therefore, divided into two periods based on 1964. Table 8.13 presents the result of the analysis. The analysis results are almost the same as the CP10. The analysis yields a total of 1 impact points and a corresponding DHRAM class of 2.

The USGS gauging station (08186000, CP35) located on the Cibolo Creek near Falls City has daily recorded flow data for the period 1930 to present. There are also no clear human influences on the flow regime at this gauge for the period-of-record. The period-of-record is, therefore, divided into two periods on the basis of 1973 that is the middle year for the period-of-record. Table 8.14 presents the result of the analysis. The analysis results are almost the same as the above control points. The analysis yields a total of seven impact points and a corresponding DHRAM class of 3.

Table 8.5 DHRAM results at USGS 08032000 (Control Point NENE)

Control Point:	NENE(USGS)		Period:	1940-2013		
IHA statistics group	1940-1961		1962-2013		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1153.0	0.8116	845.2	1.094	26.7%	34.8%
February	1230.0	0.7364	1041.0	0.757	15.4%	2.7%
March	1327.0	0.7282	1246.0	0.753	6.1%	3.4%
April	1498.0	1.058	1081.0	0.895	27.8%	15.4%
May	1675.0	0.8675	1088.0	1.046	35.0%	20.6%
June	787.4	1.075	709.7	1.082	9.9%	0.7%
July	244.9	1.161	325.0	2.564	32.7%	120.8%
August	74.9	1.248	128.2	0.970	71.3%	22.3%
September	121.5	1.639	187.3	1.382	54.2%	15.7%
October	228.9	1.169	251.6	1.861	9.9%	59.2%
November	576.8	1.535	415.0	1.285	28.1%	16.3%
December	814.0	1.017	732.2	1.096	10.0%	7.8%
Average Score					27.3%	26.6%
					1	0
Parameter Group #2						
1-day minimum	23.27	0.7129	64.9	0.5399	178.9%	24.3%
3-day minimum	24.44	0.721	66.63	0.5295	172.6%	26.6%
7-day minimum	26.48	0.7308	68.73	0.5221	159.6%	28.6%
30-day minimum	38.75	0.7246	81.34	0.5339	109.9%	26.3%
90-day minimum	108.1	1.198	124.3	0.9416	15.0%	21.4%
1-day maximum	11010	0.8298	6370	0.8797	42.1%	6.0%
3-day maximum	10090	0.8044	5956	0.8565	41.0%	6.5%
7-day maximum	7869	0.7363	4925	0.812	37.4%	10.3%
30-day maximum	3331	0.5409	2563	0.6157	23.1%	13.8%
90-day maximum	1911	0.4962	1561	0.5725	18.3%	15.4%
Average Score					79.8%	17.9%
					1	0
Parameter Group #3						
Date of minimum	247	0.05489	237.7	0.1327	3.8%	141.8%
Date of maximum	130.5	0.2121	74.38	0.1885	43.0%	11.1%
Average Score					23.4%	76.4%
					2	3
Parameter Group #4						
Low pulse count	3.227	0.4775	3.981	0.8346	23.4%	74.8%
Low pulse duration	40.44	1.074	23.36	1.186	42.2%	10.4%
High pulse count	3.227	0.7215	1.962	0.8971	39.2%	24.3%
High pulse duration	7.816	0.3388	8.891	0.4013	13.8%	18.4%
Average Score					29.6%	32.0%
					0	1
Parameter Group #5						
Rise rate	205.6	0.6876	137.2	0.696	33.3%	1.2%
Fall rate	-112.7	-0.5826	-67.11	-0.6445	40.5%	10.6%
Number of reversals	56.27	0.1625	78.92	0.1661	40.3%	2.2%
Average Score					38.0%	4.7%
					0	0
Total Point					8	
Classification					3	
note:					Moderate risk of impact	

Table 8.6 DHRAM results at USGS 08033500 (Control Point NERO)

Control Point:	NERO(USGS)		Period:	1940-2013		
IHA statistics group	1940-1961		1962-2013		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	4053.0	0.9705	3564.0	0.996	12.1%	2.7%
February	4270.0	0.7708	4077.0	0.744	4.5%	3.5%
March	4020.0	0.6792	4198.0	0.821	4.4%	20.9%
April	3967.0	0.9121	3404.0	0.800	14.2%	12.3%
May	5481.0	1.073	3225.0	0.937	41.2%	12.6%
June	2674.0	0.8613	2368.0	1.166	11.4%	35.4%
July	904.6	0.9399	1230.0	1.801	36.0%	91.6%
August	337.9	1.048	406.6	1.514	20.3%	44.5%
September	403.0	1.237	455.4	1.239	13.0%	0.2%
October	627.1	1.291	859.6	1.950	37.1%	51.0%
November	1844.0	1.574	1563.0	1.368	15.2%	13.1%
December	2737.0	1.26	2528.0	0.959	7.6%	23.9%
Average Score					18.1%	26.0%
					0	0
Parameter Group #2						
1-day minimum	91.09	0.7557	110.2	0.607	21.0%	19.7%
3-day minimum	93.52	0.7605	112.6	0.6127	20.4%	19.4%
7-day minimum	99.07	0.7664	118.1	0.6139	19.2%	19.9%
30-day minimum	131.7	0.7609	161.8	0.7516	22.9%	1.2%
90-day minimum	315.9	0.9481	303.8	0.9812	3.8%	3.5%
1-day maximum	18110	0.6856	16290	0.6045	10.0%	11.8%
3-day maximum	17590	0.6857	15680	0.6034	10.9%	12.0%
7-day maximum	15910	0.6747	13930	0.5877	12.4%	12.9%
30-day maximum	9558	0.6412	8216	0.5332	14.0%	16.8%
90-day maximum	5907	0.5475	5210	0.5589	11.8%	2.1%
Average Score					14.6%	11.9%
					0	0
Parameter Group #3						
Date of minimum	258	0.1711	257.8	0.07513	0.1%	56.1%
Date of maximum	131	0.2506	139.8	0.2694	6.7%	7.5%
Average Score					3.4%	31.8%
					0	0
Parameter Group #4						
Low pulse count	3.045	0.4586	3.769	0.5137	23.8%	12.0%
Low pulse duration	39.26	1.011	30.61	1.004	22.0%	0.7%
High pulse count	2.773	0.7207	2.788	0.7758	0.5%	7.6%
High pulse duration	14.91	0.6251	11.12	0.5513	25.4%	11.8%
Average Score					17.9%	8.0%
					0	0
Parameter Group #5						
Rise rate	383.5	0.5392	407.5	0.5681	6.3%	5.4%
Fall rate	-209.7	-0.5431	-204.5	-0.5587	2.5%	2.9%
Number of reversals	59.91	0.1505	68.71	0.1538	14.7%	2.2%
Average Score					7.8%	3.5%
					0	0
Total Point					0	
Classification					1	
note:					<u>Un-impacted condition</u>	

Table 8.7 DHRAM results at USGS 08041000 (Control Point NEEV)

Control Point:	NEEV(USGS)		Period: 1940-2013			
IHA statistics group	1940-1964		1965-2013		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	9980.0	0.9255	7290.0	0.843	27.0%	8.9%
February	10560.0	0.7117	8744.0	0.725	17.2%	1.9%
March	9862.0	0.6156	9984.0	0.760	1.2%	23.4%
April	9167.0	0.7086	8951.0	0.709	2.4%	0.0%
May	12050.0	1.039	7722.0	0.766	35.9%	26.3%
June	6486.0	0.9567	6664.0	0.820	2.7%	14.3%
July	2520.0	0.7021	5094.0	0.898	102.1%	27.9%
August	1343.0	0.7074	3346.0	0.661	149.1%	6.6%
September	1298.0	0.962	2951.0	0.527	127.3%	45.2%
October	1707.0	1.305	3055.0	0.861	79.0%	34.0%
November	3615.0	1.569	3720.0	0.864	2.9%	44.9%
December	6873.0	1.396	5233.0	0.831	23.9%	40.5%
Average Score					47.6%	22.8%
					2	0
Parameter Group #2						
1-day minimum	374.8	0.7416	1336	0.6506	256.5%	12.3%
3-day minimum	387.1	0.7487	1380	0.6326	256.5%	15.5%
7-day minimum	408.8	0.7276	1485	0.601	263.3%	17.4%
30-day minimum	534.4	0.6818	1828	0.534	242.1%	21.7%
90-day minimum	1010	0.7764	2334	0.5064	131.1%	34.8%
1-day maximum	34700	0.6253	21520	0.4239	38.0%	32.2%
3-day maximum	34150	0.6241	20890	0.4319	38.8%	30.8%
7-day maximum	32230	0.6263	19230	0.4502	40.3%	28.1%
30-day maximum	21420	0.6448	14840	0.4931	30.7%	23.5%
90-day maximum	13680	0.5608	11440	0.5578	16.4%	0.5%
Average Score					131.4%	21.7%
					2	0
Parameter Group #3						
Date of minimum	284.9	0.09446	318.3	0.2093	11.7%	121.6%
Date of maximum	119.5	0.261	63.45	0.209	46.9%	19.9%
Average Score					29.3%	70.7%
					2	3
Parameter Group #4						
Low pulse count	3.6	0.5893	2	1.35	44.4%	129.1%
Low pulse duration	29.52	0.8241	14.38	1.001	51.3%	21.5%
High pulse count	2.4	0.6697	2.02	1.008	15.8%	50.5%
High pulse duration	17.8	0.718	21.93	0.8367	23.2%	16.5%
Average Score					33.7%	54.4%
					0	1
Parameter Group #5						
Rise rate	693.5	0.4347	497.9	0.3784	28.2%	13.0%
Fall rate	-486.5	-0.421	-375	-0.4057	22.9%	3.6%
Number of reversals	60.24	0.1864	78.31	0.1568	30.0%	15.9%
Average Score					27.0%	10.8%
					0	0
Total Point Classification					10	3
note:					<u>Moderate risk of impact</u>	

Table 8.8 DHRAM results at USGS 08041500 (Control Point VIKO)

Control Point:	VIKO(USGS)		Period: 1940-2013			
IHA statistics group	1940-1976		1977-2013		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1386.0	1.016	1438.0	0.746	3.8%	26.5%
February	1316.0	0.7582	1649.0	0.655	25.3%	13.6%
March	1020.0	0.6697	1319.0	0.595	29.3%	11.1%
April	1028.0	0.8626	1120.0	1.092	8.9%	26.6%
May	1203.0	1.093	941.6	1.113	21.7%	1.8%
June	786.5	1.559	806.7	1.176	2.6%	24.6%
July	429.0	1.043	553.0	1.542	28.9%	47.8%
August	261.5	1.341	239.7	0.995	8.3%	25.8%
September	299.5	1.374	347.7	1.237	16.1%	10.0%
October	356.9	1.993	673.8	2.335	88.8%	17.2%
November	746.8	1.708	874.6	1.278	17.1%	25.2%
December	1005.0	1.116	1191.0	0.827	18.5%	25.9%
Average Score					22.4%	21.3%
					1	0
Parameter Group #2						
1-day minimum	78.05	0.6986	75.95	0.4939	2.7%	29.3%
3-day minimum	79.45	0.7047	77.61	0.4972	2.3%	29.4%
7-day minimum	82.67	0.728	81.32	0.5073	1.6%	30.3%
30-day minimum	109.8	0.973	105.3	0.5263	4.1%	45.9%
90-day minimum	189.3	1.052	188.9	0.8197	0.2%	22.1%
1-day maximum	13130	1.07	14660	0.8553	11.7%	20.1%
3-day maximum	10810	0.9765	12320	0.8058	14.0%	17.5%
7-day maximum	7108	0.8081	8411	0.7403	18.3%	8.4%
30-day maximum	3035	0.7291	3522	0.5403	16.0%	25.9%
90-day maximum	1714	0.5746	2017	0.4526	17.7%	21.2%
Average Score					8.9%	25.0%
					0	0
Parameter Group #3						
Date of minimum	254	0.1031	254.7	0.09211	0.3%	10.7%
Date of maximum	163.2	0.2766	301.7	0.311	84.9%	12.4%
Average Score					42.6%	11.5%
					3	0
Parameter Group #4						
Low pulse count	4.514	0.6841	4.973	0.5948	10.2%	13.1%
Low pulse duration	21.91	0.7163	15.68	0.7503	28.4%	4.7%
High pulse count	4	0.8539	4.946	0.6688	23.7%	21.7%
High pulse duration	5.547	0.3702	5.866	0.3352	5.8%	9.5%
Average Score					17.0%	12.2%
					0	0
Parameter Group #5						
Rise rate	303.8	0.7268	335.3	0.5706	10.4%	21.5%
Fall rate	-164.2	-0.774	-176.4	-0.6086	7.4%	21.4%
Number of reversals	69.78	0.1055	71.41	0.08268	2.3%	21.6%
Average Score					6.7%	21.5%
					0	0
Total Point Classification					4	2
note:					<u>Low risk of impact</u>	

Table 8.9 DHRAM results at USGS 08167000 (Control Point CP01)

Control Point:	CP01(USGS)	Period:	1940-2013			
IHA statistics group	1940-1975		1976-2013	Absolute Chages		
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	144.5	1.072	198.5	0.843	37.4%	21.3%
February	184.8	1.073	238.6	1.173	29.1%	9.3%
March	158.5	0.8727	272.9	0.999	72.2%	14.4%
April	183.3	0.9184	303.4	1.170	65.5%	27.4%
May	263.6	1.118	298.1	0.918	13.1%	17.9%
June	158.8	0.9292	396.9	1.632	149.9%	75.6%
July	86.1	1.001	395.1	2.488	358.8%	148.6%
August	147.4	2.089	290.0	2.748	96.7%	31.5%
September	128.4	1.088	179.2	0.977	39.6%	10.2%
October	226.5	1.257	290.5	1.476	28.3%	17.4%
November	134.5	0.9117	235.4	1.128	75.0%	23.7%
December	132.6	0.8372	256.0	1.680	93.1%	100.7%
Average Score					88.2%	41.5%
					3	1
Parameter Group #2						
1-day minimum	28.56	1.08	54.05	0.8358	89.3%	22.6%
3-day minimum	29.31	1.064	55.55	0.8266	89.5%	22.3%
7-day minimum	30.97	1.049	58.74	0.8021	89.7%	23.5%
30-day minimum	36.74	0.9606	68.4	0.7329	86.2%	23.7%
90-day minimum	55.79	0.8003	95.83	0.7098	71.8%	11.3%
1-day maximum	5661	1.085	12240	1.288	116.2%	18.7%
3-day maximum	2905	0.9678	6727	1.431	131.6%	47.9%
7-day maximum	1596	0.8675	3506	1.414	119.7%	63.0%
30-day maximum	602.5	0.7139	1202	1.138	99.5%	59.4%
90-day maximum	331.2	0.6515	624.4	0.9187	88.5%	41.0%
Average Score					98.2%	33.3%
					2	0
Parameter Group #3						
Date of minimum	210.4	0.1445	216.3	0.1923	2.8%	33.1%
Date of maximum	181.2	0.2212	182.1	0.2669	0.5%	20.7%
Average Score					1.7%	26.9%
					0	0
Parameter Group #4						
Low pulse count	4.306	0.9991	2.316	1.695	46.2%	69.7%
Low pulse duration	20.01	0.7701	14.36	0.6426	28.2%	16.6%
High pulse count	3.306	0.7116	3.237	0.8602	2.1%	20.9%
High pulse duration	2.829	0.8083	5.143	1.554	81.8%	92.3%
Average Score					39.6%	49.8%
					1	1
Parameter Group #5						
Rise rate	135.7	0.7785	233.1	1.1	71.8%	41.3%
Fall rate	-51.97	-0.7456	-81.76	-1.05	57.3%	40.8%
Number of reversals	91.97	0.1391	102.2	0.1317	11.1%	5.3%
Average Score					46.7%	29.1%
					1	0
Total Point					9	
Classification					3	
note:					Moderate risk of impact	

Table 8.10 DHRAM results at USGS 08167500 (Control Point CP02)

Control Point:	CP02(USGS)		Period:	1934-2013		
IHA statistics group	1934-1973		1974-2013		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	255.3	1.281	347.7	1.040	36.2%	18.8%
February	292.9	1.17	458.6	1.506	56.6%	28.7%
March	272.5	1.041	502.2	1.188	84.3%	14.1%
April	313.7	1.063	499.2	1.123	59.1%	5.6%
May	456.5	1.11	552.5	0.961	21.0%	13.4%
June	374.6	1.723	789.0	1.787	110.6%	3.7%
July	154.4	1.301	712.2	2.405	361.3%	84.9%
August	162.8	2.052	413.3	2.220	153.9%	8.2%
September	343.9	2.081	278.0	1.022	19.2%	50.9%
October	322.3	1.32	414.3	1.042	28.5%	21.1%
November	202.3	1.045	421.7	1.398	108.5%	33.8%
December	219.5	0.9134	424.2	1.850	93.3%	102.5%
Average Score					94.4%	32.1%
					3	1
Parameter Group #2						
1-day minimum	37.46	0.9507	74.9	0.7801	99.9%	17.9%
3-day minimum	38.39	0.9501	76.91	0.7762	100.3%	18.3%
7-day minimum	40.12	0.9429	79.12	0.7721	97.2%	18.1%
30-day minimum	49.22	0.8942	90.77	0.7479	84.4%	16.4%
90-day minimum	81.35	0.8494	137	0.7407	68.4%	12.8%
1-day maximum	9490	1.407	15880	1.22	67.3%	13.3%
3-day maximum	5152	1.176	9326	1.312	81.0%	11.6%
7-day maximum	2859	1.033	5352	1.314	87.2%	27.2%
30-day maximum	1114	0.9387	2039	1.113	83.0%	18.6%
90-day maximum	609.7	0.8131	1068	0.9379	75.2%	15.3%
Average Score					84.4%	16.9%
					1	0
Parameter Group #3						
Date of minimum	211.1	0.1754	223.3	0.2165	5.8%	23.4%
Date of maximum	178.3	0.2395	193.9	0.2592	8.7%	8.2%
Average Score					7.3%	15.8%
					1	0
Parameter Group #4						
Low pulse count	3.744	0.9742	2	1.789	46.6%	83.6%
Low pulse duration	23.34	0.7895	22.11	0.5101	5.3%	35.4%
High pulse count	3.333	0.8688	3.634	0.852	9.0%	1.9%
High pulse duration	2.626	0.5838	4.684	0.9653	78.4%	65.3%
Average Score					34.8%	46.6%
					0	1
Parameter Group #5						
Rise rate	282.8	1.013	380.5	1.059	34.5%	4.5%
Fall rate	-87.27	-1.038	-120	-0.9673	37.5%	6.8%
Number of reversals	74.49	0.173	90.95	0.1511	22.1%	12.7%
Average Score					31.4%	8.0%
					0	0
Total Point Classification					7	3
note:					<u>Moderate risk of impact</u>	

Table 8.11 DHRAM results at USGS 08171000 (Control Point CP08)

Control Point:	CP08USGS)		Period:	1934-2013		
IHA statistics group	1934-1973		1974-2013		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	120.6	1.644	135.1	1.059	12.0%	35.6%
February	146.8	1.31	185.2	1.369	26.2%	4.5%
March	134.8	1.117	194.0	1.117	43.9%	0.0%
April	158.9	1.152	177.1	1.075	11.5%	6.7%
May	191.6	1.229	190.0	0.799	0.8%	35.0%
June	148.4	1.197	300.1	1.619	102.2%	35.3%
July	67.3	1.409	226.2	2.273	236.0%	61.3%
August	38.7	0.7887	76.9	1.183	98.7%	50.0%
September	108.3	2.182	80.2	0.979	25.9%	55.1%
October	93.8	1.562	156.7	1.610	67.0%	3.1%
November	82.3	1.331	192.6	1.875	134.1%	40.9%
December	98.1	1.191	160.3	1.589	63.3%	33.4%
Average Score					68.5%	30.1%
					3	1
Parameter Group #2						
1-day minimum	17.87	0.7196	29.07	0.6189	62.7%	14.0%
3-day minimum	18.21	0.7113	29.74	0.6174	63.3%	13.2%
7-day minimum	18.79	0.6984	30.64	0.6156	63.1%	11.9%
30-day minimum	22.04	0.6715	34.77	0.6097	57.8%	9.2%
90-day minimum	31.59	0.7558	49.6	0.6978	57.0%	7.7%
1-day maximum	4314	1.467	5864	1.191	35.9%	18.8%
3-day maximum	2084	1.2	3044	1.187	46.1%	1.1%
7-day maximum	1192	1.06	1838	1.159	54.2%	9.3%
30-day maximum	474.6	0.8429	746.2	0.9949	57.2%	18.0%
90-day maximum	261.4	0.7883	382.5	0.8479	46.3%	7.6%
Average Score					54.4%	11.1%
					1	0
Parameter Group #3						
Date of minimum	224.9	0.2766	215.3	0.3331	4.3%	20.4%
Date of maximum	176.6	0.2451	191.7	0.2803	8.6%	14.4%
Average Score					6.4%	17.4%
					0	0
Parameter Group #4						
Low pulse count	3.744	1.311	2.171	1.739	42.0%	32.6%
Low pulse duration	23.64	0.8518	13.92	0.6313	41.1%	25.9%
High pulse count	3.308	0.9689	3.415	0.8073	3.2%	16.7%
High pulse duration	2.54	0.6942	4.054	0.8692	59.6%	25.2%
Average Score					36.5%	25.1%
					1	0
Parameter Group #5						
Rise rate	168	1.093	165.3	1.126	1.6%	3.0%
Fall rate	-46.13	-1.028	-47.59	-0.9882	3.2%	3.9%
Number of reversals	68.49	0.1384	92.54	0.1504	35.1%	8.7%
Average Score					13.3%	5.2%
					0	0
Total Point					6	
Classification					3	
note:					<u>Moderate risk of impact</u>	

Table 8.12 DHRAM results at USGS 08172000 (Control Point CP10)

Control Point:	CP10(USGS)	Period:	1940-2013			
IHA statistics group	1940-1975		1976-2013	Absolute Chages		
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	345.1	1.143	422.6	0.851	22.5%	25.6%
February	423.0	0.9228	480.2	1.220	13.5%	32.2%
March	354.4	0.7754	482.3	1.030	36.1%	32.8%
April	457.7	0.8866	451.5	0.876	1.4%	1.2%
May	545.1	0.9969	473.0	0.725	13.2%	27.2%
June	452.4	0.8071	701.0	1.444	55.0%	78.9%
July	261.8	0.9448	497.4	1.681	90.0%	77.9%
August	188.6	0.5873	253.0	0.923	34.1%	57.1%
September	311.1	1.062	258.6	0.651	16.9%	38.7%
October	348.6	1.164	463.0	1.941	32.8%	66.8%
November	322.3	0.9056	543.7	1.647	68.7%	81.9%
December	314.7	0.7869	468.7	1.365	48.9%	73.5%
Average Score					36.1%	49.5%
					1	1
Parameter Group #2						
1-day minimum	115	0.4386	128.6	0.3997	11.8%	8.9%
3-day minimum	121.1	0.4188	132.8	0.3829	9.7%	8.6%
7-day minimum	124.8	0.4127	136.5	0.3796	9.4%	8.0%
30-day minimum	136.3	0.4231	147.5	0.3888	8.2%	8.1%
90-day minimum	164.4	0.4914	178.2	0.4864	8.4%	1.0%
1-day maximum	8907	0.7921	11360	1.431	27.5%	80.7%
3-day maximum	4906	0.751	6760	1.255	37.8%	67.1%
7-day maximum	2758	0.7541	4120	1.159	49.4%	53.7%
30-day maximum	1121	0.6626	1714	0.9609	52.9%	45.0%
90-day maximum	668	0.6222	914.7	0.8019	36.9%	28.9%
Average Score					25.2%	31.0%
					0	0
Parameter Group #3						
Date of minimum	219	0.2716	214.9	0.2845	1.9%	4.7%
Date of maximum	178.9	0.221	186.1	0.2911	4.0%	31.7%
Average Score					2.9%	18.2%
					0	0
Parameter Group #4						
Low pulse count	5.139	1.353	3.605	1.382	29.9%	2.1%
Low pulse duration	18.58	0.8272	20.38	0.6991	9.7%	15.5%
High pulse count	4.833	0.7361	4.342	0.9154	10.2%	24.4%
High pulse duration	2.185	0.448	3.982	0.7598	82.2%	69.6%
Average Score					33.0%	27.9%
					0	0
Parameter Group #5						
Rise rate	267.4	0.8036	299.5	1.201	12.0%	49.5%
Fall rate	-106.2	-0.654	-93.07	-0.9613	12.4%	47.0%
Number of reversals	123.1	0.3753	107.5	0.2018	12.7%	46.2%
Average Score					12.3%	47.6%
					0	0
Total Point Classification					2	2
note:					Low risk of impact	

Table 8.13 DHRAM results at USGS 08176500 (Control Point CP15)

Control Point:	CP15(USGS)	Period:	1935-2013			
IHA statistics group	1935-1963		1964-2013		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1356.0	0.8342	1897.0	0.961	39.9%	15.2%
February	1635.0	1.042	2141.0	1.196	30.9%	14.8%
March	1430.0	0.8008	2005.0	0.883	40.2%	10.2%
April	1793.0	0.8691	2201.0	0.981	22.8%	12.8%
May	2539.0	1.097	2759.0	0.932	8.7%	15.0%
June	2088.0	0.9867	2996.0	1.319	43.5%	33.7%
July	1939.0	1.827	2045.0	1.482	5.5%	18.9%
August	692.5	0.645	1269.0	1.073	83.2%	66.4%
September	1469.0	1.024	1856.0	1.234	26.3%	20.5%
October	1742.0	1.269	2167.0	2.044	24.4%	61.1%
November	1521.0	1.201	2236.0	1.351	47.0%	12.5%
December	1269.0	0.8799	1892.0	1.096	49.1%	24.6%
				Average Score	35.1%	25.5%
					1	0
Parameter Group #2						
1-day minimum	376.7	0.6008	489.2	0.562	29.9%	6.5%
3-day minimum	413.8	0.578	516.2	0.5476	24.7%	5.3%
7-day minimum	437.6	0.5735	538.9	0.5328	23.1%	7.1%
30-day minimum	488.8	0.5625	601.6	0.5207	23.1%	7.4%
90-day minimum	655.2	0.5804	770.3	0.539	17.6%	7.1%
1-day maximum	24430	1.072	29520	1.552	20.8%	44.8%
3-day maximum	20060	1.075	24270	1.393	21.0%	29.6%
7-day maximum	13820	0.9815	16740	1.178	21.1%	20.0%
30-day maximum	5534	0.8567	7249	0.9208	31.0%	7.5%
90-day maximum	3136	0.7578	4188	0.7898	33.5%	4.2%
				Average Score	24.6%	13.9%
					0	0
Parameter Group #3						
Date of minimum	222.4	0.2196	224.4	0.2802	0.9%	27.6%
Date of maximum	179.7	0.2275	186	0.2881	3.5%	26.6%
				Average Score	2.2%	27.1%
					0	0
Parameter Group #4						
Low pulse count	5.379	1.105	4.64	1.287	13.7%	16.5%
Low pulse duration	16.63	1.225	13.94	1.205	16.2%	1.6%
High pulse count	4.034	0.8207	4.76	0.7728	18.0%	5.8%
High pulse duration	4.024	0.4154	6.021	0.6892	49.6%	65.9%
				Average Score	24.4%	22.5%
					0	0
Parameter Group #5						
Rise rate	518.8	0.8176	550	0.8987	6.0%	9.9%
Fall rate	-359.9	-0.7243	-357.4	-0.8588	0.7%	18.6%
Number of reversals	154.8	0.1571	144.1	0.1914	6.9%	21.8%
				Average Score	4.5%	16.8%
					0	0
Total Point Classification					1	2
note:					Low risk of impact	

Table 8.14 DHRAM results at USGS 08186000 (Control Point CP35)

Control Point:	CP35(USGS)		Period:	1934-2013		
IHA statistics group	1934-1972		1973-2013		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	97.9	2.677	97.3	1.331	0.6%	50.3%
February	91.7	1.746	141.4	2.024	54.2%	15.9%
March	50.8	0.8508	109.9	1.506	116.3%	77.0%
April	147.4	1.704	154.6	1.802	4.9%	5.8%
May	258.0	1.666	188.8	1.379	26.8%	17.2%
June	166.7	1.669	320.9	1.799	92.5%	7.8%
July	96.4	2.653	212.9	2.869	121.0%	8.1%
August	60.1	2.57	73.1	1.880	21.7%	26.8%
September	178.3	2.032	153.1	1.979	14.1%	2.6%
October	104.0	1.614	222.0	2.506	113.5%	55.3%
November	72.3	1.516	204.7	2.076	183.0%	36.9%
December	72.2	1.413	137.9	2.490	90.9%	76.2%
				Average Score	70.0%	31.7%
					3	1
Parameter Group #2						
1-day minimum	8.821	0.6038	18.44	0.7547	109.0%	25.0%
3-day minimum	9.137	0.5863	18.87	0.738	106.5%	25.9%
7-day minimum	9.604	0.5543	19.55	0.7216	103.6%	30.2%
30-day minimum	11.46	0.5388	22.85	0.6808	99.4%	26.4%
90-day minimum	18.89	0.6543	32.29	0.6552	70.9%	0.1%
1-day maximum	6853	0.8522	8005	1.146	16.8%	34.5%
3-day maximum	4173	0.8891	4950	1.169	18.6%	31.5%
7-day maximum	2299	0.9094	2868	1.161	24.7%	27.7%
30-day maximum	673	0.8573	946.7	1.06	40.7%	23.6%
90-day maximum	314.1	0.8626	414.6	0.9568	32.0%	10.9%
				Average Score	62.2%	23.6%
					1	0
Parameter Group #3						
Date of minimum	214.5	0.1574	228.7	0.1786	6.6%	13.5%
Date of maximum	177.7	0.2103	213.4	0.24	20.1%	14.1%
				Average Score	13.4%	13.8%
					1	0
Parameter Group #4						
Low pulse count	6.359	0.9016	1.878	1.507	70.5%	67.1%
Low pulse duration	12.62	0.6168	16.32	0.7075	29.3%	14.7%
High pulse count	4.051	0.7094	4.146	0.8045	2.3%	13.4%
High pulse duration	1.973	0.422	2.668	0.6985	35.2%	65.5%
				Average Score	34.3%	40.2%
					0	1
Parameter Group #5						
Rise rate	236.5	0.866	221.3	1.035	6.4%	19.5%
Fall rate	-100.7	-0.7555	-103.8	-1.202	3.1%	59.1%
Number of reversals	83.79	0.1745	85.27	0.1853	1.8%	6.2%
				Average Score	3.8%	28.3%
					0	0
Total Point Classification					7	3
note:					<u>Moderate risk of impact</u>	

8.1.2 Assessment of Naturalized versus Simulated Regulated Flows

Daily naturalized flows are considered un-impacted flows, and regulated flows, simulated by daily WRAP models under a full authorized water use scenarios, are considered impacted flows at the control points that have the environmental flow standards. The alterations between both the flow regimes on the flows are assessed by the DHRAM method. In fact, even though the environmental flow standards were established at each control point, these cannot restore flow regimes because other water rights have senior priorities to the environmental flow standards at the control points. Thus, the environmental flow standards restrict only future water uses. In other words, the DHRAM analysis theoretically indicates how much the flow regimes of the regulated flows are different from the naturalized flows under a full authorized water use condition.

Sabine River Basin

The period-of-analysis of the Sabine WAM is from 1940 to 2013. The Sabine WAM has the five control points that have environmental flow standards, but the DHRAM analyses are performed at the four control points for the comparison with the results with the USGS recorded flows.

Table 8.15 presents the result of the analysis at the control point BSBS. The means and coefficients of variance of both flows at the five parameter groups are almost identical. The analysis yields a total of 0 impact points and a corresponding DHRAM class of 1. This means that there are no human influences on the river flow.

The result of the analysis at the control point SRGW is as shown in Table 8.16. The monthly regulated flows decrease than the naturalized flows. The minimum and maximum flows of the regulated flow also decrease than the naturalized flow. Flow timing parameters of both flows are totally different. The analysis yields a total of 9 impact points and a corresponding DHRAM class of 3.

Table 8.17 provides the result of the analysis at the control point SRBE. The result is almost similar to the control point SRGW. The analysis yields a total of 10 impact points and a corresponding DHRAM class of 3.

Table 8.18 presents the result of the analysis at the control point SRRL. The result also is closely similar to the control points SRGW and SRBE. The analysis yields a total of 11 impact points and a corresponding DHRAM class of 4.

Neches River Basin

The period-of-analysis of the Neches WAM is from 1940 to 2013. The Neches WAM has also the five control points that have environmental flow standards, but the DHRAM analyses also are performed at the four control points for the comparison with the results with the USGS recorded flows.

Table 8.19 presents the result of the analysis at the control point NENE. The monthly flows and the minimum and maximum for certain days of the regulated flow slightly decrease than the naturalized flows. Most of all, flow timings of both the flows are tremendously different. The analysis yields a total of 6 impact points and a corresponding DHRAM class of 3.

The result of the analysis at the control points NERO is as shown in Table 8.20. The parameters of both the flows are nearly similar except for the parameter for flow timing. The average date of minimum of the regulated flow is put forward about 160 days than the naturalized flow. The analysis yields a total of 5 impact points and a corresponding DHRAM class of 3.

Table 8.21 presents the result of the analysis at the control points NEEV. The monthly flows of the regulated flow decrease than the naturalized flow. The minimum and maximum flows of the regulated flow also decrease than the naturalized flow. The average dates of minimum and maximum of the regulated flow are put forward about 190 and 70 days, respectively than the naturalized flow. The analysis yields a total of 11 impact points and a corresponding DHRAM class of 4.

Table 8.22 presents the result of the analysis at the control point VIKO. The means and coefficients of variance of both the flows at the five parameter groups are almost identical. The analysis yields a total of 0 impact points and a corresponding DHRAM class of 1.

Table 8.15 DHRAM results at Control Point BSBS (Naturalized vs. Regulated Flows)

Control Point:	BSBS		Period:	1940-2013		
IHA statistics group	Naturalized		Regulated		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	238.4	0.8167	233.9	0.8434	1.9%	3.3%
February	284.5	0.6652	280.2	0.6911	1.5%	3.9%
March	304	0.8101	299.3	0.8346	1.5%	3.0%
April	286.3	0.883	280.9	0.9071	1.9%	2.7%
May	283.2	0.8972	278.5	0.9213	1.7%	2.7%
June	170.5	0.9918	163.8	1.035	3.9%	4.4%
July	85.66	1.305	79.98	1.387	6.6%	6.3%
August	36.92	0.9991	33.94	1.01	8.1%	1.1%
September	59.45	1.452	55.79	1.533	6.2%	5.6%
October	97.84	1.95	93.12	2.05	4.8%	5.1%
November	155.7	1.174	148.9	1.234	4.4%	5.1%
December	235.8	0.8424	230.2	0.8702	2.4%	3.3%
Average Score					3.7%	3.9%
					0	0
Parameter Group #2						
1-day minimum	7.218	1.005	6.301	1.007	12.7%	0.2%
3-day minimum	7.844	0.9778	6.935	0.9662	11.6%	1.2%
7-day minimum	8.934	0.9385	7.922	0.9189	11.3%	2.1%
30-day minimum	17.35	0.6844	15.92	0.6635	8.2%	3.1%
90-day minimum	36.33	0.7197	33.31	0.7256	8.3%	0.8%
1-day maximum	2413	0.5878	2397	0.5941	0.7%	1.1%
3-day maximum	2137	0.6097	2123	0.6154	0.7%	0.9%
7-day maximum	1505	0.6067	1497	0.6134	0.5%	1.1%
30-day maximum	660.2	0.509	656.3	0.5179	0.6%	1.7%
90-day maximum	400.7	0.4952	396.5	0.5079	1.0%	2.6%
Average Score					5.6%	1.5%
					0	0
Parameter Group #3						
Date of minimum	204.7	0.2539	205.6	0.2416	0.4%	4.8%
Date of maximum	148.4	0.3181	148.4	0.318	0.0%	0.0%
Average Score					0.2%	2.4%
					0	0
Parameter Group #4						
Low pulse count	8.635	0.5587	9.122	0.5065	5.6%	9.3%
Low pulse duration	11.07	0.5347	10.17	0.482	8.1%	9.9%
High pulse count	5.676	0.5855	5.554	0.5841	2.1%	0.2%
High pulse duration	4.625	0.3475	4.688	0.3469	1.4%	0.2%
Average Score					4.3%	4.9%
					0	0
Parameter Group #5						
Rise rate	107.8	0.6271	102.8	0.6443	4.6%	2.7%
Fall rate	-36.28	-0.5553	-35.97	-0.5634	0.9%	1.5%
Number of reversals	98.69	0.156	101.8	0.1643	3.2%	5.3%
Average Score					2.9%	3.2%
					0	0
Total Point Classification					0	1
note:					Un-impacted condition	

Table 8.16 DHRAM results at Control Point SRGW (Naturalized vs. Regulated Flows)

Control Point:	SRGW		Period:	1940-2013		
IHA statistics group	Naturalized		Regulated, 50		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	2425	0.9114	1680	0.9105	30.7%	0.1%
February	2988	0.8102	1964	0.8478	34.3%	4.6%
March	3184	0.8404	2144	0.934	32.7%	11.1%
April	3140	1.137	2171	1.386	30.9%	21.9%
May	3899	1.058	2674	1.123	31.4%	6.1%
June	2442	1.221	1680	1.366	31.2%	11.9%
July	801.1	1.719	515.3	1.657	35.7%	3.6%
August	270	1.56	174.2	1.864	35.5%	19.5%
September	480.3	1.646	287.7	1.569	40.1%	4.7%
October	1184	2.126	658.2	2.015	44.4%	5.2%
November	1758	1.341	1143	1.469	35.0%	9.5%
December	2645	1.058	1751	1.112	33.8%	5.1%
Average Score					34.6%	8.6%
					1	0
Parameter Group #2						
1-day minimum	35.75	1.237	0.08108	8.602	99.8%	595.4%
3-day minimum	37.57	1.22	3.928	2.636	89.5%	116.1%
7-day minimum	41.84	1.189	14.97	1.781	64.2%	49.8%
30-day minimum	82.65	1.085	45.03	1.232	45.5%	13.5%
90-day minimum	247.8	0.9685	150.4	0.9816	39.3%	1.4%
1-day maximum	16970	0.6045	13190	0.7146	22.3%	18.2%
3-day maximum	15940	0.6093	11650	0.7449	26.9%	22.3%
7-day maximum	14330	0.6179	10040	0.7607	29.9%	23.1%
30-day maximum	8461	0.5885	5786	0.6781	31.6%	15.2%
90-day maximum	4929	0.5464	3333	0.6166	32.4%	12.8%
Average Score					48.1%	86.8%
					1	1
Parameter Group #3						
Date of minimum	255.4	0.1017	87.91	0.2015	65.6%	98.1%
Date of maximum	162.1	0.3099	162.5	0.3037	0.2%	2.0%
Average Score					32.9%	50.1%
					2	1
Parameter Group #4						
Low pulse count	3.946	0.4854	9.784	0.3546	147.9%	26.9%
Low pulse duration	26.22	0.7151	10.55	0.7223	59.8%	1.0%
High pulse count	2.919	0.6728	4.662	0.6423	59.7%	4.5%
High pulse duration	12.68	0.5818	6.875	0.6044	45.8%	3.9%
Average Score					78.3%	9.1%
					2	0
Parameter Group #5						
Rise rate	330.4	0.5222	405.4	0.4825	22.7%	7.6%
Fall rate	-194.4	-0.5095	-249.2	-0.471	28.2%	7.6%
Number of reversals	56.82	0.1528	118.7	0.1381	108.9%	9.6%
Average Score					53.3%	8.3%
					1	0
Total Point Classification					9	3
note:					<u>Moderate risk of impact</u>	

Table 8.17 DHRAM results at Control Point SRBE (Naturalized vs. Regulated Flows)

Control Point:	SRBE		Period: 1940-2013			
IHA statistics group	Naturalized		Regulated, 50		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	3514	0.8648	2543	0.9011	27.6%	4.2%
February	4193	0.7109	2972	0.7669	29.1%	7.9%
March	4257	0.7737	3009	0.8621	29.3%	11.4%
April	4005	1.036	2901	1.289	27.6%	24.4%
May	4904	1.015	3442	1.066	29.8%	5.0%
June	3215	1.085	2315	1.22	28.0%	12.4%
July	1117	1.389	725.6	1.436	35.0%	3.4%
August	424	1.401	270.8	1.689	36.1%	20.6%
September	663.5	1.541	410.2	1.515	38.2%	1.7%
October	1471	2.012	883.9	2.006	39.9%	0.3%
November	2325	1.23	1572	1.327	32.4%	7.9%
December	3384	0.9509	2294	1.011	32.2%	6.3%
Average Score					32.1%	8.8%
					1	0
Parameter Group #2						
1-day minimum	76.84	0.9809	3.565	3.533	95.4%	260.2%
3-day minimum	80.32	0.9697	22.1	1.33	72.5%	37.2%
7-day minimum	88.92	0.9438	39.47	1.114	55.6%	18.0%
30-day minimum	150.3	0.9023	78.53	0.9691	47.8%	7.4%
90-day minimum	399.5	0.9217	240.3	0.9706	39.8%	5.3%
1-day maximum	17110	0.5888	13710	0.6496	19.9%	10.3%
3-day maximum	16510	0.5971	12850	0.6596	22.2%	10.5%
7-day maximum	15230	0.6145	11460	0.6792	24.8%	10.5%
30-day maximum	10060	0.582	7260	0.6445	27.8%	10.7%
90-day maximum	6261	0.5175	4461	0.5787	28.7%	11.8%
Average Score					43.4%	38.2%
					1	0
Parameter Group #3						
Date of minimum	252.3	0.1018	99.58	0.2598	60.5%	155.2%
Date of maximum	153.7	0.3076	149.3	0.3004	2.9%	2.3%
Average Score					31.7%	78.8%
					2	3
Parameter Group #4						
Low pulse count	3.527	0.5828	8.473	0.457	140.2%	21.6%
Low pulse duration	33.34	0.8444	12.69	0.8744	61.9%	3.6%
High pulse count	3.054	0.7085	4.541	0.6928	48.7%	2.2%
High pulse duration	14.47	0.5802	8.268	0.5774	42.9%	0.5%
Average Score					73.4%	7.0%
					2	0
Parameter Group #5						
Rise rate	330.4	0.5169	408.8	0.4806	23.7%	7.0%
Fall rate	-202	-0.5119	-269.3	-0.4645	33.3%	9.3%
Number of reversals	56.39	0.1386	115.5	0.1326	104.8%	4.3%
Average Score					54.0%	6.9%
					1	0
Total Point Classification					10	3
note:					<u>Moderate risk of impact</u>	

Table 8.18 DHRAM results at Control Point SRRL (Naturalized vs. Regulated Flows)

Control Point:	SRRL		Period:	1940-2013		
IHA statistics group	Naturalized		Regulated, 50		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	13120	0.7497	8407	1.084	35.9%	44.6%
February	14690	0.578	10120	0.7961	31.1%	37.7%
March	13140	0.6133	9166	0.8048	30.2%	31.2%
April	12510	0.7303	8606	0.9995	31.2%	36.9%
May	11740	0.9706	7571	1.331	35.5%	37.1%
June	8696	0.925	5773	1.289	33.6%	39.4%
July	4722	1.278	3048	1.724	35.5%	34.9%
August	2270	1.13	1407	1.34	38.0%	18.6%
September	2523	1.212	1509	1.447	40.2%	19.4%
October	4069	1.49	2051	1.695	49.6%	13.8%
November	6420	1.156	3324	1.629	48.2%	40.9%
December	10280	0.8322	5894	1.176	42.7%	41.3%
Average Score					37.6%	33.0%
					1	1
Parameter Group #2						
1-day minimum	646.9	0.74	13.47	3.81	97.9%	414.9%
3-day minimum	660.6	0.7401	51.02	1.893	92.3%	155.8%
7-day minimum	692.8	0.7343	155.3	1.512	77.6%	105.9%
30-day minimum	926.3	0.7261	403.8	0.8313	56.4%	14.5%
90-day minimum	1768	0.7263	871.2	0.724	50.7%	0.3%
1-day maximum	45300	0.5594	36630	0.6513	19.1%	16.4%
3-day maximum	42360	0.5417	33770	0.6425	20.3%	18.6%
7-day maximum	37480	0.5058	29230	0.6407	22.0%	26.7%
30-day maximum	25840	0.4848	19610	0.6636	24.1%	36.9%
90-day maximum	17500	0.44	12450	0.6396	28.9%	45.4%
Average Score					48.9%	83.5%
					1	0
Parameter Group #3						
Date of minimum	261.6	0.1064	79.39	0.2629	69.7%	147.1%
Date of maximum	65.57	0.2011	74.54	0.212	13.7%	5.4%
Average Score					41.7%	76.3%
					3	3
Parameter Group #4						
Low pulse count	2.446	0.602	7.73	0.4549	216.0%	24.4%
Low pulse duration	44.17	0.6551	13.11	0.8384	70.3%	28.0%
High pulse count	3.176	0.6961	3.068	0.7405	3.4%	6.4%
High pulse duration	14.4	0.6013	13.51	0.6782	6.2%	12.8%
Average Score					74.0%	17.9%
					2	0
Parameter Group #5						
Rise rate	947.3	0.5524	871.9	0.5648	8.0%	2.2%
Fall rate	-479.8	-0.5361	-554.2	-0.5134	15.5%	4.2%
Number of reversals	58.31	0.153	84.61	0.1606	45.1%	5.0%
Average Score					22.9%	3.8%
					0	0
Total Point Classification					11	4
note:					High risk of impact	

Table 8.19 DHRAM results at Control Point NENE (Naturalized vs. Regulated Flows)

Control Point:	NENE		Period:	1940-2013		
IHA statistics group	Naturalized		Regulated, 2009		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1015	0.8536	515	1.174	49.3%	37.5%
February	1239	0.7049	691.9	0.9881	44.2%	40.2%
March	1219	0.6986	691.5	1.072	43.3%	53.4%
April	1155	1.074	683.3	1.441	40.8%	34.2%
May	1209	1.056	661.1	1.38	45.3%	30.7%
June	726.5	1.156	398.7	1.44	45.1%	24.6%
July	236.9	1.836	146.4	1.629	38.2%	11.3%
August	72.4	1.964	75.6	1.156	4.4%	41.1%
September	212.2	1.812	115.5	1.305	45.6%	28.0%
October	446.8	2.142	235.1	2.422	47.4%	13.1%
November	668.5	1.32	348.2	1.562	47.9%	18.3%
December	962.4	0.9138	432.4	1.075	55.1%	17.6%
Average Score					42.2%	29.2%
					1	0
Parameter Group #2						
1-day minimum	5.162	2.743	1.000	2.715	80.6%	1.0%
3-day minimum	5.946	2.855	2.216	1.782	62.7%	37.6%
7-day minimum	8.743	3.286	6.552	0.9939	25.1%	69.8%
30-day minimum	24.94	1.804	30.480	0.5144	22.2%	71.5%
90-day minimum	93.09	1.407	67.600	0.6896	27.4%	51.0%
1-day maximum	7681	0.6525	5423	0.765	29.4%	17.2%
3-day maximum	6887	0.6553	4565	0.8029	33.7%	22.5%
7-day maximum	5969	0.6271	3787	0.8088	36.6%	29.0%
30-day maximum	3056	0.5362	1845	0.7713	39.6%	43.8%
90-day maximum	1773	0.5009	1029	0.7476	42.0%	49.3%
Average Score					39.9%	39.3%
					0	0
Parameter Group #3						
Date of minimum	221.7	0.1533	63.16	0.2035	71.5%	32.7%
Date of maximum	67.27	0.1753	140.6	0.2785	109.0%	58.9%
Average Score					90.3%	45.8%
					3	1
Parameter Group #4						
Low pulse count	4.324	0.4563	10.61	0.2773	145.4%	39.2%
Low pulse duration	24.74	0.8672	9.132	0.7695	63.1%	11.3%
High pulse count	4.635	0.6191	4.892	0.654	5.5%	5.6%
High pulse duration	7.569	0.4332	5.111	0.6827	32.5%	57.6%
Average Score					61.6%	28.4%
					1	0
Parameter Group #5						
Rise rate	216.8	0.5456	179.2	0.6231	17.3%	14.2%
Fall rate	-114.8	-0.5323	-115.5	-0.5858	0.6%	10.1%
Number of reversals	52.88	0.2205	76.28	0.1905	44.3%	13.6%
Average Score					20.7%	12.6%
					0	0
Total Point Classification					6	3
note:					<u>Moderate risk of impact</u>	

Table 8.20 DHRAM results at Control Point NERO (Naturalized vs. Regulated Flows)

Control Point:	NERO		Period:	1940-2013		
IHA statistics group	Naturalized		Regulated, 2009		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	3854	0.8892	3284	0.9706	14.8%	9.2%
February	4158	0.7283	3602	0.8015	13.4%	10.1%
March	3573	0.7663	2961	0.894	17.1%	16.7%
April	3289	0.9322	2793	1.024	15.1%	9.8%
May	3709	1.1	3075	1.227	17.1%	11.5%
June	2347	1.105	1959	1.251	16.5%	13.2%
July	1002	1.686	837	1.864	16.5%	10.6%
August	305.2	1.549	250.4	1.622	18.0%	4.7%
September	519.9	1.478	386.1	1.595	25.7%	7.9%
October	1194	1.801	950.6	1.903	20.4%	5.7%
November	2181	1.457	1823	1.571	16.4%	7.8%
December	3072	1.081	2510	1.182	18.3%	9.3%
Average Score					17.4%	9.7%
					0	0
Parameter Group #2						
1-day minimum	45.64	1.637	2.203	6.26	95.2%	282.4%
3-day minimum	48.16	1.669	3.117	5.278	93.5%	216.2%
7-day minimum	53.78	1.666	6.502	3.249	87.9%	95.0%
30-day minimum	98.56	1.526	46.890	1.767	52.4%	15.8%
90-day minimum	298.9	1.249	216.200	1.392	27.7%	11.4%
1-day maximum	17230	0.6091	15770	0.6305	8.5%	3.5%
3-day maximum	15990	0.6048	14590	0.6364	8.8%	5.2%
7-day maximum	14270	0.5882	12850	0.6364	10.0%	8.2%
30-day maximum	8774	0.5577	7716	0.6207	12.1%	11.3%
90-day maximum	5512	0.5299	4754	0.6009	13.8%	13.4%
Average Score					41.0%	66.3%
					0	0
Parameter Group #3						
Date of minimum	247.2	0.1121	89.93	0.2799	63.6%	149.7%
Date of maximum	148.8	0.3018	150.1	0.2998	0.9%	0.7%
Average Score					32.2%	75.2%
					2	3
Parameter Group #4						
Low pulse count	2.905	0.5227	4.919	0.4486	69.3%	14.2%
Low pulse duration	35.54	0.7146	19.68	0.6138	44.6%	14.1%
High pulse count	3.486	0.7919	3.514	0.7317	0.8%	7.6%
High pulse duration	11.57	0.6183	11.32	0.7306	2.2%	18.2%
Average Score					29.2%	13.5%
					0	0
Parameter Group #5						
Rise rate	442.7	0.6301	448	0.6005	1.2%	4.7%
Fall rate	-207.3	-0.5878	-227.2	-0.551	9.6%	6.3%
Number of reversals	54.27	0.2735	63.69	0.2097	17.4%	23.3%
Average Score					9.4%	11.4%
					0	0
Total Point Classification					5	3
note:					Moderate risk of impact	

Table 8.21 DHRAM results at Control Point NEEV (Naturalized vs. Regulated Flows)

Control Point:	NEEV		Period: 1940-2013			
IHA statistics group	Naturalized		Regulated, 2009		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	9770	0.8516	6044	1.047	38.1%	22.9%
February	10790	0.6577	8426	0.9122	21.9%	38.7%
March	9273	0.7049	7594	0.9738	18.1%	38.1%
April	8424	0.8106	6024	1.062	28.5%	31.0%
May	8684	1.072	5801	1.124	33.2%	4.9%
June	5928	1.051	5057	1.38	14.7%	31.3%
July	2628	1.362	2126	1.653	19.1%	21.4%
August	998.8	1.151	642.6	1.51	35.7%	31.2%
September	1663	1.381	745.5	1.695	55.2%	22.7%
October	3318	1.822	1198	1.741	63.9%	4.4%
November	5588	1.398	3019	1.802	46.0%	28.9%
December	7953	0.9712	5126	1.242	35.5%	27.9%
Average Score					34.2%	25.3%
					1	0
Parameter Group #2						
1-day minimum	200.9	1.223	1.703	6.52	99.2%	433.1%
3-day minimum	209.6	1.22	9.554	3.111	95.4%	155.0%
7-day minimum	227.9	1.201	33.33	2.346	85.4%	95.3%
30-day minimum	395.7	1.223	133.5	1.606	66.3%	31.3%
90-day minimum	900.5	1.015	377.2	1.193	58.1%	17.5%
1-day maximum	37530	0.652	27020	0.6014	28.0%	7.8%
3-day maximum	35310	0.6293	25390	0.5739	28.1%	8.8%
7-day maximum	32120	0.5985	23360	0.5673	27.3%	5.2%
30-day maximum	20880	0.5489	16150	0.6132	22.7%	11.7%
90-day maximum	13450	0.5012	10110	0.6488	24.8%	29.4%
Average Score					53.5%	79.5%
					1	0
Parameter Group #3						
Date of minimum	259.4	0.1293	70.57	0.2688	72.8%	107.9%
Date of maximum	141.2	0.2954	74.97	0.1803	46.9%	39.0%
Average Score					59.9%	73.4%
					3	3
Parameter Group #4						
Low pulse count	2.514	0.4429	8.162	0.5905	224.7%	33.3%
Low pulse duration	41.43	0.9228	17.7	1.878	57.3%	103.5%
High pulse count	2.608	0.7558	3.122	0.7882	19.7%	4.3%
High pulse duration	16.94	0.6418	15.09	0.7559	10.9%	17.8%
Average Score					78.1%	39.7%
					2	1
Parameter Group #5						
Rise rate	677.1	0.5428	682.7	0.542	0.8%	0.1%
Fall rate	-386.8	-0.5466	-554	-0.6975	43.2%	27.6%
Number of reversals	45.53	0.19	82.43	0.2293	81.0%	20.7%
Average Score					41.7%	16.1%
					0	0
Total Point Classification					11	4
note:					High risk of impact	

Table 8.22 DHRAM results at Control Point VIKO (Naturalized vs. Regulated Flows)

Control Point:	VIKO		Period:	1940-2013		
IHA statistics group	Naturalized		Regulated, 2009		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1381	0.8871	1381	0.8874	0.0%	0.0%
February	1395	0.7546	1395	0.7549	0.0%	0.0%
March	1046	0.7362	1045	0.7367	0.1%	0.1%
April	959.4	1.103	958.7	1.103	0.1%	0.0%
May	1020	1.172	1020	1.172	0.0%	0.0%
June	827.8	1.456	827.2	1.457	0.1%	0.1%
July	532.5	1.415	531.6	1.416	0.2%	0.1%
August	264.1	1.148	263.4	1.149	0.3%	0.1%
September	398.1	1.492	397	1.494	0.3%	0.1%
October	650.2	2.009	649.6	2.011	0.1%	0.1%
November	968.2	1.409	967.6	1.41	0.1%	0.1%
December	1170	0.9349	1170	0.9354	0.0%	0.1%
Average Score					0.1%	0.1%
					0	0
Parameter Group #2						
1-day minimum	2.135	2.893	2.095	2.904	1.9%	0.4%
3-day minimum	2.428	2.747	2.419	2.716	0.4%	1.1%
7-day minimum	3.714	2.704	3.705	2.687	0.2%	0.6%
30-day minimum	35.5	2.269	35.350	2.279	0.4%	0.4%
90-day minimum	162	1.094	161.400	1.098	0.4%	0.4%
1-day maximum	9326	0.684	9326	0.6841	0.0%	0.0%
3-day maximum	8566	0.6866	8565	0.6867	0.0%	0.0%
7-day maximum	7177	0.6383	7176	0.6383	0.0%	0.0%
30-day maximum	3488	0.6087	3488	0.6088	0.0%	0.0%
90-day maximum	1921	0.5368	1921	0.537	0.0%	0.0%
Average Score					0.3%	0.3%
					0	0
Parameter Group #3						
Date of minimum	200.1	0.2048	199.1	0.2099	0.5%	2.5%
Date of maximum	59.78	0.2275	59.74	0.2275	0.1%	0.0%
Average Score					0.3%	1.2%
					0	0
Parameter Group #4						
Low pulse count	7.014	0.378	7.054	0.3826	0.6%	1.2%
Low pulse duration	13.02	0.3544	13.03	0.3538	0.1%	0.2%
High pulse count	4.595	0.705	4.608	0.7046	0.3%	0.1%
High pulse duration	7.483	0.5295	7.461	0.5294	0.3%	0.0%
Average Score					0.3%	0.4%
					0	0
Parameter Group #5						
Rise rate	305.8	0.5543	305.7	0.5547	0.0%	0.1%
Fall rate	-140.5	-0.5584	-140.7	-0.5587	0.1%	0.1%
Number of reversals	54.68	0.1635	54.81	0.1637	0.2%	0.1%
Average Score					0.1%	0.1%
					0	0
Total Point Classification					0	1
note:					Un-impacted condition	

GSA River Basins

The period-of-analysis of the GSA WAM is from 1934 to 2013. The GSA WAM has the fifteen control points that have environmental flow standards, but the DHRAM analyses also are performed at the six control points for the comparison with the results with the USGS recorded flows.

The flow regimes of both the flows are almost identical at the six control points in the GSA River Basins. The results of the analyses at the six control points are as shown in Tables 8.23 to 8.28, respectively. The DHRAM yields a total of 1 impact points at the control points CP01 and CP10, and a total of 0 impact points at the control points CP02, CP08, CP15, and CP35, respectively.

8.2 Comparative Evaluation

The DHRAM method is applied based on two different kinds of daily flow data: (1) the USGS daily recorded flows are firstly used in dividing into the un-impacted and impacted periods, and (2) naturalized and regulated flows, computed by the daily WRAP model, are used. Both results should be different due to the two reasons: (1) it is impossible to find the exact moment that divide into the un-impacted and impacted periods, and (2) the WRAP model also ideally simulated under the full authorized water use scenarios. In addition, the total of impact points based the WRAP simulation flows is theoretically higher than the USGS recorded flows due to the full water use scenarios if groundwater impacts are excluded.

Both the results may have similarities on the impact points at the five parameter groups, if the simulation is properly performed based on appropriate input datasets and water use scenarios. Thus, if these similarities between both results are proved though comparative analysis at the same point in this research, the daily WRAP model can be applied to predict and quantify the alteration of river flow regimes due to human influences.

Table 8.23 DHRAM results at Control Point CP01 (Naturalized vs. Regulated Flows)

Control Point:	CP01		Period:	1934-2013		
IHA statistics group	Naturalized		Regulated01		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	157.6	0.8912	147.1	0.93	6.7%	4.4%
February	189.1	0.9158	177.1	0.9555	6.3%	4.3%
March	184.1	0.836	171	0.8817	7.1%	5.5%
April	203.7	1.121	188.1	1.192	7.7%	6.3%
May	254.2	1.009	236.1	1.056	7.1%	4.7%
June	295.7	1.72	274.8	1.824	7.1%	6.0%
July	187.2	1.782	168.4	1.92	10.0%	7.7%
August	187.8	3.015	172.5	3.241	8.1%	7.5%
September	212.9	1.601	191.8	1.735	9.9%	8.4%
October	268	1.327	251.3	1.388	6.2%	4.6%
November	185.7	1.389	174.2	1.461	6.2%	5.2%
December	169.9	1.11	160	1.153	5.8%	3.9%
				Average Score	7.4%	5.7%
					0	0
Parameter Group #2						
1-day minimum	0.5	1.955	0.13	3.96	73.7%	102.7%
3-day minimum	0.6246	1.885	0.19	3.43	69.6%	82.0%
7-day minimum	1.18	1.813	0.58	2.41	50.9%	33.0%
30-day minimum	13.06	1.43	10.27	1.47	21.4%	2.7%
90-day minimum	62.3	0.8049	53.38	0.84	14.3%	4.0%
1-day maximum	6544	1.743	6416	1.77	2.0%	1.8%
3-day maximum	3550	1.501	3465	1.53	2.4%	2.2%
7-day maximum	2102	1.266	2045	1.30	2.7%	2.6%
30-day maximum	836.8	0.9856	806	1.02	3.7%	3.2%
90-day maximum	454.3	0.843	431	0.88	5.1%	3.8%
				Average Score	24.6%	23.8%
					0	0
Parameter Group #3						
Date of minimum	163.3	0.2483	131.6	0.2261	19.4%	8.9%
Date of maximum	191.8	0.2314	185.3	0.2383	3.4%	3.0%
				Average Score	11.4%	6.0%
					1	0
Parameter Group #4						
Low pulse count	7.075	0.4586	7.475	0.4187	5.7%	8.7%
Low pulse duration	12.75	0.4767	11.93	0.4308	6.4%	9.6%
High pulse count	2.238	0.7048	2.225	0.7257	0.6%	3.0%
High pulse duration	3.501	0.8156	3.485	0.8891	0.5%	9.0%
				Average Score	3.3%	7.6%
					0	0
Parameter Group #5						
Rise rate	156.7	1.087	136.8	1.086	12.7%	0.1%
Fall rate	-52.06	-1.003	-53.69	-1.02	3.1%	1.7%
Number of reversals	61.38	0.1761	74.2	0.174	20.9%	1.2%
				Average Score	12.2%	1.0%
					0	0
Total Point Classification					1	2
note:					Low risk of impact	

Table 8.24 DHRAM results at Control Point CP02 (Naturalized vs. Regulated Flows)

Control Point:	CP02		Period:	1934-2013		
IHA statistics group	Naturalized		Regulated01		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	301	1.141	291.2	1.166	3.3%	2.2%
February	341.7	1.084	330.7	1.109	3.2%	2.3%
March	334	0.9794	322.1	1.006	3.6%	2.7%
April	357.5	1.129	343.4	1.161	3.9%	2.8%
May	449.1	0.9971	432.4	1.018	3.7%	2.1%
June	549.8	1.747	530.2	1.797	3.6%	2.9%
July	308	1.62	290.5	1.682	5.7%	3.8%
August	253.1	2.392	239.4	2.498	5.4%	4.4%
September	333.5	1.59	314.8	1.659	5.6%	4.3%
October	427.4	1.134	412.3	1.158	3.5%	2.1%
November	329.3	1.439	319.1	1.476	3.1%	2.6%
December	308.7	1.143	299.7	1.165	2.9%	1.9%
				Average Score	4.0%	2.9%
					0	0
Parameter Group #2						
1-day minimum	2.209	2.178	1.72	2.49	22.0%	14.2%
3-day minimum	2.817	2.008	2.27	2.26	19.6%	12.5%
7-day minimum	4.691	2.033	4.01	2.20	14.6%	8.0%
30-day minimum	30.61	1.118	27.38	1.15	10.6%	2.8%
90-day minimum	98.25	0.8236	90.41	0.84	8.0%	2.1%
1-day maximum	7820	1.394	7740	1.40	1.0%	0.7%
3-day maximum	4733	1.194	4669	1.21	1.4%	0.9%
7-day maximum	3090	1.024	3040	1.04	1.6%	1.3%
30-day maximum	1375	0.884	1347	0.90	2.0%	1.5%
90-day maximum	779.1	0.7937	758	0.81	2.7%	1.7%
				Average Score	8.3%	4.6%
					0	0
Parameter Group #3						
Date of minimum	157.6	0.2414	151	0.2381	4.2%	1.4%
Date of maximum	187.7	0.251	186.2	0.2553	0.8%	1.7%
				Average Score	2.5%	1.5%
					0	0
Parameter Group #4						
Low pulse count	6.463	0.5394	6.663	0.5226	3.1%	3.1%
Low pulse duration	13.49	0.6091	13.27	0.6599	1.6%	8.3%
High pulse count	2.675	0.7658	2.65	0.7877	0.9%	2.9%
High pulse duration	5.356	0.6773	5.299	0.6805	1.1%	0.5%
				Average Score	1.7%	3.7%
					0	0
Parameter Group #5						
Rise rate	190.4	0.9338	180.8	0.9456	5.0%	1.3%
Fall rate	-66.24	-0.8401	-66.62	-0.847	0.6%	0.8%
Number of reversals	62.45	0.143	68.5	0.1525	9.7%	6.6%
				Average Score	5.1%	2.9%
					0	0
Total Point Classification					0	1
note:					Un-impacted condition	

Table 8.25 DHRAM results at Control Point CP08 (Naturalized vs. Regulated Flows)

Control Point:	CP08		Period:	1934-2013		
IHA statistics group	Naturalized		Regulated01		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	125.3	1.414	124.4	1.417	0.7%	0.2%
February	150.7	1.179	149.8	1.183	0.6%	0.3%
March	147.1	1.043	146.1	1.046	0.7%	0.3%
April	154.4	1.179	152.9	1.181	1.0%	0.2%
May	196.4	1.05	194.5	1.054	1.0%	0.4%
June	224.4	1.418	222.3	1.426	0.9%	0.6%
July	116.1	1.735	114.8	1.744	1.1%	0.5%
August	61.9	1.279	60.88	1.287	1.6%	0.6%
September	112	1.629	110.2	1.642	1.6%	0.8%
October	150.4	1.393	148.7	1.398	1.1%	0.4%
November	141.1	1.47	139.7	1.479	1.0%	0.6%
December	122.4	1.441	121.4	1.443	0.8%	0.1%
Average Score					1.0%	0.4%
					0	0
Parameter Group #2						
1-day minimum	2.708	1.002	2.46	1.01	9.3%	1.2%
3-day minimum	3.015	0.974	2.86	0.96	5.2%	1.0%
7-day minimum	3.816	1.03	3.64	0.98	4.6%	4.8%
30-day minimum	12.56	0.8365	12.26	0.84	2.4%	0.3%
90-day minimum	38.24	0.7582	37.57	0.76	1.8%	0.1%
1-day maximum	3545	1.406	3529	1.41	0.5%	0.1%
3-day maximum	2090	1.048	2077	1.05	0.6%	0.1%
7-day maximum	1294	0.885	1286	0.88	0.6%	0.0%
30-day maximum	563.9	0.7527	560	0.76	0.7%	0.4%
90-day maximum	308.9	0.7148	306	0.72	0.9%	0.4%
Average Score					2.7%	0.8%
					0	0
Parameter Group #3						
Date of minimum	78.81	0.2986	78.2	0.2942	0.8%	1.5%
Date of maximum	187.7	0.2637	187.7	0.2637	0.0%	0.0%
Average Score					0.4%	0.7%
					0	0
Parameter Group #4						
Low pulse count	7.5	0.5446	7.713	0.5556	2.8%	2.0%
Low pulse duration	11.48	0.5492	11.33	0.564	1.3%	2.7%
High pulse count	2.75	0.8069	2.763	0.811	0.5%	0.5%
High pulse duration	5.452	0.6799	5.436	0.6956	0.3%	2.3%
Average Score					1.2%	1.9%
					0	0
Parameter Group #5						
Rise rate	77.11	1.014	74.77	1.015	3.0%	0.1%
Fall rate	-29.96	-0.8709	-30.14	-0.8706	0.6%	0.0%
Number of reversals	83.13	0.1457	87.59	0.142	5.4%	2.5%
Average Score					3.0%	0.9%
					0	0
Total Point Classification					0	1
note:					Un-impacted condition	

Table 8.26 DHRAM results at Control Point CP10 (Naturalized vs. Regulated Flows)

Control Point:	CP10		Period:	1934-2013		
IHA statistics group	Naturalized		Regulated01		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	371.5	1.111	364.6	1.123	1.9%	1.1%
February	399	0.922	392.2	0.93	1.7%	0.9%
March	339.4	0.7574	331.9	0.7639	2.2%	0.9%
April	410.3	0.9457	400.8	0.9536	2.3%	0.8%
May	498.4	0.9128	486.7	0.922	2.3%	1.0%
June	536.5	1.296	522.3	1.319	2.6%	1.8%
July	316.7	1.229	305.5	1.254	3.5%	2.0%
August	199.2	0.6832	190.6	0.69	4.3%	1.0%
September	299.6	0.9683	290.7	0.9817	3.0%	1.4%
October	443.3	1.387	434.5	1.404	2.0%	1.2%
November	419.9	1.279	412.8	1.293	1.7%	1.1%
December	358.2	0.9849	351.8	0.9904	1.8%	0.6%
Average Score					2.4%	1.1%
					0	0
Parameter Group #2						
1-day minimum	1.696	2.088	0.88	3.14	48.0%	50.4%
3-day minimum	2.135	2.004	1.32	2.70	38.3%	34.5%
7-day minimum	3.689	1.877	2.73	2.31	25.9%	23.3%
30-day minimum	32.75	0.9026	31.12	0.92	5.0%	1.5%
90-day minimum	129.5	0.5554	124.90	0.56	3.6%	1.0%
1-day maximum	7061	1.104	6990	1.11	1.0%	0.7%
3-day maximum	4707	0.8669	4644	0.87	1.3%	0.8%
7-day maximum	3181	0.7928	3141	0.80	1.3%	0.8%
30-day maximum	1360	0.7428	1339	0.75	1.5%	1.0%
90-day maximum	776.3	0.6635	762	0.67	1.9%	1.0%
Average Score					12.8%	11.5%
					0	0
Parameter Group #3						
Date of minimum	166.4	0.2556	113.3	0.2631	31.9%	2.9%
Date of maximum	181.2	0.2372	179.1	0.2353	1.2%	0.8%
Average Score					16.5%	1.9%
					1	0
Parameter Group #4						
Low pulse count	9.113	0.4966	9	0.4576	1.2%	7.9%
Low pulse duration	9.845	0.331	9.931	0.3444	0.9%	4.0%
High pulse count	4.913	0.6631	4.925	0.6619	0.2%	0.2%
High pulse duration	3.909	0.4972	3.841	0.5278	1.7%	6.2%
Average Score					1.0%	4.6%
					0	0
Parameter Group #5						
Rise rate	330.1	0.6384	309.9	0.6435	6.1%	0.8%
Fall rate	-79.4	-0.5742	-79.25	-0.5875	0.2%	2.3%
Number of reversals	66.4	0.1356	72.69	0.119	9.5%	12.2%
Average Score					5.3%	5.1%
					0	0
Total Point					1	
Classification					2	
note:					Low risk of impact	

Table 8.27 DHRAM results at Control Point CP15 (Naturalized vs. Regulated Flows)

Control Point:	CP15		Period:	1934-2013		
IHA statistics group	Naturalized		Regulated01		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1599	1.104	1455	1.064	9.0%	3.6%
February	1703	1.001	1582	0.9751	7.1%	2.6%
March	1445	0.7707	1292	0.817	10.6%	6.0%
April	1742	1.006	1523	0.9749	12.6%	3.1%
May	2516	1.059	2171	1.116	13.7%	5.4%
June	2544	1.428	2283	1.362	10.3%	4.6%
July	1575	1.616	1513	1.588	3.9%	1.7%
August	855	1.033	761.2	1.066	11.0%	3.2%
September	1585	1.216	1327	1.244	16.3%	2.3%
October	2124	1.417	1939	1.379	8.7%	2.7%
November	1989	1.305	1889	1.245	5.0%	4.6%
December	1565	1.067	1560	0.9242	0.3%	13.4%
Average Score					9.0%	4.4%
					0	0
Parameter Group #2						
1-day minimum	59.18	1.461	44.16	0.94	25.4%	35.5%
3-day minimum	67.62	1.421	54.17	1.16	19.9%	18.5%
7-day minimum	86	1.378	73.07	1.37	15.0%	0.5%
30-day minimum	238.1	0.9643	257.30	0.94	8.1%	2.2%
90-day minimum	535.8	0.6979	541.20	0.60	1.0%	13.5%
1-day maximum	15310	0.8621	12750	0.90	16.7%	4.5%
3-day maximum	14200	0.8593	11900	0.91	16.2%	5.7%
7-day maximum	12460	0.8599	10410	0.92	16.5%	7.1%
30-day maximum	6431	0.8038	5645	0.84	12.2%	4.6%
90-day maximum	3657	0.7115	3297	0.76	9.8%	7.2%
Average Score					14.1%	9.9%
					0	0
Parameter Group #3						
Date of minimum	197.3	0.2327	193	0.2518	2.2%	8.2%
Date of maximum	187.8	0.2567	177.6	0.2739	5.4%	6.7%
Average Score					3.8%	7.5%
					0	0
Parameter Group #4						
Low pulse count	5.025	0.6041	6.663	0.4074	32.6%	32.6%
Low pulse duration	20.01	1.01	13.89	0.9515	30.6%	5.8%
High pulse count	2.75	0.7923	3.725	0.9061	35.5%	14.4%
High pulse duration	9.291	0.5614	8.485	0.8978	8.7%	59.9%
Average Score					26.8%	28.2%
					0	0
Parameter Group #5						
Rise rate	373.8	0.6418	304.7	0.632	18.5%	1.5%
Fall rate	-168.3	-0.6072	-193.3	-0.646	14.9%	6.4%
Number of reversals	53.58	0.1281	75.9	0.2099	41.7%	63.9%
Average Score					25.0%	23.9%
					0	0
Total Point Classification					0	1
note:					Un-impacted condition	

Table 8.28 DHRAM results at Control Point CP35 (Naturalized vs. Regulated Flows)

Control Point:	CP35		Period:	1934-2013		
IHA statistics group	Naturalized		Regulated01		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	94.84	2.25	93.96	2.269	0.9%	0.8%
February	96.71	1.479	95.43	1.495	1.3%	1.1%
March	65.14	1.775	63.04	1.828	3.2%	3.0%
April	141	1.841	138.1	1.873	2.1%	1.7%
May	204.2	1.613	200.6	1.636	1.8%	1.4%
June	211.8	1.976	207.8	2.008	1.9%	1.6%
July	135	2.715	132.4	2.759	1.9%	1.6%
August	60.42	2.136	58.49	2.19	3.2%	2.5%
September	159.5	1.97	157.2	1.995	1.4%	1.3%
October	163.8	1.942	162.2	1.958	1.0%	0.8%
November	113.6	1.855	112.4	1.873	1.1%	1.0%
December	94.16	2.414	93.29	2.434	0.9%	0.8%
Average Score					1.7%	1.5%
					0	0
Parameter Group #2						
1-day minimum	2.164	1.2	1.30	1.84	40.0%	53.2%
3-day minimum	2.337	1.153	1.56	1.60	33.4%	38.6%
7-day minimum	2.828	1.081	2.15	1.31	24.0%	21.1%
30-day minimum	7.356	0.8235	6.72	0.86	8.6%	4.1%
90-day minimum	20.65	0.6713	19.49	0.68	5.6%	0.8%
1-day maximum	2775	1.109	2769	1.11	0.2%	0.2%
3-day maximum	2293	1.045	2287	1.05	0.3%	0.2%
7-day maximum	1659	1.01	1653	1.01	0.4%	0.3%
30-day maximum	690.2	0.9501	686	0.96	0.6%	0.5%
90-day maximum	321.9	0.8916	319	0.90	1.0%	0.7%
Average Score					11.4%	12.0%
					0	0
Parameter Group #3						
Date of minimum	180.6	0.2573	174	0.2492	3.7%	3.1%
Date of maximum	198.4	0.2498	200.2	0.253	0.9%	1.3%
Average Score					2.3%	2.2%
					0	0
Parameter Group #4						
Low pulse count	7.213	0.5371	7.75	0.5244	7.4%	2.4%
Low pulse duration	12.74	0.69	12.32	0.6372	3.3%	7.7%
High pulse count	2.925	0.8511	2.925	0.8511	0.0%	0.0%
High pulse duration	5.651	0.6739	5.641	0.6764	0.2%	0.4%
Average Score					2.7%	2.6%
					0	0
Parameter Group #5						
Rise rate	99.08	0.9965	95.94	1.012	3.2%	1.6%
Fall rate	-25.62	-0.8949	-25.82	-0.8924	0.8%	0.3%
Number of reversals	63.75	0.1421	67.9	0.1419	6.5%	0.1%
Average Score					3.5%	0.7%
					0	0
Total Point Classification					0	1
note:					Un-impacted condition	

Table 8.29 allows easy comparison of both analysis results at the four control points in the Sabine River Basin. This table indicates that both the results have considerable similarities.

Table 8.29 A Total of Impact Points on DHRAM Analyses
in the Sabine River Basin

Control Point	Total of Impact Points	
	USGS Recorded Flow (Un-impacted vs. Impacted)	WRAP Simulation Flow (Naturalized vs. Regulated)
BSBS	3	0
SRGW	5	9
SRBE	3	10
SRRL	6	11

Table 8.30 presents both the analysis results at the four control points in the Neches River Basin. This indicates that both results have similar hydrologic alteration characteristic at the four control points expect for NERO. At the control point NERO, both the results have the same impact points on the four parameter groups except for the flow timing parameter group.

Table 8.30 A Total of Impact Points on DHRAM Analyses
in the Neches River Basin

Control Point	Total of Impact Points	
	USGS Recorded Flow (Un-impacted vs. Impacted)	WRAP Simulation Flow (Naturalized vs. Regulated)
NENE	8	6
NERO	0	5
NEEV	10	11
VIKO	4	0

Both the comparisons prove that the regulated flows, computed in the simulation of the daily WRAP model can relatively represent a specific condition of water resources development, regulation and use in the Sabine and Neches River Basins.

Table 8.31 provides both the analysis results at the six control points in the GSA River Basin. The totals of impact points of the USGS recorded flows are totally higher than the WRAP simulation flows. Even though the differences between both the impact points at the control point CP 10 and CP15 are ignorable, both the results are completely different from each other, If both the results of analyses are closely reviewed, the alterations of flow timings on both the results have similar tendency while the alteration of flow amount have totally different tendency.

Table 8.31 A Total of Impact Points on DHRAM
Analyses in the GSA River Basins

Control Point	Total of Impact Points	
	USGS Recorded Flow (Un-impacted vs. Impacted)	WRAP Simulation Flow (Naturalized vs. Regulated)
CP01	9	1
CP02	7	0
CP08	6	0
CP10	2	1
CP15	1	0
CP35	7	0

The average monthly flows and minimum and maximum flows for the impacted periods are considerably greater than un-impacted periods in the USGS recorded flows while there are no evident long-term trends in precipitation in Texas, including the GSA River Basins (Wurbs and Zhang, 2014). These are presumably attributed to return flows from municipal ground water use within the watershed (Wurbs and Zhang, 2014). Although the regulated flows cannot represent the USGS recorded data due to the return flows, the alterations of flow regimes between naturalized and regulated flows can represent the alterations of flow regimes on the GSA River flows by other human influences.

CHAPTER IX

EVALUATING IMPACTS OF THE ENVIRONMENTAL FLOW STANDARDS

The effects of the SB3 environmental flow standards in the Sabine, Neches, and GSA River Basins are evaluated in this chapter based on comparative frequency analyses of WAM naturalized, regulated, and unappropriated flows.

9.1 Impacts on Future Water Rights

SB3 environmental flow standards (EFS) conceptually do not impact existing water rights because the EFS have junior priorities relative to all existing water rights. The environmental flow standards were established to secure the current flow regimes from future water rights. The EFS at a site may influence future water rights up and downstream of sites. However, the flows protected by a EFS at a site can be depleted for water rights at downstream sites.

The daily WRAP model computes sequences of regulated and unappropriated flows based on naturalized flow datasets for the period-of-analysis. Unappropriated flows are naturalized flows still remaining after the streamflow depletions are made and return flows are returned for all the water rights (Wurbs, 2013b). In other words, future water rights can appropriate the unappropriated flows. Therefore, the impacts on future water rights by the EFS can be evaluated through the comparison of unappropriated flows at a control point with and without the EFS using the daily WRAP model.

9.1.1 Sabine River Basin

The Sabine WAM has 18 primary control points. SB3 EFSs were incorporated at five of these control points which are listed in Table 6.2. Table 9.1 presents the unappropriated flow frequency metrics with and without EFS at the control points CFGV, SRWP, and SRMN. There is no EFS influence on the unappropriated flows at the control point CFGV.

The unappropriated flows with EFS slightly decrease than the flows without EFS at the control points SRWP and SRMN.

Table 9.1 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points CFGV, SRWP, and SRMN in the Sabine River Basin

	CFGV		SRWP		SRMN	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	17.8	17.3	170.0	168.0	510.8	487.5
Stan Dev	169.3	164.6	1,156	1,151	1,894	1,876
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	0.0	0.0	0.0
75%	0.0	0.0	0.0	0.0	0.0	0.0
70%	0.0	0.0	0.0	0.0	0.0	0.0
60%	0.0	0.0	0.0	0.0	0.0	0.0
50%	0.0	0.0	0.0	0.0	0.0	0.0
40%	0.0	0.0	0.0	0.0	0.0	0.0
30%	0.0	0.0	0.0	0.0	0.0	0.0
25%	0.0	0.0	0.0	0.0	0.0	0.0
20%	0.0	0.0	0.0	0.0	148.9	43.6
15%	0.0	0.0	0.0	0.0	568.9	433.1
10%	0.0	0.0	0.1	0.0	1,295	1,169
5%	5.2	4.2	253.7	213.4	2,945	2,886
2%	168.0	162.8	2,635	2,600	5,997	5,867
1%	481.5	468.0	5,055	5,042	9,096	9,031
0.50%	994.7	975.1	7,944	7,904	13,030	12,980
Maximum	8,577	8,577	35,453	35,453	34,590	34,590

Table 9.2 provides unappropriated flow frequency metrics with and without EFS at the control points LFQT, BSBS, and SRGW. SB3 EFS was incorporated into the control points BSBS and SRGW. The unappropriated flows with EFS are smaller than the flows without EFS at the three control points. EFS significantly impacts the unappropriated flows at the control point BSBS, but the influence of EFS is much less at the control point LFQT.

Table 9.2 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points LFQT, BSBS, and SRGW in the Sabine River Basin

	LFQT		BSBS		SRGW	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	68.8	65.7	130.8	97.2	1,423	1,202
Stan Dev	452.4	449.1	380.4	304.2	3,971	3,670
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	0.0	0.0	0.0
75%	0.0	0.0	0.0	0.0	0.0	0.0
70%	0.0	0.0	0.0	0.0	0.0	0.0
60%	0.0	0.0	0.0	0.0	0.0	0.0
50%	0.0	0.0	0.0	0.0	0.0	0.0
40%	0.0	0.0	0.0	0.0	0.0	0.0
30%	0.0	0.0	0.0	0.0	0.0	0.0
25%	0.0	0.0	16.9	0.0	577.4	87.0
20%	5.4	0.0	132.5	50.6	1,664	879.0
15%	44.7	32.5	262.6	156.7	2,989	2,114
10%	124.3	112.3	433.4	320.0	4,816	4,029
5%	298.9	285.9	770.2	611.0	8,153	7,307
2%	668.1	655.9	1,319	1,048	13,335	12,402
1%	1,152	1,120	1,757	1,402	18,089	16,961
0.50%	2,063	1,999	2,272	1,836	23,784	22,251
Maximum	22,647	22,647	7,902	6,317	84,596	84,596

Table 9.3 tabulates unappropriated flow frequency metrics with and without EFS at the control points SRBE, MCTT, and MBGR. SB3 EFS was incorporated into the control point SRBE. The unappropriated flows with EFS are smaller than the flows without EFS at the three control points. EFS significantly impacts the unappropriated flows at control point SRBE, but the influence of the EFS is minimal at control points MCTT and MBGR.

Table 9.3 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points SRBE, MCTT, and MBGR in the Sabine River Basin

	SRBE		MCTT		MBGR	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	2,160	1,834	94.1	93.6	105.1	104.4
Stan Dev	5,317	4,974	373.1	372.7	543.1	542.5
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	0.0	0.0	0.0
75%	0.0	0.0	0.0	0.0	0.0	0.0
70%	0.0	0.0	0.0	0.0	0.0	0.0
60%	0.0	0.0	0.0	0.0	0.0	0.0
50%	0.0	0.0	0.0	0.0	0.0	0.0
40%	0.0	0.0	0.0	0.0	0.0	0.0
30%	0.0	0.0	0.0	0.0	0.0	0.0
25%	1,318	250.0	0.0	0.0	1.0	0.8
20%	2,997	1,773	5.2	4.3	4.2	3.8
15%	5,053	3,792	89.8	87.6	18.8	17.1
10%	7,919	6,769	240.3	238.0	118.6	115.9
5%	12,685	11,435	533.3	530.3	548.4	543.0
2%	18,677	17,528	1,138	1,138	1,502	1,497
1%	23,300	22,490	1,758	1,758	2,451	2,443
0.50%	28,847	27,701	2,509	2,509	3,575	3,575
Maximum	99,116	99,440	10,250	10,250	22,270	22,270

Table 9.4 tabulates the unappropriated flow frequency metrics with and without EFS at the control points SRLP, TCSV, and BTTR. The unappropriated flows with EFS are smaller than the flows without EFS at the three control points. The influences by EFS are imperceptible at the control point SRLP, however, EFS considerably reduces the unappropriated flows at the control points TCSV and BTTR.

Table 9.4 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points SRLP, TCSV, and BTTR in the Sabine River Basin

	SRLP		TCSV		BTTR	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	3,033	3,001	101.7	101.2	293.8	284.2
Stan Dev	7,178	7,172	370.3	370.1	969.1	962.5
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	0.0	0.3	0.0
75%	0.0	0.0	0.0	0.0	2.9	0.5
70%	0.0	0.0	0.0	0.0	5.9	3.3
60%	0.0	0.0	0.0	0.0	14.6	11.0
50%	0.0	0.0	0.0	0.0	30.5	26.2
40%	0.0	0.0	0.0	0.0	59.6	53.1
30%	0.0	0.0	0.0	0.0	114.9	105.9
25%	1,752	1,424	10.0	7.8	165.5	152.8
20%	4,439	4,302	49.9	47.8	251.0	234.4
15%	7,458	7,416	113.5	112.0	397.7	374.2
10%	11,534	11,486	238.9	236.6	685.7	655.8
5%	18,044	18,040	589.2	587.2	1,455	1,411
2%	25,981	25,973	1,257	1,254	2,925	2,854
1%	31,457	31,370	1,865	1,865	4,152	4,091
0.50%	38,707	38,707	2,477	2,477	5,825	5,807
Maximum	99,440	99,440	11,230	11,230	28,820	28,820

Table 9.5 provides the unappropriated flow frequency metrics with and without EFS at the control points SRBU, BARP, and SRBW. The unappropriated flows are decreased by EFS at the three control points, but the influences of the EFS are relatively small at all the control points.

Table 9.5 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points SRBU, BARP, and SRBW in the Sabine River Basin

	SRBU		BARP		SRBW	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	5,525	5,474	936.3	901.7	7,276	7,177
Stan Dev	12,451	12,449	2,584	2,572	13,896	13,930
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	5.3	0.0	0.0	0.0
75%	0.0	0.0	45.7	0.0	0.0	0.0
70%	0.0	0.0	76.7	45.8	182.5	32.4
60%	3.0	0.0	142.8	116.1	678.8	462.4
50%	117.6	85.3	242.1	207.9	1,321	1,103
40%	360.5	317.0	395.6	361.2	2,365	2,193
30%	1,247	1,127	633.5	591.3	4,778	4,525
25%	3,210	2,872	826.9	778.1	7,170	7,093
20%	7,572	7,370	1,106	1,054	10,938	10,890
15%	13,329	13,286	1,548	1,494	16,611	16,612
10%	20,901	20,856	2,330	2,267	24,331	24,315
5%	31,924	31,872	4,084	3,988	36,214	36,214
2%	45,656	45,607	7,004	6,939	51,543	51,543
1%	58,435	58,435	9,853	9,834	63,867	63,867
0.50%	71,695	71,695	14,240	14,193	78,537	78,537
Maximum	148,363	148,363	104,595	104,595	187,197	187,197

Table 9.6 presents unappropriated flow frequency metrics with and without EFS at the control points SRRL, CBMV, and SRSL. SB3 EFS was incorporated into the control point SRBE. The unappropriated flows with EFS tremendously decrease than the flows at control point SRRL. However, there are no differences between the flows with and without EFS at the control points CBMV and SRSL.

Table 9.6 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points SRRL, CBMV, and SRSL in the Sabine River Basin

	SRRL		CBMV		SRSL	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	7,609	5,223	227.7	227.8	5,693	5,694
Stan Dev	12,606	8,954	566.2	566.2	9,030	9,031
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	65.5	65.0
85%	0.0	0.0	0.1	0.1	172.3	172.7
80%	143.7	28.7	1.0	1.0	319.0	318.7
75%	412.3	157.7	3.2	3.2	487.4	487.3
70%	708.7	307.0	6.7	6.7	674.4	673.4
60%	1,431	672.8	17.7	17.7	1,197	1,198
50%	2,435	1,314	37.7	37.7	1,955	1,955
40%	4,008	2,474	72.8	72.9	3,191	3,190
30%	6,881	4,698	140.4	140.5	5,397	5,396
25%	9,019	6,344	194.4	194.6	7,020	7,028
20%	12,315	8,715	274.3	274.4	9,314	9,321
15%	16,959	12,089	403.0	403.5	12,634	12,638
10%	22,710	16,030	620.7	622.4	16,554	16,560
5%	32,461	22,797	1,104	1,104	23,366	23,366
2%	46,611	32,831	1,848	1,848	33,402	33,443
1%	57,958	40,645	2,515	2,515	41,169	41,133
0.50%	69,353	48,823	3,459	3,459	49,610	49,610
Maximum	215,512	164,417	16,098	16,098	164,417	164,417

9.1.2 Neches River Basin

The Neches WAM has 20 primary control points. SB3 EFSs were incorporated into the five control points, listed in Table 6.3. Table 9.7 presents the unappropriated flow frequency metrics with and without EFS at the control points KIBR, NEPA, and NENE. SB3 EFS was incorporated into the control point NENE. The unappropriated flows with EFS decrease than the flows without EFS at the three control points. The EFS tremendously decrease the flows at the control point NENE.

Table 9.7 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points KIBR, NEPA, and NENE in the Neches River Basin

	KIBR		NEPA		NENE	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	69.3	67.6	204.7	198.2	536.0	385.0
Stan Dev	439.9	434.7	1,053	1,033	1,678	1,391
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	0.0	0.0	0.0
75%	0.0	0.0	0.0	0.0	0.0	0.0
70%	0.0	0.0	0.0	0.0	0.0	0.0
60%	0.0	0.0	0.0	0.0	0.0	0.0
50%	0.0	0.0	0.0	0.0	0.0	0.0
40%	0.0	0.0	0.0	0.0	0.0	0.0
30%	0.0	0.0	0.0	0.0	99.7	0.0
25%	0.0	0.0	0.0	0.0	278.3	60.5
20%	0.0	0.0	0.0	0.0	495.1	174.8
15%	0.0	0.0	0.0	0.0	817.3	389.0
10%	0.0	0.0	0.0	0.0	1,434	893.8
5%	160.9	133.3	1,094	991.8	3,037	2,355
2%	1,127	1,116	3,586	3,511	5,832	4,782
1%	2,160	2,092	5,625	5,509	8,267	7,034
0.50%	3,074	3,055	7,449	7,386	10,654	8,858
Maximum	12,163	12,163	24,563	24,563	46,537	40,548

Table 9.8 provides the unappropriated flow frequency metrics with and without EFS at the control points NEAL, NEDI, and NERO. SB3 EFS was incorporated into the control point NERO. The unappropriated flows with EFS are smaller than the flows without EFS at the three control points. The EFS considerably decreases flows at the control point NERO.

Table 9.8 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points NEAL, NEDI, and NERO in the Neches River Basin

	NEAL		NEDI		NERO	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	1,207	1,135	2,241	2,072	3,451	2,551
Stan Dev	2,695	2,676	4,363	4,325	6,667	5,241
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	0.0	0.0	0.0
75%	0.0	0.0	0.0	0.0	0.0	0.0
70%	0.0	0.0	0.0	0.0	0.0	0.0
60%	0.0	0.0	0.0	0.0	0.0	0.0
50%	0.0	0.0	0.0	0.0	0.0	0.0
40%	140.9	0.3	760.0	247.2	1,288	122.2
30%	840.3	598.0	1,972	1,443	3,187	1,607
25%	1,269	1,030	2,776	2,267	4,369	2,651
20%	1,868	1,658	3,938	3,521	5,943	4,153
15%	2,663	2,539	5,270	4,983	7,993	6,192
10%	3,832	3,720	7,256	7,085	10,968	9,071
5%	6,171	6,063	10,589	10,466	16,359	13,777
2%	9,557	9,474	15,312	15,196	23,872	19,582
1%	12,544	12,423	19,206	19,126	30,374	24,141
0.50%	15,935	15,859	24,743	24,620	38,389	28,372
Maximum	49,809	49,809	94,643	94,643	99,417	60,202

Table 9.9 presents the unappropriated flow frequency metrics with and without EFS at the control points MUTY, MUJA, and EFACU. The unappropriated flows with EFS are slightly smaller than the flows without EFS at the three control points.

Table 9.9 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points MUTY, MUJA, and EFACU in the Neches River Basin

	MUTY		MUJA		EFACU	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	18.9	18.7	125.1	122.9	157.8	149.6
Stan Dev	128.2	128.1	562.6	561.0	453.9	445.1
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	0.0	0.0	0.0
75%	0.0	0.0	0.0	0.0	0.0	0.0
70%	0.0	0.0	0.0	0.0	0.0	0.0
60%	0.0	0.0	0.0	0.0	0.0	0.0
50%	0.0	0.0	0.0	0.0	0.0	0.0
40%	0.0	0.0	0.0	0.0	0.0	0.0
30%	0.0	0.0	0.0	0.0	27.4	6.0
25%	0.0	0.0	0.0	0.0	71.0	50.5
20%	0.0	0.0	0.0	0.0	139.7	116.1
15%	0.0	0.0	0.0	0.0	255.7	228.2
10%	0.0	0.0	54.4	12.0	466.4	436.9
5%	44.9	43.1	805.4	790.2	950.1	928.2
2%	260.2	258.6	1,941	1,934	1,709	1,678
1%	532.2	532.2	3,096	3,087	2,280	2,262
0.50%	933.2	933.2	3,948	3,948	2,791	2,791
Maximum	3,825	3,825	10,910	10,910	12,278	12,278

Table 9.10 tabulates the unappropriated flow frequency metrics with and without EFS at the control points ANAL, ANLU, and ATCH. SB3 EFS was incorporated into the control point ANAL. The unappropriated flows with EFS are smaller than the flows

without EFS at the three control points. The EFS considerably decrease the flows at control point ANAL.

Table 9.10 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points ANAL, ANLU, and ATCH in the Neches River Basin

	ANAL		ANLU		ATCH	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	963.7	808.3	1,304	1,286	586.8	580.3
Stan Dev	2,525	2,343	3,208	3,199	1,533	1,530
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	0.0	0.0	0.0
75%	0.0	0.0	0.0	0.0	0.0	0.0
70%	0.0	0.0	0.0	0.0	0.0	0.0
60%	0.0	0.0	0.0	0.0	0.0	0.0
50%	0.0	0.0	0.0	0.0	0.0	0.0
40%	0.0	0.0	0.0	0.0	0.0	0.0
30%	153.0	4.8	339.0	240.0	116.3	83.7
25%	587.8	265.5	993.2	912.2	347.2	327.0
20%	1,126	680.6	1,739	1,674	692.6	670.7
15%	1,883	1,342	2,714	2,676	1,142	1,126
10%	3,144	2,473	4,379	4,344	1,904	1,882
5%	5,602	4,922	7,527	7,504	3,441	3,417
2%	9,604	8,927	12,289	12,259	5,845	5,822
1%	12,137	11,625	15,273	15,259	7,571	7,558
0.50%	15,396	15,102	18,649	18,649	9,230	9,222
Maximum	38,281	34,623	55,506	55,506	41,600	41,600

Table 9.11 provides the unappropriated flow frequency metrics with and without EFS at the control points AYSA, ANSR, and NETB. The EFS slightly decrease the unappropriated flows at the three control points, but the changes are small.

Table 9.11 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points AYSA, ANSR, and NETB in the Neches River Basin

	AYSA		ANSR		NETB	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	130.3	129.2	3,050	2,915	6,159	5,742
Stan Dev	435.6	433.3	6,372	6,142	11,509	11,246
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	0.0	0.0	0.0
75%	0.0	0.0	0.0	0.0	0.0	0.0
70%	0.0	0.0	0.0	0.0	0.0	0.0
60%	0.0	0.0	0.0	0.0	0.0	0.0
50%	0.0	0.0	0.0	0.0	37.2	0.0
40%	0.0	0.0	0.0	0.0	1,515	813.5
30%	4.7	3.9	1,635	1,251	5,002	3,384
25%	23.3	21.5	3,089	2,744	7,584	6,299
20%	71.8	68.7	5,062	4,835	10,616	9,906
15%	168.2	165.1	7,387	7,202	14,725	14,275
10%	368.9	363.7	11,045	10,866	21,755	21,268
5%	827.8	825.0	16,947	16,626	31,673	31,073
2%	1,421	1,416	23,967	22,929	42,617	41,699
1%	1,954	1,931	28,445	27,370	49,683	48,079
0.50%	2,565	2,547	32,616	31,289	57,379	54,141
Maximum	12,637	12,637	89,980	89,980	150,680	150,680

Table 9.12 tabulates the unappropriated flow frequency metrics with and without EFS at the control points NEEV, VIKO, and PISL. SB3 EFS was incorporated into the control points NEEV and VIKO. The unappropriated flows are decreased by the EFS at the two control points NEEV and VIKO. The unappropriated flows with and without EFS are identical at control point PISL.

Table 9.12 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points NEEV, VIKO, and PISL in the Neches River Basin

	NEEV		VIKO		PISL	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	6,710	6,037	1,544	1,194	686.8	686.8
Stan Dev	11,574	12,001	3,222	2,811	1,574	1,574
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	0.0	0.0	0.0
75%	0.0	0.0	0.0	0.0	0.0	0.0
70%	0.0	0.0	0.0	0.0	0.0	0.0
60%	0.0	0.0	57.7	0.0	0.0	0.0
50%	609.2	159.6	263.9	48.9	0.0	0.0
40%	2,336	1,145	626.9	246.6	240.8	240.8
30%	6,327	3,343	1,200	698.2	575.0	575.0
25%	9,063	5,877	1,635	1,050	802.8	802.8
20%	12,269	9,308	2,241	1,575	1,090	1,090
15%	16,643	14,048	3,120	2,323	1,515	1,515
10%	23,825	22,441	4,547	3,626	2,072	2,072
5%	32,753	33,718	7,537	6,522	3,186	3,186
2%	40,316	46,286	10,946	9,951	4,817	4,817
1%	44,770	51,908	14,668	13,119	6,439	6,439
0.50%	53,341	56,828	19,746	17,136	7,812	7,812
Maximum	157,127	183,372	64,700	57,550	42,449	42,449

Table 9.13 provides the unappropriated flow frequency metrics with and without EFS at the control points NEBA and NESL. Both the flows at the control points NEBA and NESL are exactly the same..

Table 9.13 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points NEBA and NESL in the Neches River Basin

	NEBA		NESL	
	Without EFS	With EFS	Without EFS	With EFS
Mean	10,406	10,406	12,213	12,213
Stan Dev	17,381	17,381	19,416	19,416
Minimum	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.6	0.6
90%	0.0	0.0	0.9	0.9
85%	0.0	0.0	26.5	26.5
80%	0.0	0.0	121.3	121.3
75%	0.0	0.0	285.6	285.6
70%	0.0	0.0	555.4	555.4
60%	267.4	267.4	1,580	1,580
50%	1,926	1,926	3,388	3,388
40%	4,806	4,806	6,388	6,388
30%	9,922	9,922	11,606	11,606
25%	13,590	13,590	15,267	15,267
20%	18,421	18,421	20,752	20,752
15%	25,038	25,038	27,931	27,931
10%	34,995	34,995	38,801	38,801
5%	48,361	48,361	56,564	56,564
2%	61,643	61,643	72,328	72,328
1%	73,687	73,687	81,939	81,939
0.50%	89,131	89,131	94,265	94,265
Maximum	183,372	183,372	199,611	199,611

9.1.3 GSA River Basins

The GSA WAM has 46 primary control points. SB3 EFSs were incorporated into fifteen control points that include thirteen primary control points and two secondary control points which are tabulated in Table 6.5. The 15 primary control points selected to compare unappropriated flow frequency metrics with and without EFS include 13 primary

control points that have SB3 EFS and two other control points are located at the confluence of the Guadalupe and San Antonio Rivers and the estuary of both rivers.

Table 9.14 presents the unappropriated flow frequency metrics with and without EFS at the control points CP01, CP02, and CP08. The unappropriated flows with EFS are decreased by the EFS at the three control points.

Table 9.14 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points CP01, CP02, and CP08 in the GSA River Basins

	CP01		CP02		CP08	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	42.4	29.3	77.3	50.6	115.5	83.9
Stan Dev	273.4	227.2	453.1	348.5	489.2	413.0
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	0.0	0.0	0.0
75%	0.0	0.0	0.0	0.0	0.0	0.0
70%	0.0	0.0	0.0	0.0	0.0	0.0
60%	0.0	0.0	0.0	0.0	0.0	0.0
50%	0.0	0.0	0.0	0.0	0.0	0.0
40%	0.0	0.0	0.0	0.0	0.0	0.0
30%	0.0	0.0	0.0	0.0	0.0	0.0
25%	0.0	0.0	0.0	0.0	0.0	0.0
20%	0.0	0.0	0.0	0.0	0.0	0.0
15%	0.0	0.0	0.0	0.0	89.4	0.0
10%	0.0	0.0	0.0	0.0	307.5	143.7
5%	74.9	0.0	215.1	0.0	703.5	495.7
2%	779.4	513.4	1,311	910.2	1,323	1,095
1%	1,257	989.9	2,263	1,388	2,022	1,707
0.50%	1,625	1,302	3,134	2,174	2,833	2,427
Maximum	16,263	15,510	16,898	14,970	20,517	20,424

Table 9.15 provides the unappropriated flow frequency metrics with and without EFS at the control points CP10, CP11, and CP13. The unappropriated flows with EFS are smaller than the flows without EFS at the three control points.

Table 9.15 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points CP10, CP11, and CP13 in the GSA River Basins

	CP10		CP11		CP13	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	323.0	208.4	115.8	74.0	215.8	179.7
Stan Dev	1,269	962.0	668.6	460.1	1,137	1,091
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	0.0	0.0	0.0
75%	0.0	0.0	0.0	0.0	0.0	0.0
70%	0.0	0.0	0.0	0.0	0.0	0.0
60%	0.0	0.0	0.0	0.0	0.0	0.0
50%	0.0	0.0	0.0	0.0	1.6	0.0
40%	0.0	0.0	0.0	0.0	9.8	1.9
30%	0.0	0.0	0.0	0.0	31.3	10.6
25%	0.0	0.0	0.0	0.0	53.0	28.1
20%	121.9	42.9	7.8	1.1	91.9	57.0
15%	454.4	165.4	38.3	9.9	160.8	110.1
10%	899.6	456.5	139.0	71.0	332.5	228.3
5%	1,796	1,053	514.3	292.1	905.8	642.8
2%	3,512	2,542	1,486	929.4	2,397	1,989
1%	5,286	3,958	2,555	1,715	4,437	3,972
0.50%	7,854	5,892	3,948	2,960	6,896	6,438
Maximum	71,622	40,412	37,291	20,510	65,183	65,170

Table 9.16 presents unappropriated flow frequency metrics with and without EFS at control points CP14, CP15, and CP28. The unappropriated flows with EFS decrease relative to the flows without EFS at the three control points. However, the unappropriated flow regimes with EFS are slightly improved than the flow regimes without EFS at the control points CP14 and CP15.

Table 9.16 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points CP14, CP15, and CP28 in the GSA River Basins

	CP14		CP15		CP28	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	2,062	1,414	2,389	1,769	124.9	80.8
Stan Dev	5,308	4,606	5,204	4,577	829.2	710.7
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	47.6	0.0	0.0
75%	0.0	0.0	0.0	100.5	0.0	0.0
70%	0.0	0.0	95.9	154.9	0.0	0.0
60%	0.0	29.9	706.1	316.7	0.0	0.0
50%	241.1	121.9	1,159	570.3	0.0	0.0
40%	725.6	252.3	1,416	757.7	0.0	0.0
30%	1,156	383.9	1,639	955.4	0.0	0.0
25%	1,626	511.2	2,084	1,075	0.0	0.0
20%	2,293	714.0	2,471	1,354	0.0	0.0
15%	3,180	1,252	3,305	1,964	6.4	0.0
10%	5,808	3,672	6,059	4,445	204.3	0.0
5%	10,658	8,610	10,629	9,275	554.9	280.8
2%	17,232	14,195	16,943	14,712	1,295	942.2
1%	23,653	20,402	23,801	21,289	2,392	1,710
0.50%	32,627	29,582	33,655	30,452	4,122	2,958
Maximum	102,902	88,752	92,130	81,426	44,209	41,425

Table 9.17 presents the unappropriated flow frequency metrics with and without EFS at the control points CP29, CP32, and CP35. The unappropriated flows with EFS decrease relative to the flows without EFS at the three control points.

Table 9.17 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points CP29, CP32, and CP35 in the GSA River Basins

	CP29		CP32		CP35	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	188.6	114.6	250.8	189.0	166.3	131.9
Stan Dev	1,144.1	811.4	1,147	1,002	685.3	564.6
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.0	0.0	0.0	0.0
99%	0.0	0.0	0.0	0.0	0.0	0.0
98%	0.0	0.0	0.0	0.0	0.0	0.0
95%	0.0	0.0	0.0	0.0	0.0	0.0
90%	0.0	0.0	0.0	0.0	0.0	0.0
85%	0.0	0.0	0.0	0.0	0.0	0.0
80%	0.0	0.0	0.0	0.0	0.0	0.0
75%	0.0	0.0	0.0	0.0	0.0	0.0
70%	0.0	0.0	0.0	0.0	0.0	0.0
60%	0.0	0.0	0.0	0.0	0.0	0.0
50%	0.0	0.0	0.0	0.0	0.0	0.0
40%	0.0	0.0	0.0	0.0	0.2	0.0
30%	0.0	0.0	0.0	0.0	37.8	4.8
25%	0.0	0.0	0.8	0.0	64.5	37.1
20%	0.0	0.0	77.0	0.0	109.0	81.4
15%	54.0	0.0	224.1	68.2	188.0	152.7
10%	272.6	48.4	517.7	292.5	341.7	288.4
5%	793.4	476.6	1,234	918.4	764.3	620.0
2%	2,007	1,369	2,960	2,388	1,696	1,311
1%	4,039	2,546	4,935	4,076	3,033	2,344
0.50%	7,144	4,293	7,211	5,991	4,789	3,815
Maximum	49,210	36,296	38,636	37,551	15,800	12,308

Table 9.18 presents the unappropriated flow frequency metrics with and without EFS at the control points CP37, CP38, and CPEST. The unappropriated flows with EFS are less than the flows without EFS at the control point CP37 like twelve other control points. Both the unappropriated flows with and without EFS are completely identical at the control points CP38 and CPEST. This attributes that the flows, used for EFS are fully returned to their downstream.

Table 9.18 Unappropriated Flow Frequency Metrics Without and With EFS at the Control Points CP37, CP38, and CPEST in the GSA River Basins

	CP37		CP38		CPEST	
	Without EFS	With EFS	Without EFS	With EFS	Without EFS	With EFS
Mean	835.3	612.6	4,665	4,664	4,948	4,948
Stan Dev	2,743.0	2,349.3	8,968	8,967	9,358	9,358
Minimum	0.0	0.0	0.0	0.0	0.0	0.0
99.50%	0.0	0.0	0.5	0.5	1.6	1.6
99%	0.0	0.0	0.5	0.5	13.1	13.1
98%	0.0	0.0	2.0	2.0	24.4	24.5
95%	0.0	0.0	41.4	41.0	76.5	76.9
90%	0.0	0.0	133.5	133.3	194.7	193.1
85%	0.0	0.0	258.2	258.1	341.9	343.0
80%	0.0	0.0	393.9	393.8	495.4	497.2
75%	0.0	0.0	569.8	571.3	738.7	740.4
70%	0.0	0.0	1,007	1,011	1,218	1,217
60%	0.0	0.0	1,757	1,754	1,929	1,925
50%	75.2	0.0	2,278	2,276	2,414	2,414
40%	211.1	38.8	2,481	2,482	2,681	2,687
30%	413.6	151.5	3,574	3,577	3,820	3,827
25%	584.4	296.0	4,611	4,607	4,982	4,981
20%	841.5	511.9	6,094	6,097	6,577	6,584
15%	1,236	880.9	8,233	8,222	8,751	8,744
10%	2,027	1,507	11,406	11,412	12,114	12,100
5%	3,767	2,900	18,092	18,127	18,684	18,676
2%	7,501	6,063	29,185	29,215	29,803	29,730
1%	11,793	9,494	40,265	40,265	42,342	42,341
0.50%	17,167	13,819	57,653	57,654	59,617	59,630
Maximum	100,118	101,251	229,785	229,785	241,981	241,981

9.2 Evaluation of Environmental Flow Standard Effects

SB3 environmental flow standards are assigned priorities based on the date that the BBEST submits its recommendation report to the TCEQ. The SB3 EFS do not affect senior water rights already in existence. The SB3 EFS protect river flow regimes from being adversely affected by future water right permit applicants.

This chapter includes a comparison of the effects of hypothetically assigning the SB3 EFS the most senior priorities in the WAMs versus the most junior priorities. The effects of EFS priorities are evaluated in a river through the comparison of the alterations between naturalized and regulated flows with EFS assigned the most senior and junior priorities. The alterations between both the flows with the different priorities are measured by two different methods, the DHRAM method and annual mean flow duration curve. The control points where SB3 EFSs were incorporated are chosen for the comparison of flow alterations because flow alterations at the control points with EFSs are more apparent than others.

9.2.1 Sabine WAM

The daily Sabine WAM has a priority number of 50000000 for assigning the SB3 EFSs to make them junior to all other water rights in the water right modeling (Wurbs *et al.*, 2014a). The priority number was changed to a priority number of 170000000 for making the assumed Sabine WAM in which SB3 EFSs have a senior priority to all other water rights. This assumed WRAP model for the Sabine WAM that has a senior priority to all other water rights re-computes the regulated flows.

The DHRAM method quantifies the alterations between the naturalized and regulated flows that are computed by the WRAP model at the five control points. In the section 8.1.2, the alterations between the naturalized and regulated flows, computed by the original WRAP model, were quantified by the DHRAM method at the four control points except for the control point 29500. In this section, the DHRAM quantifies both the alterations at the control point 29500. Table 9.19 summarizes the totals of impact points at the five control points to easily compare both the hydrologic alterations between the

naturalized and two different regulated flows. The results of the DHRAM analyses indicate that SB3 EFSs slightly reduce the impacts on the river flows at two of the five sites from existing human influences, but the effects are imperceptible. The results of the DHRAM analyses at the five control points are presented in Tables 9.20 to 9.24.

Table 9.19 A Total of Impact Points on DHRAM Analyses in the Sabine River Basin

Control Point	Total of Impact Points	
	Junior Priority (50000000) (Naturalized vs. Regulated)	Senior Priority (17000000) (Naturalized vs. Regulated)
BSBS	0	0
SRGW	9	9
SRBE	10	9
29500	0	0
SRRL	11	10

The annual median values at the five control points are extracted from the naturalized and two different regulated flows under SB3 EFSs with a junior and a senior priority to all other water rights. The three duration curves are plotted to compare the hydrologic states of each flow for the period 1940-2013 at the five control points.

Table 9.20 DHRAM results at Control Point BSBS (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	BSBS		Period: 1940-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	238.4	0.8167	234.8	0.8375	1.5%	2.5%
February	284.5	0.6652	280.8	0.6857	1.3%	3.1%
March	304	0.8101	299.2	0.8344	1.6%	3.0%
April	286.3	0.883	281.5	0.9012	1.7%	2.1%
May	283.2	0.8972	278.3	0.9212	1.7%	2.7%
June	170.5	0.9918	163.8	1.034	3.9%	4.3%
July	85.66	1.305	79.99	1.386	6.6%	6.2%
August	36.92	0.9991	33.44	1.024	9.4%	2.5%
September	59.45	1.452	55.63	1.54	6.4%	6.1%
October	97.84	1.95	93.47	2.048	4.5%	5.0%
November	155.7	1.174	149	1.232	4.3%	4.9%
December	235.8	0.8424	230.3	0.8685	2.3%	3.1%
Average Score					3.8%	3.8%
					0	0
Parameter Group #2						
1-day minimum	7.218	1.005	6.662	0.9177	7.7%	8.7%
3-day minimum	7.844	0.9778	7.198	0.8905	8.2%	8.9%
7-day minimum	8.934	0.9385	8.085	0.858	9.5%	8.6%
30-day minimum	17.35	0.6844	15.59	0.6467	10.1%	5.5%
90-day minimum	36.33	0.7197	33.07	0.729	9.0%	1.3%
1-day maximum	2413	0.5878	2396	0.5935	0.7%	1.0%
3-day maximum	2137	0.6097	2121	0.6149	0.7%	0.9%
7-day maximum	1505	0.6067	1495	0.6128	0.7%	1.0%
30-day maximum	660.2	0.509	656.1	0.5173	0.6%	1.6%
90-day maximum	400.7	0.4952	396.6	0.5065	1.0%	2.3%
Average Score					4.8%	4.0%
					0	0
Parameter Group #3						
Date of minimum	204.7	0.2539	197.4	0.2411	3.6%	5.0%
Date of maximum	148.4	0.3181	145.1	0.3121	2.2%	1.9%
Average Score					2.9%	3.5%
					0	0
Parameter Group #4						
Low pulse count	8.635	0.5587	9.338	0.4999	8.1%	10.5%
Low pulse duration	11.07	0.5347	9.633	0.4534	13.0%	15.2%
High pulse count	5.676	0.5855	5.527	0.5878	2.6%	0.4%
High pulse duration	4.625	0.3475	4.71	0.3379	1.8%	2.8%
Average Score					6.4%	7.2%
					0	0
Parameter Group #5						
Rise rate	107.8	0.6271	111.8	0.6149	3.7%	1.9%
Fall rate	-36.28	-0.5553	-38.36	-0.5406	5.7%	2.6%
Number of reversals	98.69	0.156	93.09	0.1603	5.7%	2.8%
Average Score					5.0%	2.4%
					0	0
Total Point Classification					0	1
note:					<u>Un-impacted condition</u>	

Table 9.21 DHRAM results at Control Point SRGW (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	SRGW		Period: 1940-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	2425	0.9114	1718	0.8795	29.2%	3.5%
February	2988	0.8102	1996	0.8129	33.2%	0.3%
March	3184	0.8404	2149	0.912	32.5%	8.5%
April	3140	1.137	2233	1.339	28.9%	17.8%
May	3899	1.058	2653	1.096	32.0%	3.6%
June	2442	1.221	1674	1.353	31.4%	10.8%
July	801.1	1.719	514.4	1.659	35.8%	3.5%
August	270	1.56	173.7	1.867	35.7%	19.7%
September	480.3	1.646	289.5	1.563	39.7%	5.0%
October	1184	2.126	670.3	1.976	43.4%	7.1%
November	1758	1.341	1115	1.395	36.6%	4.0%
December	2645	1.058	1749	1.105	33.9%	4.4%
Average Score					34.3%	7.4%
					1	0
Parameter Group #2						
1-day minimum	35.75	1.237	0.08108	5.306	99.8%	328.9%
3-day minimum	37.57	1.22	9.788	1.461	73.9%	19.8%
7-day minimum	41.84	1.189	18.55	1.353	55.7%	13.8%
30-day minimum	82.65	1.085	46.13	1.206	44.2%	11.2%
90-day minimum	247.8	0.9685	152.6	0.9799	38.4%	1.2%
1-day maximum	16970	0.6045	13030	0.7049	23.2%	16.6%
3-day maximum	15940	0.6093	11530	0.7356	27.7%	20.7%
7-day maximum	14330	0.6179	9911	0.7506	30.8%	21.5%
30-day maximum	8461	0.5885	5738	0.6663	32.2%	13.2%
90-day maximum	4929	0.5464	3318	0.603	32.7%	10.4%
Average Score					45.9%	45.7%
					1	0
Parameter Group #3						
Date of minimum	255.4	0.1017	103	0.225	59.7%	121.2%
Date of maximum	162.1	0.3099	162.5	0.3037	0.2%	2.0%
Average Score					30.0%	61.6%
					2	2
Parameter Group #4						
Low pulse count	3.946	0.4854	9.865	0.3835	150.0%	21.0%
Low pulse duration	26.22	0.7151	10.41	0.6772	60.3%	5.3%
High pulse count	2.919	0.6728	4.878	0.6446	67.1%	4.2%
High pulse duration	12.68	0.5818	6.703	0.6257	47.1%	7.5%
Average Score					81.1%	9.5%
					2	0
Parameter Group #5						
Rise rate	330.4	0.5222	398	0.5021	20.5%	3.8%
Fall rate	-194.4	-0.5095	-250.2	-0.4832	28.7%	5.2%
Number of reversals	56.82	0.1528	127.5	0.1249	124.4%	18.3%
Average Score					57.9%	9.1%
					1	0

Total Point 9
Classification 3
note: Moderate risk of impact

Table 9.22 DHRAM results at Control Point SRBE (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	SRBE		Period: 1940-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	3514	0.8648	2588	0.8739	26.4%	1.1%
February	4193	0.7109	3008	0.7388	28.3%	3.9%
March	4257	0.7737	3013	0.8442	29.2%	9.1%
April	4005	1.036	2978	1.248	25.6%	20.5%
May	4904	1.015	3424	1.05	30.2%	3.4%
June	3215	1.085	2314	1.202	28.0%	10.8%
July	1117	1.389	727.5	1.429	34.9%	2.9%
August	424	1.401	272.4	1.677	35.8%	19.7%
September	663.5	1.541	414.4	1.508	37.5%	2.1%
October	1471	2.012	901.9	1.969	38.7%	2.1%
November	2325	1.23	1546	1.267	33.5%	3.0%
December	3384	0.9509	2292	1.006	32.3%	5.8%
Average Score					31.7%	7.0%
					1	0
Parameter Group #2						
1-day minimum	76.84	0.9809	3.135	3.69	95.9%	276.2%
3-day minimum	80.32	0.9697	20.45	1.517	74.5%	56.4%
7-day minimum	88.92	0.9438	38.84	1.068	56.3%	13.2%
30-day minimum	150.3	0.9023	79.26	0.9334	47.3%	3.4%
90-day minimum	399.5	0.9217	245.4	0.9557	38.6%	3.7%
1-day maximum	17110	0.5888	13600	0.6452	20.5%	9.6%
3-day maximum	16510	0.5971	12760	0.6579	22.7%	10.2%
7-day maximum	15230	0.6145	11390	0.6755	25.2%	9.9%
30-day maximum	10060	0.582	7206	0.634	28.4%	8.9%
90-day maximum	6261	0.5175	4451	0.5651	28.9%	9.2%
Average Score					43.8%	40.1%
					1	0
Parameter Group #3						
Date of minimum	252.3	0.1018	146.4	0.2251	42.0%	121.1%
Date of maximum	153.7	0.3076	149.4	0.3003	2.8%	2.4%
Average Score					22.4%	61.7%
					2	2
Parameter Group #4						
Low pulse count	3.527	0.5828	8.189	0.5019	132.2%	13.9%
Low pulse duration	33.34	0.8444	13.17	0.7524	60.5%	10.9%
High pulse count	3.054	0.7085	4.703	0.6931	54.0%	2.2%
High pulse duration	14.47	0.5802	7.974	0.589	44.9%	1.5%
Average Score					72.9%	7.1%
					2	0
Parameter Group #5						
Rise rate	330.4	0.5169	410.3	0.4866	24.2%	5.9%
Fall rate	-202	-0.5119	-275.7	-0.4646	36.5%	9.2%
Number of reversals	56.39	0.1386	119.5	0.1103	111.9%	20.4%
Average Score					57.5%	11.8%
					1	0

Total Point Classification
9
3
note: Moderate risk of impact

Table 9.23 DHRAM results at Control Point 29500 (Naturalized vs. Regulated Flows
under SB3 EFS with Junior and Senior Priority)

Control Point:	29500	Period:	1940-2013			
IHA statistics group	Naturalized		Regulated		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	286.7	0.797	286.7	0.797	0.0%	0.0%
February	305.8	0.7578	305.8	0.7578	0.0%	0.0%
March	261.4	0.6793	261.4	0.6793	0.0%	0.0%
April	256.5	0.7487	256.5	0.7487	0.0%	0.0%
May	176.7	1.08	176.7	1.08	0.0%	0.0%
June	213.4	1.086	213.4	1.086	0.0%	0.0%
July	166.6	1.337	166.6	1.337	0.0%	0.0%
August	103.9	1.131	103.9	1.131	0.0%	0.0%
September	88.77	1.346	88.77	1.346	0.0%	0.0%
October	108.4	1.196	108.4	1.196	0.0%	0.0%
November	133.2	1	133.2	1	0.0%	0.0%
December	221.3	0.8691	221.3	0.8691	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Parameter Group #2						
1-day minimum	5.784	1.167	5.78	1.17	0.0%	0.0%
3-day minimum	6.126	1.149	6.13	1.15	0.0%	0.0%
7-day minimum	6.942	1.14	6.94	1.14	0.0%	0.0%
30-day minimum	17.06	1.082	17.06	1.08	0.0%	0.0%
90-day minimum	55.91	0.7298	55.91	0.73	0.0%	0.0%
1-day maximum	3535	0.8368	3535	0.84	0.0%	0.0%
3-day maximum	2012	0.6841	2012	0.68	0.0%	0.0%
7-day maximum	1219	0.5975	1219	0.60	0.0%	0.0%
30-day maximum	594.7	0.4491	595	0.45	0.0%	0.0%
90-day maximum	370.5	0.4468	371	0.45	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Parameter Group #3						
Date of minimum	195.6	0.2556	195.6	0.2556	0.0%	0.0%
Date of maximum	156.1	0.3029	156.1	0.3029	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Parameter Group #4						
Low pulse count	8	0.5379	8	0.5379	0.0%	0.0%
Low pulse duration	12.49	0.5924	12.49	0.5924	0.0%	0.0%
High pulse count	7.176	0.6061	7.176	0.6061	0.0%	0.0%
High pulse duration	2.999	0.7261	2.999	0.7261	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Parameter Group #5						
Rise rate	186.1	1.025	186.1	1.025	0.0%	0.0%
Fall rate	-53.97	-0.676	-53.97	-0.676	0.0%	0.0%
Number of reversals	107.1	0.1438	107.1	0.1438	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Total Point Classification					0	1
note:					Un-impacted condition	

Table 9.23 (Continued)

Control Point:	29500	Period:	1940-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	286.7	0.797	286.7	0.797	0.0%	0.0%
February	305.8	0.7578	305.8	0.7578	0.0%	0.0%
March	261.4	0.6793	261.4	0.6793	0.0%	0.0%
April	256.5	0.7487	256.5	0.7487	0.0%	0.0%
May	176.7	1.08	176.7	1.08	0.0%	0.0%
June	213.4	1.086	213.4	1.086	0.0%	0.0%
July	166.6	1.337	166.6	1.337	0.0%	0.0%
August	103.9	1.131	103.9	1.131	0.0%	0.0%
September	88.77	1.346	88.77	1.346	0.0%	0.0%
October	108.4	1.196	108.4	1.196	0.0%	0.0%
November	133.2	1	133.2	1	0.0%	0.0%
December	221.3	0.8691	221.3	0.8691	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Parameter Group #2						
1-day minimum	5.784	1.167	5.784	1.167	0.0%	0.0%
3-day minimum	6.126	1.149	6.126	1.149	0.0%	0.0%
7-day minimum	6.942	1.14	6.942	1.14	0.0%	0.0%
30-day minimum	17.06	1.082	17.06	1.082	0.0%	0.0%
90-day minimum	55.91	0.7298	55.91	0.7298	0.0%	0.0%
1-day maximum	3535	0.8368	3535	0.8368	0.0%	0.0%
3-day maximum	2012	0.6841	2012	0.6841	0.0%	0.0%
7-day maximum	1219	0.5975	1219	0.5975	0.0%	0.0%
30-day maximum	594.7	0.4491	594.7	0.4491	0.0%	0.0%
90-day maximum	370.5	0.4468	370.5	0.4468	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Parameter Group #3						
Date of minimum	195.6	0.2556	195.6	0.2556	0.0%	0.0%
Date of maximum	156.1	0.3029	156.1	0.3029	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Parameter Group #4						
Low pulse count	8	0.5379	8	0.5379	0.0%	0.0%
Low pulse duration	12.49	0.5924	12.49	0.5924	0.0%	0.0%
High pulse count	7.176	0.6061	7.176	0.6061	0.0%	0.0%
High pulse duration	2.999	0.7261	2.999	0.7261	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Parameter Group #5						
Rise rate	186.1	1.025	186.1	1.025	0.0%	0.0%
Fall rate	-53.97	-0.676	-53.97	-0.676	0.0%	0.0%
Number of reversals	107.1	0.1438	107.1	0.1438	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Total Point					0	
Classification					1	
note:					Un-impacted condition	

Table 9.24 DHRAM results at Control Point SRRL (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	SRRL	Period:	1940-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	13120	0.7497	8452	1.075	35.6%	43.4%
February	14690	0.578	10070	0.7913	31.4%	36.9%
March	13140	0.6133	9180	0.8016	30.1%	30.7%
April	12510	0.7303	8675	0.9911	30.7%	35.7%
May	11740	0.9706	7587	1.319	35.4%	35.9%
June	8696	0.925	5785	1.279	33.5%	38.3%
July	4722	1.278	3036	1.719	35.7%	34.5%
August	2270	1.13	1417	1.33	37.6%	17.7%
September	2523	1.212	1518	1.435	39.8%	18.4%
October	4069	1.49	2053	1.671	49.5%	12.1%
November	6420	1.156	3302	1.593	48.6%	37.8%
December	10280	0.8322	5936	1.17	42.3%	40.6%
				Average Score	37.5%	31.8%
					1	1
Parameter Group #2						
1-day minimum	646.9	0.74	15.84	3.349	97.6%	352.6%
3-day minimum	660.6	0.7401	78.33	1.405	88.1%	89.8%
7-day minimum	692.8	0.7343	190.4	1.277	72.5%	73.9%
30-day minimum	926.3	0.7261	428.4	0.7776	53.8%	7.1%
90-day minimum	1768	0.7263	884.2	0.71	50.0%	2.2%
1-day maximum	45300	0.5594	36530	0.6521	19.4%	16.6%
3-day maximum	42360	0.5417	33710	0.643	20.4%	18.7%
7-day maximum	37480	0.5058	29130	0.6399	22.3%	26.5%
30-day maximum	25840	0.4848	19520	0.6592	24.5%	36.0%
90-day maximum	17500	0.44	12470	0.634	28.7%	44.1%
				Average Score	47.7%	66.7%
					1	0
Parameter Group #3						
Date of minimum	261.6	0.1064	162	0.2997	38.1%	181.7%
Date of maximum	65.57	0.2011	72.11	0.2139	10.0%	6.4%
				Average Score	24.0%	94.0%
					2	3
Parameter Group #4						
Low pulse count	2.446	0.602	8.203	0.4792	235.4%	20.4%
Low pulse duration	44.17	0.6551	12.05	0.6647	72.7%	1.5%
High pulse count	3.176	0.6961	3.041	0.7413	4.3%	6.5%
High pulse duration	14.4	0.6013	13.65	0.6819	5.2%	13.4%
				Average Score	79.4%	10.4%
					2	0
Parameter Group #5						
Rise rate	947.3	0.5524	866.1	0.5678	8.6%	2.8%
Fall rate	-479.8	-0.5361	-552.6	-0.5083	15.2%	5.2%
Number of reversals	58.31	0.153	87.72	0.1715	50.4%	12.1%
				Average Score	24.7%	6.7%
					0	0
Total Point Classification					10	3
note:					<u>Moderate risk of impact</u>	

Figure 9.1 shows that the three different flows have similar hydrologic states at the control point BSBS. The regulated flow with the senior priority gets more similar to the naturalized flow than the regulated flow with the junior priority, but it is not apparent.

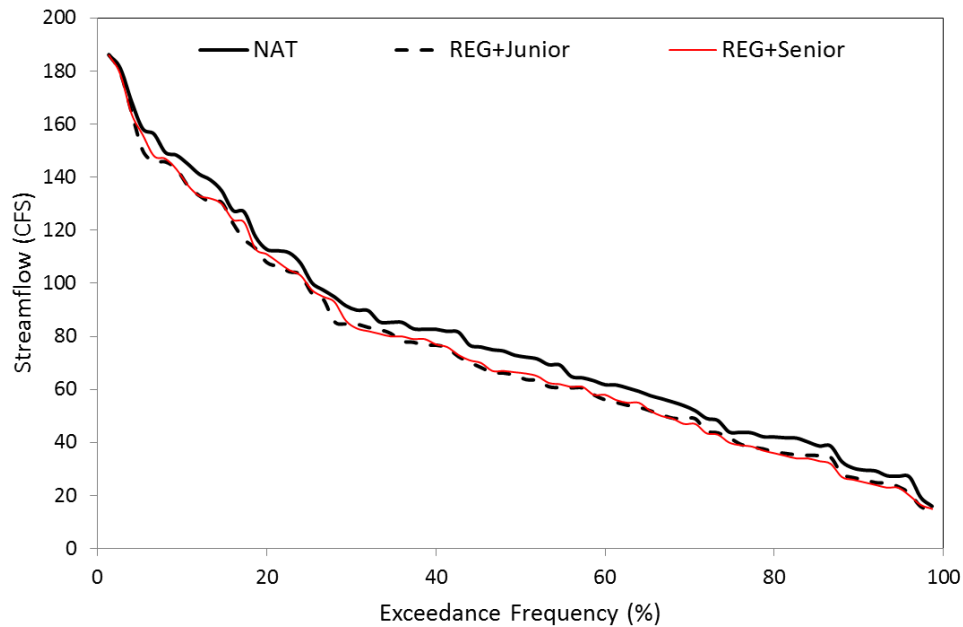


Figure 9.1 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point BSBS

Figure 9.2 presents the hydrologic states of the three different flows at the control point SRGW. The annual median values of regulated flows with the junior or senior priorities decrease than the naturalized flow. The engaged flows by SB3 EFS are based on the regulated flows at the previous day. Thus, if the naturalized flows are bigger than the engaged flows at a control point, the remainder flows after meeting SB EFS are available for other water rights at the control point and other control points. This is the reason why the hydrologic state of the naturalized flow is different from the regulated flow with SB3 EFS.

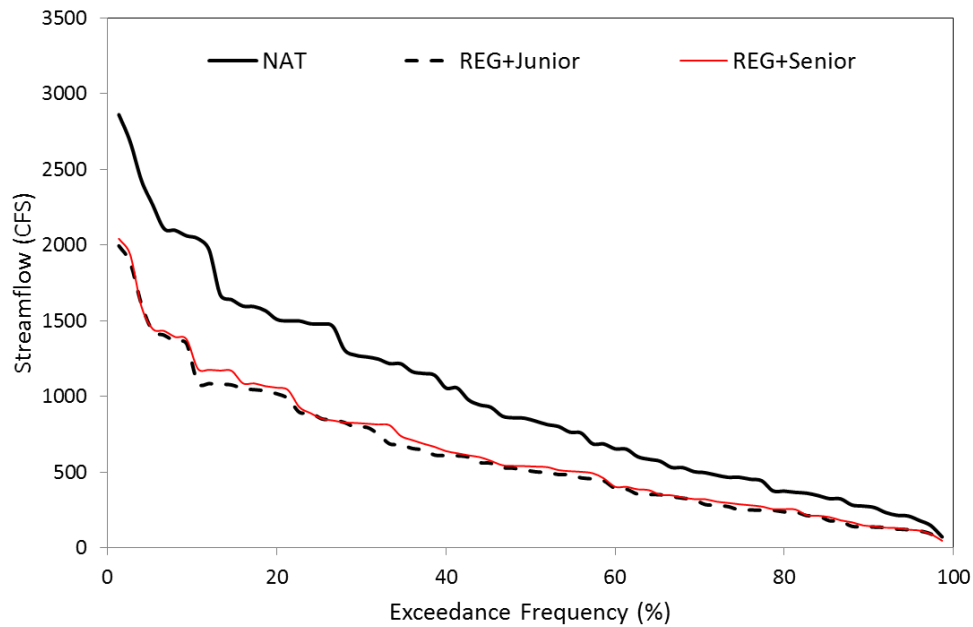


Figure 9.2 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point SRGW

Figure 9.3 shows the hydrologic states of the three different flows at the control point SRBE. The flow duration curves are very similar to the curves at the control points SRGW. The EFS with the senior priority slightly restores the regulated flow to the naturalized flow than with the junior priority, but it is also insignificant.

Figure 9.4 presents the hydrologic states of the three different flows at the control point SRBE. The three flow duration curves are almost same. That means that the river flow at the control point doesn't have serious human influences.

Figure 9.5 exhibits the hydrologic states of the three different flows at the control point SRBE. The hydrologic states of the naturalized flow are totally different from the two different regulated flows, but the two regulated flows are almost identical. This indicates that the effects of SB3 EFS are limited at the control point SRRL.

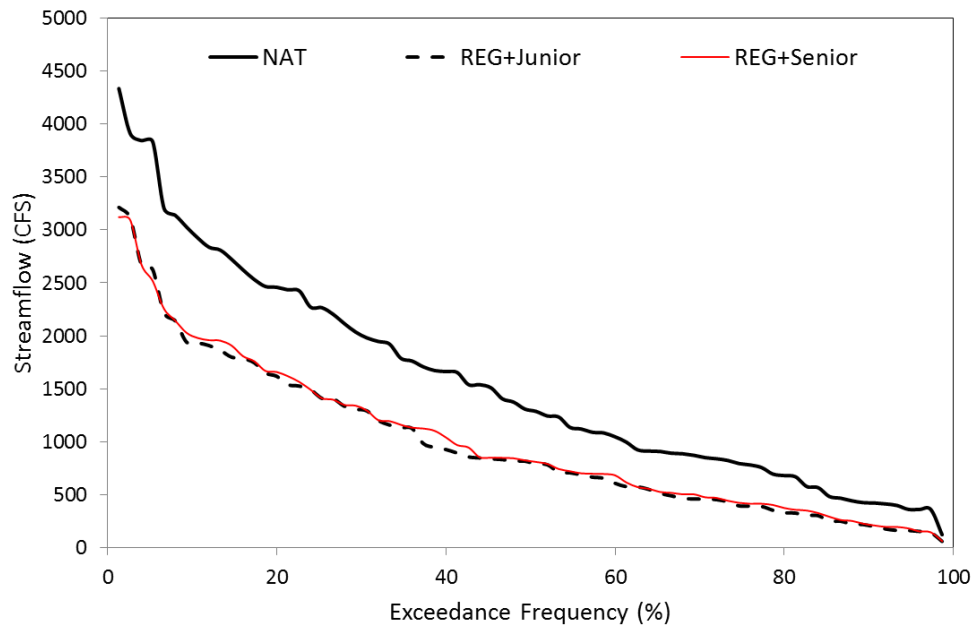


Figure 9.3 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point SRBE

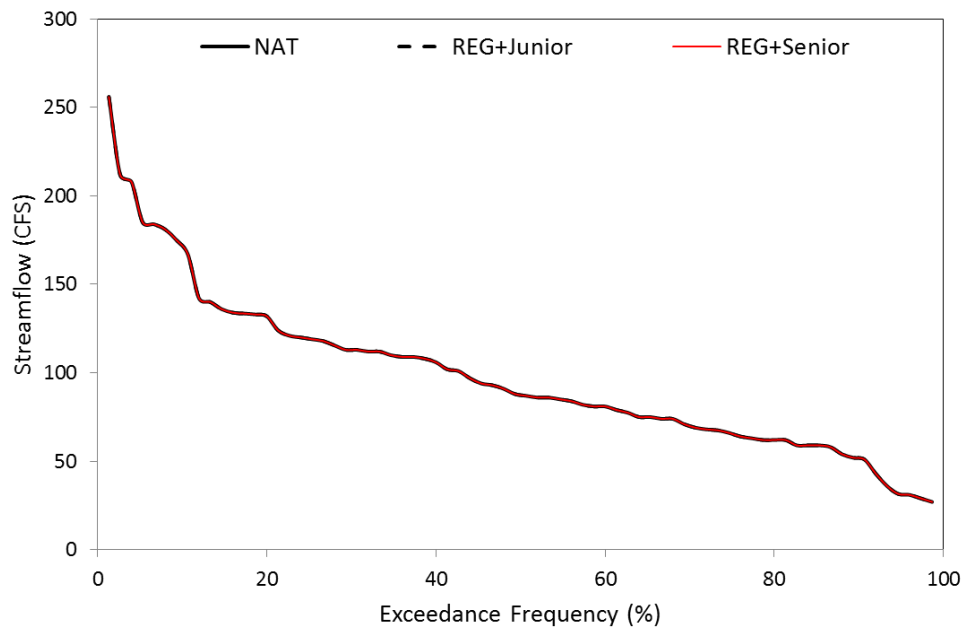


Figure 9.4 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point 29500

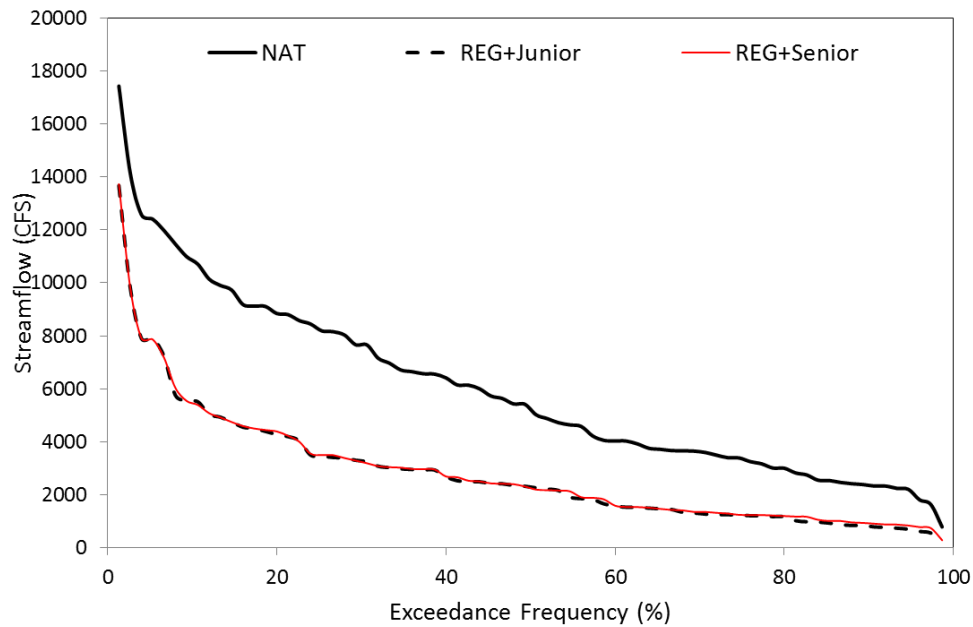


Figure 9.5 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point SRRL

9.2.2 Neches WAM

The Neches WAM has a priority number of 20091130 for assigning the SB3 EFSs reflecting the date that the BBEST recommendations were submitted to the TCEQ. The EFS are junior to all other water rights in WAM (Wurbs *et al.*, 2014b). The priority number was changed to 190000000 for making the hypothetical Neches WAM in which SB3 EFSs have a senior priority to all other water rights. This assumed WRAP model for the Neches WAM, incorporating SB3 EFSs that has a senior priority to all other water rights, re-computes the regulated flows.

The DHRAM method quantifies the alterations between the naturalized and regulated flows that are computed by the assumed WRAP model at the five control points. In the section 8.1.2, the alterations between the naturalized and regulated flows, computed by the original WRAP model, were quantified by the DHRAM method at the four control points except for the control point ANAL. In this section, the DHRAM quantifies both the

alterations at the control point ANAL. Table 9.25 summarizes the totals of impact points at the five controls to easily compare both the hydrologic alterations between the naturalized and two different regulated flows.

The results of the DHRAM analyses indicate that SB3 EFSs slightly reduce the impacts on the river flows at three of the five sites from existing human influences, but the effects are imperceptible. SB3 EFS with a senior priority degenerate the flow regime at the control point NEEV. The results of the DHRAM analyses at the five control points are presented in Tables 9.26 to 9.30.

Table 9.25 A Total of Impact Points on DHRAM Analyses in the Neches River Basin

Control Point	Total of Impact Points	
	Junior Priority (20091130) (Naturalized vs. Regulated)	Senior Priority (19000000) (Naturalized vs. Regulated)
NENE	6	5
NERO	5	4
ANAL	2	1
NEEV	11	12
VIKO	0	0

The annual median values at the five control points are extracted from the naturalized and two different regulated flows under SB3 EFSs with a junior and a senior priority to all other water rights. The three duration curves are plotted to compare the hydrologic states of each flow for the period 1940-2013 at the five control points.

Table 9.26 DHRAM results at Control Point NENE (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	NENE		Period:	1940-2013		
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1015	0.8536	608.3	0.8608	40.1%	0.8%
February	1239	0.7049	737.4	0.7828	40.5%	11.1%
March	1219	0.6986	724.1	0.8992	40.6%	28.7%
April	1155	1.074	768.6	1.231	33.5%	14.6%
May	1209	1.056	670.9	1.19	44.5%	12.7%
June	726.5	1.156	401.1	1.177	44.8%	1.8%
July	236.9	1.836	142.6	1.235	39.8%	32.7%
August	72.4	1.964	77.15	1.277	6.6%	35.0%
September	212.2	1.812	133.2	1.561	37.2%	13.9%
October	446.8	2.142	256.7	1.693	42.5%	21.0%
November	668.5	1.32	359.2	1.28	46.3%	3.0%
December	962.4	0.9138	467.9	0.8749	51.4%	4.3%
Average Score					39.0%	15.0%
					1	0
Parameter Group #2						
1-day minimum	5.162	2.743	4.149	1.355	19.6%	50.6%
3-day minimum	5.946	2.855	10.7	0.7478	80.0%	73.8%
7-day minimum	8.743	3.286	14.28	0.7263	63.3%	77.9%
30-day minimum	24.94	1.804	34.89	0.5556	39.9%	69.2%
90-day minimum	93.09	1.407	76.35	0.8268	18.0%	41.2%
1-day maximum	7681	0.6525	5660	0.7907	26.3%	21.2%
3-day maximum	6887	0.6553	4590	0.7804	33.4%	19.1%
7-day maximum	5969	0.6271	3669	0.754	38.5%	20.2%
30-day maximum	3056	0.5362	1764	0.6708	42.3%	25.1%
90-day maximum	1773	0.5009	1027	0.623	42.1%	24.4%
Average Score					40.3%	42.3%
					0	0
Parameter Group #3						
Date of minimum	221.7	0.1533	178.1	0.3012	19.7%	96.5%
Date of maximum	67.27	0.1753	71.2	0.1797	5.8%	2.5%
Average Score					12.8%	49.5%
					1	1
Parameter Group #4						
Low pulse count	4.324	0.4563	11.74	0.4046	171.5%	11.3%
Low pulse duration	24.74	0.8672	8.626	0.8725	65.1%	0.6%
High pulse count	4.635	0.6191	6.419	0.6301	38.5%	1.8%
High pulse duration	7.569	0.4332	4.327	0.5732	42.8%	32.3%
Average Score					79.5%	11.5%
					2	0
Parameter Group #5						
Rise rate	216.8	0.5456	181.7	0.5273	16.2%	3.4%
Fall rate	-114.8	-0.5323	-121.2	-0.5073	5.6%	4.7%
Number of reversals	52.88	0.2205	111	0.1559	109.9%	29.3%
Average Score					43.9%	12.4%
					0	0

Total Point 5
Classification 3
note: Moderate risk of impact

Table 9.27 DHRAM results at Control Point NERO (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	NERO		Period:	1940-2013		
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	3854	0.8892	3385	0.9184	12.2%	3.3%
February	4158	0.7283	3641	0.7753	12.4%	6.5%
March	3573	0.7663	2999	0.8497	16.1%	10.9%
April	3289	0.9322	2871	0.9747	12.7%	4.6%
May	3709	1.1	3094	1.207	16.6%	9.7%
June	2347	1.105	1958	1.199	16.6%	8.5%
July	1002	1.686	840.4	1.818	16.1%	7.8%
August	305.2	1.549	258.6	1.62	15.3%	4.6%
September	519.9	1.478	401.9	1.561	22.7%	5.6%
October	1194	1.801	982.6	1.849	17.7%	2.7%
November	2181	1.457	1826	1.524	16.3%	4.6%
December	3072	1.081	2544	1.148	17.2%	6.2%
Average Score					16.0%	6.2%
					0	0
Parameter Group #2						
1-day minimum	45.64	1.637	5.216	3.183	88.6%	94.4%
3-day minimum	48.16	1.669	10.66	1.963	77.9%	17.6%
7-day minimum	53.78	1.666	19.87	1.353	63.1%	18.8%
30-day minimum	98.56	1.526	66.11	1.602	32.9%	5.0%
90-day minimum	298.9	1.249	234	1.336	21.7%	7.0%
1-day maximum	17230	0.6091	15690	0.6069	8.9%	0.4%
3-day maximum	15990	0.6048	14450	0.6137	9.6%	1.5%
7-day maximum	14270	0.5882	12690	0.6208	11.1%	5.5%
30-day maximum	8774	0.5577	7662	0.6106	12.7%	9.5%
90-day maximum	5512	0.5299	4738	0.5802	14.0%	9.5%
Average Score					34.0%	16.9%
					0	0
Parameter Group #3						
Date of minimum	247.2	0.1121	180	0.2765	27.2%	146.7%
Date of maximum	148.8	0.3018	150.1	0.2999	0.9%	0.6%
Average Score					14.0%	73.6%
					1	3
Parameter Group #4						
Low pulse count	2.905	0.5227	5.216	0.4667	79.6%	10.7%
Low pulse duration	35.54	0.7146	19.75	0.6956	44.4%	2.7%
High pulse count	3.486	0.7919	3.635	0.7208	4.3%	9.0%
High pulse duration	11.57	0.6183	10.61	0.7109	8.3%	15.0%
Average Score					34.1%	9.3%
					0	0
Parameter Group #5						
Rise rate	442.7	0.6301	416.6	0.6031	5.9%	4.3%
Fall rate	-207.3	-0.5878	-221.2	-0.5595	6.7%	4.8%
Number of reversals	54.27	0.2735	77.03	0.168	41.9%	38.6%
Average Score					18.2%	15.9%
					0	0
Total Point Classification					4	2
note:					<u>Low risk of impact</u>	

Table 9.28 DHRAM results at Control Point ANAL (Naturalized vs. Regulated Flows
under SB3 EFS with Junior and Senior Priority)

Control Point:	ANAL		Period: 1940-2013			
IHA statistics group	Naturalized		Regulated(Junior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1456	0.8634	1158	0.9685	20.5%	12.2%
February	1643	0.6733	1282	0.7664	22.0%	13.8%
March	1487	0.7317	1150	0.827	22.7%	13.0%
April	1285	0.8509	961.1	0.9464	25.2%	11.2%
May	1388	1.089	1085	1.209	21.8%	11.0%
June	783.2	1.122	568.8	1.28	27.4%	14.1%
July	302.2	1.429	207	1.511	31.5%	5.7%
August	110.5	1.238	73.84	1.247	33.2%	0.7%
September	237.8	1.504	150.8	1.521	36.6%	1.1%
October	491.2	1.742	333.4	1.82	32.1%	4.5%
November	883.6	1.365	643.8	1.542	27.1%	13.0%
December	1311	1.086	1010	1.235	23.0%	13.7%
Average Score					26.9%	9.5%
					1	0
Parameter Group #2						
1-day minimum	8.134	1.318	1.368	2.742	83.2%	108.0%
3-day minimum	9.104	1.302	2.637	2.181	71.0%	67.5%
7-day minimum	11.76	1.259	4.672	1.783	60.3%	41.6%
30-day minimum	38.51	1.221	23.04	1.141	40.2%	6.6%
90-day minimum	116.2	1.095	72.18	1.048	37.9%	4.3%
1-day maximum	8426	0.6321	7199	0.6823	14.6%	7.9%
3-day maximum	7841	0.6279	6533	0.6811	16.7%	8.5%
7-day maximum	6895	0.5728	5677	0.6298	17.7%	10.0%
30-day maximum	3606	0.5228	2919	0.5852	19.1%	11.9%
90-day maximum	2144	0.4841	1699	0.5636	20.8%	16.4%
Average Score					38.1%	28.3%
					0	0
Parameter Group #3						
Date of minimum	226.5	0.1689	164.2	0.2571	27.5%	52.2%
Date of maximum	59.08	0.1801	59.36	0.1829	0.5%	1.6%
Average Score					14.0%	26.9%
					1	0
Parameter Group #4						
Low pulse count	4.608	0.4186	6.662	0.3977	44.6%	5.0%
Low pulse duration	20.9	0.6225	13.82	0.5413	33.9%	13.0%
High pulse count	3.649	0.6811	3.784	0.6948	3.7%	2.0%
High pulse duration	10.52	0.4969	9.51	0.5356	9.6%	7.8%
Average Score					22.9%	7.0%
					0	0
Parameter Group #5						
Rise rate	201.9	0.5336	178.1	0.5418	11.8%	1.5%
Fall rate	-110.8	-0.5265	-105.4	-0.5585	4.9%	6.1%
Number of reversals	51.38	0.1397	71.3	0.1552	38.8%	11.1%
Average Score					18.5%	6.2%
					0	0
Total Point Classification					2	2
note:					<u>Low risk of impact</u>	

Table 9.28 (Continued)

Control Point:	ANAL		Period: 1940-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1456	0.8634	1174	0.9477	19.4%	9.8%
February	1643	0.6733	1286	0.7525	21.7%	11.8%
March	1487	0.7317	1137	0.8162	23.5%	11.5%
April	1285	0.8509	985.1	0.9076	23.3%	6.7%
May	1388	1.089	1092	1.188	21.3%	9.1%
June	783.2	1.122	572.3	1.262	26.9%	12.5%
July	302.2	1.429	207.2	1.449	31.4%	1.4%
August	110.5	1.238	75.45	1.209	31.7%	2.3%
September	237.8	1.504	153.5	1.536	35.4%	2.1%
October	491.2	1.742	334	1.654	32.0%	5.1%
November	883.6	1.365	643.1	1.521	27.2%	11.4%
December	1311	1.086	1011	1.211	22.9%	11.5%
Average Score					26.4%	7.9%
					1	0
Parameter Group #2						
1-day minimum	8.134	1.318	2.48	1.744	69.5%	32.3%
3-day minimum	9.104	1.302	4.518	1.413	50.4%	8.5%
7-day minimum	11.76	1.259	7.565	1.175	35.7%	6.7%
30-day minimum	38.51	1.221	25.77	1.003	33.1%	17.9%
90-day minimum	116.2	1.095	75.3	0.9895	35.2%	9.6%
1-day maximum	8426	0.6321	7002	0.6651	16.9%	5.2%
3-day maximum	7841	0.6279	6393	0.6709	18.5%	6.8%
7-day maximum	6895	0.5728	5592	0.6299	18.9%	10.0%
30-day maximum	3606	0.5228	2887	0.5678	19.9%	8.6%
90-day maximum	2144	0.4841	1689	0.5534	21.2%	14.3%
Average Score					31.9%	12.0%
					0	0
Parameter Group #3						
Date of minimum	226.5	0.1689	208.2	0.2057	8.1%	21.8%
Date of maximum	59.08	0.1801	58.11	0.1812	1.6%	0.6%
Average Score					4.9%	11.2%
					0	0
Parameter Group #4						
Low pulse count	4.608	0.4186	7.149	0.5181	55.1%	23.8%
Low pulse duration	20.9	0.6225	14.45	0.6125	30.9%	1.6%
High pulse count	3.649	0.6811	3.649	0.7293	0.0%	7.1%
High pulse duration	10.52	0.4969	9.829	0.51	6.6%	2.6%
Average Score					23.1%	8.8%
					0	0
Parameter Group #5						
Rise rate	201.9	0.5336	170	0.5429	15.8%	1.7%
Fall rate	-110.8	-0.5265	-106	-0.542	4.3%	2.9%
Number of reversals	51.38	0.1397	90.28	0.1974	75.7%	41.3%
Average Score					31.9%	15.3%
					0	0
Total Point					1	
Classification					2	
note:					<u>Low risk of impact</u>	

Table 9.29 DHRAM results at Control Point NEEV (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	NEEV		Period:	1940-2013		
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	9770	0.8516	6370	1.033	34.8%	21.3%
February	10790	0.6577	8180	0.8757	24.2%	33.1%
March	9273	0.7049	7575	0.9579	18.3%	35.9%
April	8424	0.8106	6185	1.018	26.6%	25.6%
May	8684	1.072	5784	1.096	33.4%	2.2%
June	5928	1.051	5074	1.345	14.4%	28.0%
July	2628	1.362	2129	1.564	19.0%	14.8%
August	998.8	1.151	647.3	1.276	35.2%	10.9%
September	1663	1.381	795.3	1.591	52.2%	15.2%
October	3318	1.822	1295	1.608	61.0%	11.7%
November	5588	1.398	3076	1.696	45.0%	21.3%
December	7953	0.9712	5095	1.22	35.9%	25.6%
Average Score					33.3%	20.5%
					1	0
Parameter Group #2						
1-day minimum	200.9	1.223	15.66	3.087	92.2%	152.4%
3-day minimum	209.6	1.22	55.42	1.587	73.6%	30.1%
7-day minimum	227.9	1.201	91.73	1.213	59.7%	1.0%
30-day minimum	395.7	1.223	196.1	1.149	50.4%	6.1%
90-day minimum	900.5	1.015	447.8	0.9976	50.3%	1.7%
1-day maximum	37530	0.652	26500	0.6117	29.4%	6.2%
3-day maximum	35310	0.6293	24940	0.5836	29.4%	7.3%
7-day maximum	32120	0.5985	22930	0.5739	28.6%	4.1%
30-day maximum	20880	0.5489	15800	0.6132	24.3%	11.7%
90-day maximum	13450	0.5012	10030	0.6439	25.4%	28.5%
Average Score					46.3%	24.9%
					1	0
Parameter Group #3						
Date of minimum	259.4	0.1293	70.85	0.2867	72.7%	121.7%
Date of maximum	141.2	0.2954	80.77	0.1839	42.8%	37.7%
Average Score					57.7%	79.7%
					3	3
Parameter Group #4						
Low pulse count	2.514	0.4429	13.04	0.5891	418.7%	33.0%
Low pulse duration	41.43	0.9228	8.854	1.119	78.6%	21.3%
High pulse count	2.608	0.7558	3.081	0.7927	18.1%	4.9%
High pulse duration	16.94	0.6418	15.39	0.7752	9.1%	20.8%
Average Score					131.2%	20.0%
					3	0
Parameter Group #5						
Rise rate	677.1	0.5428	627.1	0.4916	7.4%	9.4%
Fall rate	-386.8	-0.5466	-532.8	-0.6333	37.7%	15.9%
Number of reversals	45.53	0.19	109.4	0.1791	140.3%	5.7%
Average Score					61.8%	10.3%
					1	0
Total Point Classification					12	4
note:					<u>High risk of impact</u>	

Table 9.30 DHRAM results at Control Point VIKO (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	VIKO		Period:	1940-2013		
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1381	0.8871	1381	0.8874	0.0%	0.0%
February	1395	0.7546	1395	0.7549	0.0%	0.0%
March	1046	0.7362	1045	0.7367	0.1%	0.1%
April	959.4	1.103	958.8	1.103	0.1%	0.0%
May	1020	1.172	1020	1.172	0.0%	0.0%
June	827.8	1.456	827.2	1.457	0.1%	0.1%
July	532.5	1.415	531.6	1.416	0.2%	0.1%
August	264.1	1.148	263.4	1.149	0.3%	0.1%
September	398.1	1.492	397	1.494	0.3%	0.1%
October	650.2	2.009	649.6	2.011	0.1%	0.1%
November	968.2	1.409	967.6	1.41	0.1%	0.1%
December	1170	0.9349	1170	0.9355	0.0%	0.1%
Average Score					0.1%	0.1%
					0	0
Parameter Group #2						
1-day minimum	2.135	2.893	2.176	2.849	1.9%	1.5%
3-day minimum	2.428	2.747	2.459	2.693	1.3%	2.0%
7-day minimum	3.714	2.704	3.739	2.666	0.7%	1.4%
30-day minimum	35.5	2.269	35.43	2.274	0.2%	0.2%
90-day minimum	162	1.094	161.4	1.097	0.4%	0.3%
1-day maximum	9326	0.684	9326	0.6841	0.0%	0.0%
3-day maximum	8566	0.6866	8565	0.6867	0.0%	0.0%
7-day maximum	7177	0.6383	7176	0.6384	0.0%	0.0%
30-day maximum	3488	0.6087	3488	0.6088	0.0%	0.0%
90-day maximum	1921	0.5368	1921	0.537	0.0%	0.0%
Average Score					0.4%	0.5%
					0	0
Parameter Group #3						
Date of minimum	200.1	0.2048	201.1	0.2037	0.5%	0.5%
Date of maximum	59.78	0.2275	59.74	0.2275	0.1%	0.0%
Average Score					0.3%	0.3%
					0	0
Parameter Group #4						
Low pulse count	7.014	0.378	7.068	0.3822	0.8%	1.1%
Low pulse duration	13.02	0.3544	13.01	0.3556	0.1%	0.3%
High pulse count	4.595	0.705	4.608	0.7046	0.3%	0.1%
High pulse duration	7.483	0.5295	7.461	0.5294	0.3%	0.0%
Average Score					0.4%	0.4%
					0	0
Parameter Group #5						
Rise rate	305.8	0.5543	305.4	0.5557	0.1%	0.3%
Fall rate	-140.5	-0.5584	-140.5	-0.5584	0.0%	0.0%
Number of reversals	54.68	0.1635	54.76	0.1659	0.1%	1.5%
Average Score					0.1%	0.6%
					0	0
Total Point Classification					0	1
note:					Un-impacted condition	

Figure 9.6 shows the hydrologic states of the three different flows at the control point NENE. The flow duration curves of the regulated flows are considerably different from the naturalized flows. But the EFS with the senior priority slightly improves the flow regimes of the regulated flows relative to with the junior priority at the control point like the result of the DHRAM analysis.

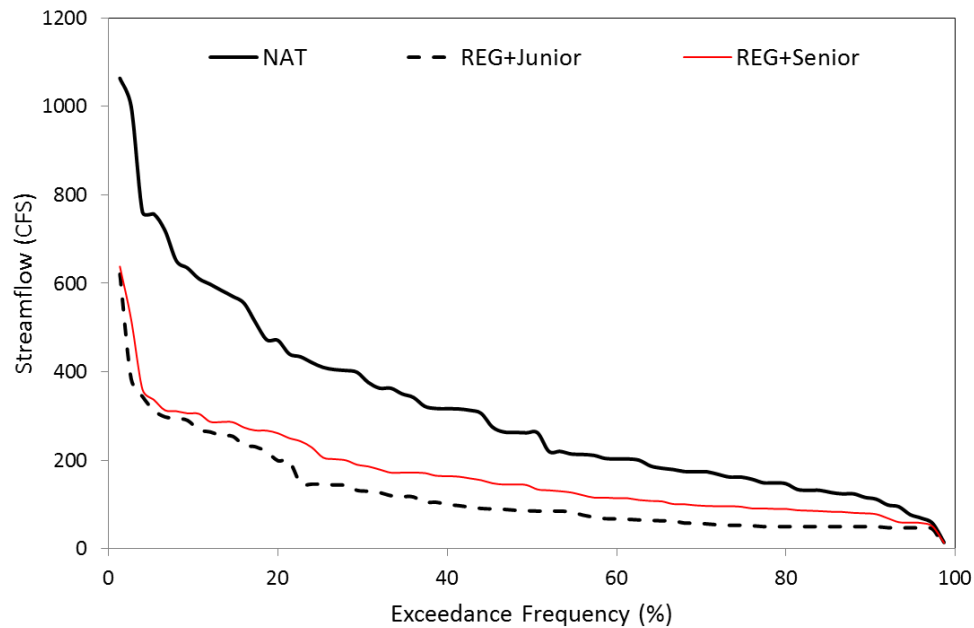


Figure 9.6 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point NENE

Figure 9.7 presents the hydrologic states of the three different flows at the control point NERO. The flow duration curves of the regulated flows are slightly different from the naturalized flows. The regulated flow seems to be slightly improved to the naturalized flow by EFS with the senior priority than with the junior priority, but it is also insignificant.

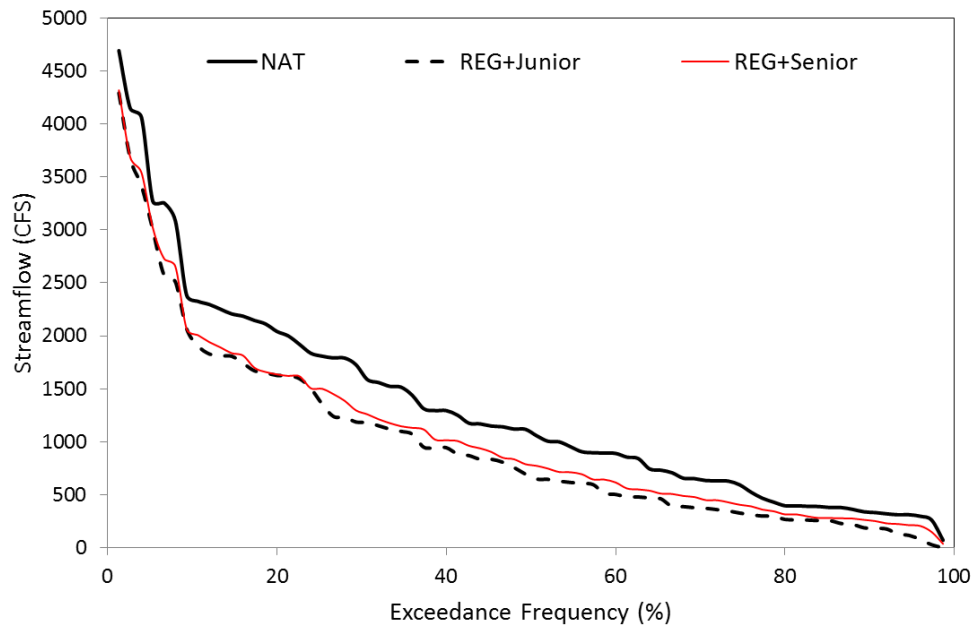


Figure 9.7 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point NERO

Figure 9.8 shows the hydrologic states of the three different flows at the control point ANAL. The flow duration curves of the regulated flows are obviously different from the naturalized flows. The regulated flow with the senior priority get closer to the naturalized flow than the flow with the junior priority, but the difference between both the regulated flows is also insignificant.

Figure 9.9 present the hydrologic states of the three different flows at the control point NEEV. The flow duration curves of both the regulated flows are obviously different from the naturalized flows. However, both duration curves of the regulated flows are almost identical. This indicates that the EFS cannot play the role to relieve the human influences on the regulated flows at the control point NEEV similar to the control point SRRL in the Sabine River Basin.

Figure 9.10 presents the hydrologic states of the three different flows at the control point VIKO. The three flow duration curves are almost identical. This means that there are no serious human influences on the river flow at the control point VIKO.

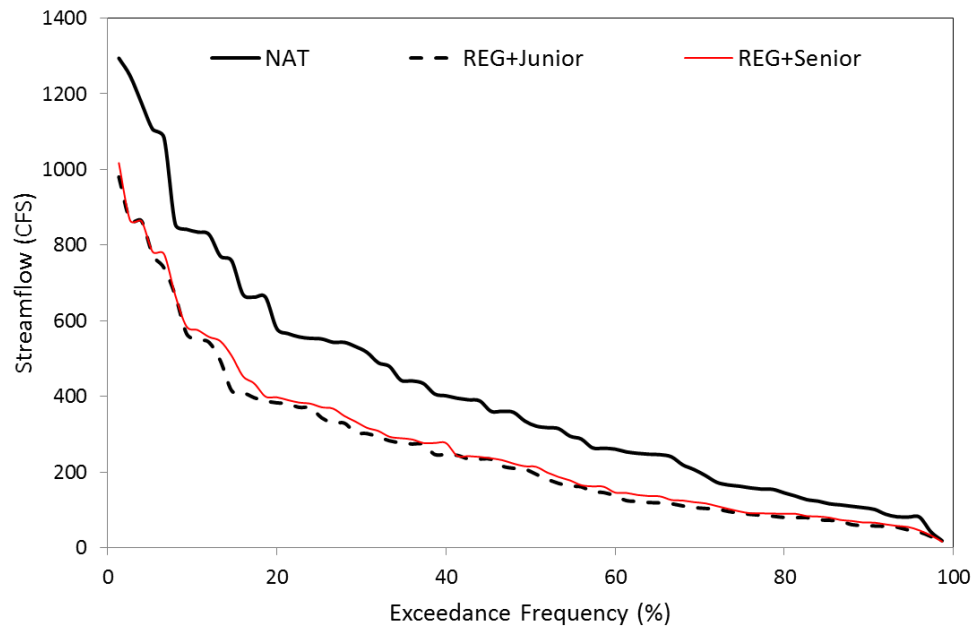


Figure 9.8 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point ANAL

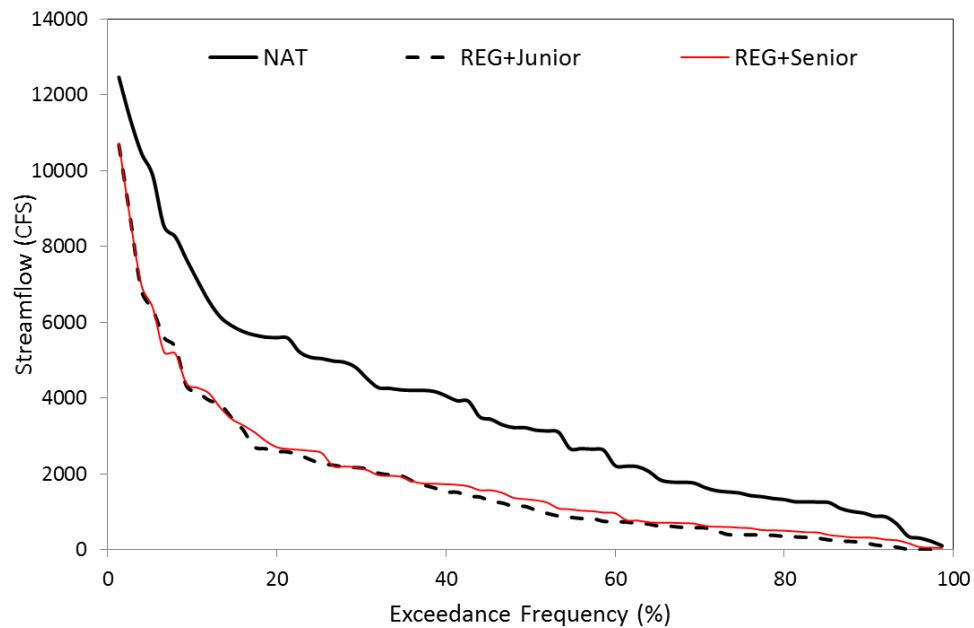


Figure 9.9 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point NEEV

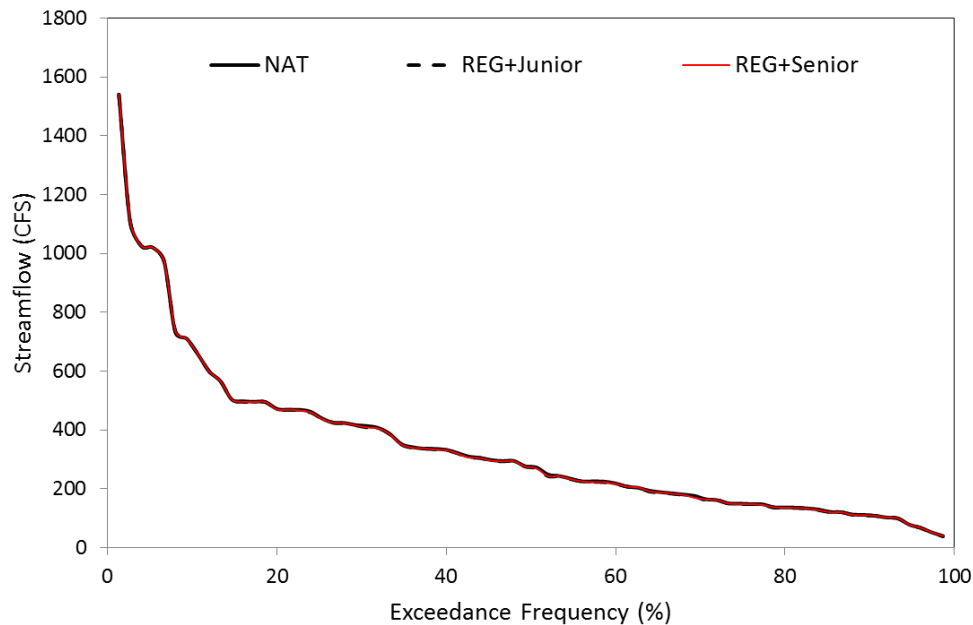


Figure 9.10 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point VIKO

9.2.3 GSA WAM

Daily GSA WAM has a priority number of 88888888 for assigning the SB3 EFSs to make them junior to all other water rights in the water right modeling (Wurbs *et al.*, 2014c). The priority number was changed to a priority number of 190000000 for making the assumed GSA WAM in which SB3 EFSs have a senior priority to all other water rights. This assumed WRAP model for the GSA WAM, incorporating SB3 EFSs that has a senior priority to all other water rights, re-computes the regulated flows.

The DHRAM method quantifies the alterations between the naturalized and regulated flows that are computed by the assumed WRAP model at the fifteen control points. In the section 8.1.2, the alterations between the naturalized and regulated flows, computed by the original WRAP model, were quantified by the DHRAM method at the six control points except for the nine control points CP11, C38461, CP13, CP14, P38241, CP28, CP29, CP32, and CP37. In this section, the DHRAM quantifies both the alterations at the nine control points. Table 9.31 summarizes the totals of impact points at the fifteen

controls to easily compare both the hydrologic alterations between the naturalized and two different regulated flows.

Table 9.31 A Total of Impact Points on DHRAM Analyses in the GSA River Basins

Control Point	Total of Impact Points	
	Junior Priority (88888888) (Naturalized vs. Regulated)	Senior Priority (19000000) (Naturalized vs. Regulated)
CP01	1	0
CP02	0	0
CP08	0	0
CP10	1	0
CP11	0	0
C38461	1	6
CP13	0	0
CP14	0	5
CP15	0	3
P38241	0	0
CP28	3	1
CP29	6	1
CP32	2	1
CP35	0	0
CP37	1	0

The results of the DHRAM analyses indicate that SB3 EFSs slightly reduce the impacts on the river flows at six of the fifteen sites from existing human influences, but degenerate the river flows at the three control points that are located in the Guadalupe River Basin. There are no differences between both the regulated flows at the six control points. This attributes that the six control points don't have obvious human influences, such as water diversion and dam impoundations their upstream. The results of the DHRAM analyses at the five control points are presented in Tables 9.32 to 9.46.

Table 9.32 DHRAM results at Control Point CP01 (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	CP01	Period:	1934-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	157.6	0.8912	147.3	0.9319	6.5%	4.6%
February	189.1	0.9158	177	0.9569	6.4%	4.5%
March	184.1	0.836	171.4	0.8775	6.9%	5.0%
April	203.7	1.121	189	1.189	7.2%	6.1%
May	254.2	1.009	237.6	1.054	6.5%	4.5%
June	295.7	1.72	275.4	1.822	6.9%	5.9%
July	187.2	1.782	169.9	1.898	9.2%	6.5%
August	187.8	3.015	171.2	3.244	8.8%	7.6%
September	212.9	1.601	191.4	1.735	10.1%	8.4%
October	268	1.327	252.3	1.376	5.9%	3.7%
November	185.7	1.389	173.4	1.472	6.6%	6.0%
December	169.9	1.11	159.2	1.17	6.3%	5.4%
Average Score					7.3%	5.7%
					0	0
Parameter Group #2						
1-day minimum	0.5	1.955	0.4688	1.923	6.2%	1.6%
3-day minimum	0.6246	1.885	0.5692	1.812	8.9%	3.9%
7-day minimum	1.18	1.813	1.096	1.721	7.1%	5.1%
30-day minimum	13.06	1.43	11.28	1.337	13.6%	6.5%
90-day minimum	62.3	0.8049	53.87	0.8185	13.5%	1.7%
1-day maximum	6544	1.743	6380	1.772	2.5%	1.7%
3-day maximum	3550	1.501	3456	1.527	2.6%	1.7%
7-day maximum	2102	1.266	2043	1.293	2.8%	2.1%
30-day maximum	836.8	0.9856	804.6	1.015	3.8%	3.0%
90-day maximum	454.3	0.843	431.4	0.8756	5.0%	3.9%
Average Score					6.6%	3.1%
					0	0
Parameter Group #3						
Date of minimum	163.3	0.2483	161.2	0.2404	1.3%	3.2%
Date of maximum	191.8	0.2314	187.4	0.2327	2.3%	0.6%
Average Score					1.8%	1.9%
					0	0
Parameter Group #4						
Low pulse count	7.075	0.4586	7.288	0.4379	3.0%	4.5%
Low pulse duration	12.75	0.4767	12.57	0.4698	1.4%	1.4%
High pulse count	2.238	0.7048	2.213	0.7108	1.1%	0.9%
High pulse duration	3.501	0.8156	3.493	0.8227	0.2%	0.9%
Average Score					1.4%	1.9%
					0	0
Parameter Group #5						
Rise rate	156.7	1.087	132	1.115	15.8%	2.6%
Fall rate	-52.06	-1.003	-54.92	-1.005	5.5%	0.2%
Number of reversals	61.38	0.1761	81.39	0.1952	32.6%	10.8%
Average Score					18.0%	4.5%
					0	0
Total Point Classification					0	1
note:					Un-impacted condition	

Table 9.33 DHRAM results at Control Point CP02 (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	CP02	Period:	1934-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	301	1.141	291.1	1.169	3.3%	2.5%
February	341.7	1.084	330.5	1.112	3.3%	2.6%
March	334	0.9794	322	1.005	3.6%	2.6%
April	357.5	1.129	343.6	1.162	3.9%	2.9%
May	449.1	0.9971	433.6	1.019	3.5%	2.2%
June	549.8	1.747	530.2	1.798	3.6%	2.9%
July	308	1.62	291.4	1.674	5.4%	3.3%
August	253.1	2.392	237.8	2.499	6.0%	4.5%
September	333.5	1.59	313.9	1.661	5.9%	4.5%
October	427.4	1.134	412.7	1.156	3.4%	1.9%
November	329.3	1.439	318.2	1.481	3.4%	2.9%
December	308.7	1.143	299	1.173	3.1%	2.6%
				Average Score	4.0%	3.0%
					0	0
Parameter Group #2						
1-day minimum	2.209	2.178	2.171	2.161	1.7%	0.8%
3-day minimum	2.817	2.008	2.755	1.956	2.2%	2.6%
7-day minimum	4.691	2.033	4.499	1.957	4.1%	3.7%
30-day minimum	30.61	1.118	28.05	1.097	8.4%	1.9%
90-day minimum	98.25	0.8236	90.66	0.8308	7.7%	0.9%
1-day maximum	7820	1.394	7732	1.399	1.1%	0.4%
3-day maximum	4733	1.194	4662	1.2	1.5%	0.5%
7-day maximum	3090	1.024	3043	1.032	1.5%	0.8%
30-day maximum	1375	0.884	1347	0.8973	2.0%	1.5%
90-day maximum	779.1	0.7937	758.3	0.8082	2.7%	1.8%
				Average Score	3.3%	1.5%
					0	0
Parameter Group #3						
Date of minimum	157.6	0.2414	157.1	0.2393	0.3%	0.9%
Date of maximum	187.7	0.251	187.7	0.251	0.0%	0.0%
				Average Score	0.2%	0.4%
					0	0
Parameter Group #4						
Low pulse count	6.463	0.5394	6.788	0.4932	5.0%	8.6%
Low pulse duration	13.49	0.6091	13.09	0.634	3.0%	4.1%
High pulse count	2.675	0.7658	2.713	0.783	1.4%	2.2%
High pulse duration	5.356	0.6773	5.182	0.6925	3.2%	2.2%
				Average Score	3.2%	4.3%
					0	0
Parameter Group #5						
Rise rate	190.4	0.9338	177.7	0.9608	6.7%	2.9%
Fall rate	-66.24	-0.8401	-66.92	-0.8351	1.0%	0.6%
Number of reversals	62.45	0.143	72.91	0.1604	16.7%	12.2%
				Average Score	8.1%	5.2%
					0	0
Total Point					0	
Classification					1	
note:					Un-impacted condition	

Table 9.34 DHRAM results at Control Point CP08 (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	CP08	Period:	1934-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	125.3	1.414	124	1.425	1.0%	0.8%
February	150.7	1.179	149.4	1.189	0.9%	0.8%
March	147.1	1.043	145.7	1.049	1.0%	0.6%
April	154.4	1.179	152.9	1.185	1.0%	0.5%
May	196.4	1.05	194.3	1.057	1.1%	0.7%
June	224.4	1.418	222.2	1.428	1.0%	0.7%
July	116.1	1.735	114.5	1.748	1.4%	0.7%
August	61.9	1.279	60.58	1.291	2.1%	0.9%
September	112	1.629	109.5	1.647	2.2%	1.1%
October	150.4	1.393	148.3	1.408	1.4%	1.1%
November	141.1	1.47	139.8	1.481	0.9%	0.7%
December	122.4	1.441	121.4	1.446	0.8%	0.3%
Average Score					1.2%	0.8%
					0	0
Parameter Group #2						
1-day minimum	2.708	1.002	2.714	1.002	0.2%	0.0%
3-day minimum	3.015	0.974	3.02	0.9737	0.2%	0.0%
7-day minimum	3.816	1.03	3.803	1.02	0.3%	1.0%
30-day minimum	12.56	0.8365	12.4	0.8265	1.3%	1.2%
90-day minimum	38.24	0.7582	37.49	0.7594	2.0%	0.2%
1-day maximum	3545	1.406	3519	1.405	0.7%	0.1%
3-day maximum	2090	1.048	2075	1.048	0.7%	0.0%
7-day maximum	1294	0.885	1286	0.8864	0.6%	0.2%
30-day maximum	563.9	0.7527	559.5	0.7569	0.8%	0.6%
90-day maximum	308.9	0.7148	306.1	0.7193	0.9%	0.6%
Average Score					0.8%	0.4%
					0	0
Parameter Group #3						
Date of minimum	78.81	0.2986	78.81	0.2986	0.0%	0.0%
Date of maximum	187.7	0.2637	187.7	0.2637	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Parameter Group #4						
Low pulse count	7.5	0.5446	7.813	0.5072	4.2%	6.9%
Low pulse duration	11.48	0.5492	11.12	0.5597	3.1%	1.9%
High pulse count	2.75	0.8069	2.738	0.8048	0.4%	0.3%
High pulse duration	5.452	0.6799	5.418	0.6933	0.6%	2.0%
Average Score					2.1%	2.8%
					0	0
Parameter Group #5						
Rise rate	77.11	1.014	74.34	1.011	3.6%	0.3%
Fall rate	-29.96	-0.8709	-30.38	-0.863	1.4%	0.9%
Number of reversals	83.13	0.1457	90.21	0.1435	8.5%	1.5%
Average Score					4.5%	0.9%
					0	0
Total Point					0	
Classification					1	
note:					Un-impacted condition	

Table 9.35 DHRAM results at Control Point CP10 (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	CP10	Period:	1934-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	371.5	1.111	364.8	1.123	1.8%	1.1%
February	399	0.922	392.8	0.9306	1.6%	0.9%
March	339.4	0.7574	333.2	0.7619	1.8%	0.6%
April	410.3	0.9457	403.3	0.9511	1.7%	0.6%
May	498.4	0.9128	489	0.9206	1.9%	0.9%
June	536.5	1.296	525.4	1.313	2.1%	1.3%
July	316.7	1.229	308.1	1.243	2.7%	1.1%
August	199.2	0.6832	192.6	0.6842	3.3%	0.1%
September	299.6	0.9683	291.3	0.9778	2.8%	1.0%
October	443.3	1.387	435.7	1.403	1.7%	1.2%
November	419.9	1.279	414.1	1.29	1.4%	0.9%
December	358.2	0.9849	352.6	0.9923	1.6%	0.8%
				Average Score	2.0%	0.9%
					0	0
Parameter Group #2						
1-day minimum	1.696	2.088	1.441	2.471	15.0%	18.3%
3-day minimum	2.135	2.004	2.027	2.128	5.1%	6.2%
7-day minimum	3.689	1.877	3.763	1.876	2.0%	0.1%
30-day minimum	32.75	0.9026	33.15	0.8768	1.2%	2.9%
90-day minimum	129.5	0.5554	126.3	0.5521	2.5%	0.6%
1-day maximum	7061	1.104	6993	1.112	1.0%	0.7%
3-day maximum	4707	0.8669	4650	0.8749	1.2%	0.9%
7-day maximum	3181	0.7928	3146	0.7991	1.1%	0.8%
30-day maximum	1360	0.7428	1342	0.7495	1.3%	0.9%
90-day maximum	776.3	0.6635	764	0.6688	1.6%	0.8%
				Average Score	3.2%	3.2%
					0	0
Parameter Group #3						
Date of minimum	166.4	0.2556	165.1	0.2493	0.8%	2.5%
Date of maximum	181.2	0.2372	176.6	0.2372	2.5%	0.0%
				Average Score	1.7%	1.2%
					0	0
Parameter Group #4						
Low pulse count	9.113	0.4966	8.95	0.4581	1.8%	7.8%
Low pulse duration	9.845	0.331	9.933	0.3472	0.9%	4.9%
High pulse count	4.913	0.6631	4.913	0.6631	0.0%	0.0%
High pulse duration	3.909	0.4972	3.866	0.5258	1.1%	5.8%
				Average Score	0.9%	4.6%
					0	0
Parameter Group #5						
Rise rate	330.1	0.6384	299.2	0.657	9.4%	2.9%
Fall rate	-79.4	-0.5742	-79.16	-0.5869	0.3%	2.2%
Number of reversals	66.4	0.1356	76.24	0.1251	14.8%	7.7%
				Average Score	8.2%	4.3%
					0	0
Total Point Classification					0	1
note:					<u>Un-impacted condition</u>	

Table 9.36 DHRAM results at Control Point CP11 (Naturalized vs. Regulated Flows under SB3 EFS with Junior and Senior Priority)

Control Point:	CP11		Period: 1934-2013			
IHA statistics group	Naturalized		Regulated (Junior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	91.02	1.737	90.68	1.739	0.4%	0.1%
February	105.5	1.552	105.2	1.553	0.3%	0.1%
March	72.75	1.757	72.26	1.755	0.7%	0.1%
April	147.5	1.686	146.7	1.685	0.5%	0.1%
May	187.4	1.605	186.3	1.609	0.6%	0.2%
June	179.3	1.939	178.4	1.944	0.5%	0.3%
July	75.47	3.604	75	3.621	0.6%	0.5%
August	29.32	3.08	29.02	3.087	1.0%	0.2%
September	73.69	1.696	72.82	1.7	1.2%	0.2%
October	164.5	2.169	163.4	2.175	0.7%	0.3%
November	112.8	2.147	112	2.153	0.7%	0.3%
December	96.09	2.03	95.51	2.026	0.6%	0.2%
Average Score					0.6%	0.2%
					0	0
Parameter Group #2						
1-day minimum	0.000	0.000	0.00	0.00	0.0%	0.0%
3-day minimum	0.000	0.000	0.00	0.00	0.0%	0.0%
7-day minimum	0.001	8.944	0.00	0.00	100.0%	100.0%
30-day minimum	0.580	2.034	0.56	2.06	2.9%	1.2%
90-day minimum	6.944	1.708	6.84	1.71	1.5%	0.2%
1-day maximum	5305	1.224	5286	1.23	0.4%	0.2%
3-day maximum	3202	0.939	3185	0.94	0.5%	0.3%
7-day maximum	1923	0.859	1913	0.86	0.5%	0.3%
30-day maximum	653.3	0.814	650	0.82	0.5%	0.3%
90-day maximum	290.6	0.792	289	0.79	0.6%	0.3%
Average Score					10.7%	10.3%
					0	0
Parameter Group #3						
Date of minimum	67.2	0.1697	65.38	0.1687	2.7%	0.6%
Date of maximum	183.5	0.2629	183.5	0.2629	0.0%	0.0%
Average Score					1.4%	0.3%
					0	0
Parameter Group #4						
Low pulse count	6.738	0.4095	6.963	0.4072	3.3%	0.6%
Low pulse duration	12.14	0.6276	12.16	0.6245	0.2%	0.5%
High pulse count	4.138	0.7474	4.125	0.7503	0.3%	0.4%
High pulse duration	3.055	0.3647	3.064	0.3714	0.3%	1.8%
Average Score					1.0%	0.8%
					0	0
Parameter Group #5						
Rise rate	267.5	0.9594	266.3	0.9694	0.4%	1.0%
Fall rate	-58.79	-0.8423	-58.92	-0.8467	0.2%	0.5%
Number of reversals	59.66	0.2018	59.9	0.204	0.4%	1.1%
Average Score					0.4%	0.9%
					0	0
Total Point Classification					0	1
note:					<u>Un-impacted condition</u>	

Table 9.36 (Continued)

Control Point:	CP11	Period:	1934-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	91.02	1.737	90.12	1.742	1.0%	0.3%
February	105.5	1.552	104.9	1.559	0.6%	0.5%
March	72.75	1.757	72.14	1.765	0.8%	0.5%
April	147.5	1.686	146.7	1.688	0.5%	0.1%
May	187.4	1.605	186.3	1.611	0.6%	0.4%
June	179.3	1.939	178.4	1.946	0.5%	0.4%
July	75.47	3.604	74.97	3.625	0.7%	0.6%
August	29.32	3.08	28.82	3.089	1.7%	0.3%
September	73.69	1.696	72.56	1.705	1.5%	0.5%
October	164.5	2.169	163.6	2.176	0.5%	0.3%
November	112.8	2.147	112.1	2.156	0.6%	0.4%
December	96.09	2.03	95.56	2.036	0.6%	0.3%
Average Score					0.8%	0.4%
					0	0
Parameter Group #2						
1-day minimum	0	0	0	0	0.0%	0.0%
3-day minimum	0	0	0	0	0.0%	0.0%
7-day minimum	0.0008929	8.944	0.0008929	8.944	0.0%	0.0%
30-day minimum	0.5803	2.034	0.5659	2.034	2.5%	0.0%
90-day minimum	6.944	1.708	6.821	1.723	1.8%	0.9%
1-day maximum	5305	1.224	5280	1.23	0.5%	0.5%
3-day maximum	3202	0.9394	3184	0.9441	0.6%	0.5%
7-day maximum	1923	0.8586	1914	0.8622	0.5%	0.4%
30-day maximum	653.3	0.814	650.2	0.8178	0.5%	0.5%
90-day maximum	290.6	0.7923	289	0.7957	0.6%	0.4%
Average Score					0.7%	0.3%
					0	0
Parameter Group #3						
Date of minimum	67.2	0.1697	68.3	0.1692	1.6%	0.3%
Date of maximum	183.5	0.2629	184.1	0.2619	0.3%	0.4%
Average Score					1.0%	0.3%
					0	0
Parameter Group #4						
Low pulse count	6.738	0.4095	6.663	0.4045	1.1%	1.2%
Low pulse duration	12.14	0.6276	12.16	0.62	0.2%	1.2%
High pulse count	4.138	0.7474	4.113	0.7502	0.6%	0.4%
High pulse duration	3.055	0.3647	3.061	0.3827	0.2%	4.9%
Average Score					0.5%	1.9%
					0	0
Parameter Group #5						
Rise rate	267.5	0.9594	266.7	0.9582	0.3%	0.1%
Fall rate	-58.79	-0.8423	-58.72	-0.8466	0.1%	0.5%
Number of reversals	59.66	0.2018	59.86	0.2008	0.3%	0.5%
Average Score					0.3%	0.4%
					0	0
Total Point					0	
Classification					1	
note:					Un-impacted condition	

Table 9.37 DHRAM results at Control Point C3824 (Naturalized vs. Regulated Flows
under SB3 EFS with Junior and Senior Priority)

Control Point:	C3824		Period: 1934-2013			
IHA statistics group	Naturalized		Regulated (Junior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1229	0.9786	1073	0.9733	12.7%	0.5%
February	1317	0.8344	1171	0.8706	11.1%	4.3%
March	1165	0.6849	1028	0.8398	11.8%	22.6%
April	1333	0.8284	1107	0.7941	17.0%	4.1%
May	1727	0.8953	1518	0.8918	12.1%	0.4%
June	1764	1.242	1517	1.09	14.0%	12.2%
July	1148	1.321	1079	1.37	6.0%	3.7%
August	722.1	0.9311	632.3	1.022	12.4%	9.8%
September	1100	0.9068	915.6	0.8789	16.8%	3.1%
October	1505	1.216	1273	1.198	15.4%	1.5%
November	1378	1.11	1246	1.067	9.6%	3.9%
December	1209	0.9573	1103	0.8722	8.8%	8.9%
Average Score					12.3%	6.3%
					0	0
Parameter Group #2						
1-day minimum	31.59	1.447	48.26	0.7568	52.8%	47.7%
3-day minimum	35.08	1.37	54.27	0.8099	54.7%	40.9%
7-day minimum	46.5	1.179	66.24	0.7629	42.5%	35.3%
30-day minimum	192.7	0.8935	224.4	0.754	16.5%	15.6%
90-day minimum	463.8	0.6091	427.3	0.4976	7.9%	18.3%
1-day maximum	12870	0.8557	10070	0.936	21.8%	9.4%
3-day maximum	11220	0.8109	8370	0.8594	25.4%	6.0%
7-day maximum	8846	0.776	6421	0.7775	27.4%	0.2%
30-day maximum	4280	0.7091	3533	0.7049	17.5%	0.6%
90-day maximum	2510	0.6279	2195	0.6887	12.5%	9.7%
Average Score					27.9%	18.4%
					0	0
Parameter Group #3						
Date of minimum	178.3	0.2386	194.3	0.2361	9.0%	1.0%
Date of maximum	179.6	0.2524	171.3	0.2663	4.6%	5.5%
Average Score					6.8%	3.3%
					0	0
Parameter Group #4						
Low pulse count	5.638	0.4979	7.838	0.3102	39.0%	37.7%
Low pulse duration	16.33	0.8308	11.72	0.8984	28.2%	8.1%
High pulse count	3.9	0.7896	6.475	0.7813	66.0%	1.1%
High pulse duration	7.179	0.5002	4.889	0.4371	31.9%	12.6%
Average Score					41.3%	14.9%
					1	0
Parameter Group #5						
Rise rate	415.3	0.6525	339.8	0.6411	18.2%	1.7%
Fall rate	-146	-0.5489	-211	-0.6745	44.5%	22.9%
Number of reversals	52.79	0.1697	86.84	0.2087	64.5%	23.0%
Average Score					42.4%	15.9%
					0	0
Total Point Classification					1	2
note:					<u>Low risk of impact</u>	

Table 9.37 (Continued)

Control Point:	C3824	Period:	1934-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1229	0.9786	1087	0.9795	11.6%	0.1%
February	1317	0.8344	1180	0.878	10.4%	5.2%
March	1165	0.6849	1062	0.8678	8.8%	26.7%
April	1333	0.8284	1162	0.8108	12.8%	2.1%
May	1727	0.8953	1517	0.9053	12.2%	1.1%
June	1764	1.242	1512	1.108	14.3%	10.8%
July	1148	1.321	1072	1.389	6.6%	5.1%
August	722.1	0.9311	642.4	1.079	11.0%	15.9%
September	1100	0.9068	923.6	0.9073	16.0%	0.1%
October	1505	1.216	1267	1.223	15.8%	0.6%
November	1378	1.11	1236	1.105	10.3%	0.5%
December	1209	0.9573	1084	0.93	10.3%	2.9%
Average Score					11.7%	5.9%
					0	0
Parameter Group #2						
1-day minimum	31.59	1.447	11.16	1.433	64.7%	1.0%
3-day minimum	35.08	1.37	18.5	1.323	47.3%	3.4%
7-day minimum	46.5	1.179	36.04	1.198	22.5%	1.6%
30-day minimum	192.7	0.8935	166.4	0.7879	13.6%	11.8%
90-day minimum	463.8	0.6091	390.4	0.5819	15.8%	4.5%
1-day maximum	12870	0.8557	11250	0.7975	12.6%	6.8%
3-day maximum	11220	0.8109	9213	0.7564	17.9%	6.7%
7-day maximum	8846	0.776	6975	0.6966	21.2%	10.2%
30-day maximum	4280	0.7091	3642	0.6717	14.9%	5.3%
90-day maximum	2510	0.6279	2252	0.6772	10.3%	7.9%
Average Score					24.1%	5.9%
					0	0
Parameter Group #3						
Date of minimum	178.3	0.2386	169.6	0.2631	4.9%	10.3%
Date of maximum	179.6	0.2524	167.4	0.2545	6.8%	0.8%
Average Score					5.8%	5.6%
					0	0
Parameter Group #4						
Low pulse count	5.638	0.4979	20.11	0.2543	256.7%	48.9%
Low pulse duration	16.33	0.8308	5.457	1.187	66.6%	42.9%
High pulse count	3.9	0.7896	9.825	0.7155	151.9%	9.4%
High pulse duration	7.179	0.5002	3.671	0.4249	48.9%	15.1%
Average Score					131.0%	29.1%
					3	0
Parameter Group #5						
Rise rate	415.3	0.6525	525.8	0.4602	26.6%	29.5%
Fall rate	-146	-0.5489	-431.5	-0.5083	195.5%	7.4%
Number of reversals	52.79	0.1697	146.2	0.1576	176.9%	7.1%
Average Score					133.0%	14.7%
					3	0
Total Point					6	
Classification					3	
note:					<u>Moderate risk of impact</u>	

Table 9.38 DHRAM results at Control Point CP13 (Naturalized vs. Regulated Flows under SB3 EFS with Junior and Senior Priority)

Control Point:	CP13		Period: 1934-2013			
IHA statistics group	Naturalized		Regulated (Junior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	112.4	1.983	111.6	1.985	0.7%	0.1%
February	111.1	1.914	110.3	1.921	0.7%	0.4%
March	63.93	2.042	63.51	2.049	0.7%	0.3%
April	138.7	1.846	137.8	1.847	0.6%	0.1%
May	201.3	1.719	200.1	1.72	0.6%	0.1%
June	206.7	2.134	205.8	2.138	0.4%	0.2%
July	77.17	3.51	76.81	3.521	0.5%	0.3%
August	33.29	3.22	33.11	3.224	0.5%	0.1%
September	192.6	2.93	191.2	2.936	0.7%	0.2%
October	221.7	2.395	220.4	2.401	0.6%	0.3%
November	121.6	2.479	120.9	2.488	0.6%	0.4%
December	99.78	2.317	99.03	2.323	0.8%	0.3%
Average Score					0.6%	0.2%
					0	0
Parameter Group #2						
1-day minimum	0.0375	3.534	0.04	3.53	0.0%	0.0%
3-day minimum	0.06042	2.531	0.06	2.53	0.0%	0.0%
7-day minimum	0.1286	2.104	0.13	2.12	1.4%	0.6%
30-day minimum	1.437	1.353	1.43	1.35	0.8%	0.1%
90-day minimum	9.875	1.103	9.80	1.10	0.7%	0.2%
1-day maximum	5100	1.452	5080	1.45	0.4%	0.0%
3-day maximum	3436	1.227	3416	1.23	0.6%	0.0%
7-day maximum	2312	1.14	2300	1.14	0.5%	0.1%
30-day maximum	823.1	1.016	819	1.02	0.5%	0.2%
90-day maximum	373.1	0.9611	371	0.96	0.5%	0.2%
Average Score					0.5%	0.1%
					0	0
Parameter Group #3						
Date of minimum	109.7	0.2301	108.9	0.2312	0.7%	0.5%
Date of maximum	185.4	0.264	185.4	0.2639	0.0%	0.0%
Average Score					0.4%	0.3%
					0	0
Parameter Group #4						
Low pulse count	8.588	0.3297	8.6	0.3313	0.1%	0.5%
Low pulse duration	10.3	0.3899	10.31	0.3874	0.1%	0.6%
High pulse count	2.638	0.9917	2.625	0.9948	0.5%	0.3%
High pulse duration	4.849	0.4993	4.841	0.5003	0.2%	0.2%
Average Score					0.2%	0.4%
					0	0
Parameter Group #5						
Rise rate	153.7	1.123	153.5	1.122	0.1%	0.1%
Fall rate	-46.96	-1.053	-46.84	-1.054	0.3%	0.1%
Number of reversals	59.95	0.1762	60.2	0.1778	0.4%	0.9%
Average Score					0.3%	0.4%
					0	0
Total Point					0	
Classification					1	
note:					<u>Un-impacted condition</u>	

Table 9.38 (Continued)

Control Point:	CP13	Period:	1934-2013			
IHA statistics group	Naturalized	Regulated (Senior)		Absolute Chages		
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	112.4	1.983	111.6	1.983	0.7%	0.0%
February	111.1	1.914	110.3	1.919	0.7%	0.3%
March	63.93	2.042	63.51	2.049	0.7%	0.3%
April	138.7	1.846	137.8	1.847	0.6%	0.1%
May	201.3	1.719	200	1.72	0.6%	0.1%
June	206.7	2.134	205.8	2.137	0.4%	0.1%
July	77.17	3.51	76.79	3.518	0.5%	0.2%
August	33.29	3.22	33.1	3.222	0.6%	0.1%
September	192.6	2.93	191.3	2.935	0.7%	0.2%
October	221.7	2.395	220.4	2.4	0.6%	0.2%
November	121.6	2.479	120.9	2.487	0.6%	0.3%
December	99.78	2.317	99.1	2.322	0.7%	0.2%
Average Score					0.6%	0.2%
					0	0
Parameter Group #2						
1-day minimum	0.0375	3.534	0.0375	3.534	0.0%	0.0%
3-day minimum	0.06042	2.531	0.06042	2.531	0.0%	0.0%
7-day minimum	0.1286	2.104	0.1286	2.104	0.0%	0.0%
30-day minimum	1.437	1.353	1.433	1.352	0.3%	0.1%
90-day minimum	9.875	1.103	9.819	1.101	0.6%	0.2%
1-day maximum	5100	1.452	5078	1.453	0.4%	0.1%
3-day maximum	3436	1.227	3414	1.228	0.6%	0.1%
7-day maximum	2312	1.14	2299	1.142	0.6%	0.2%
30-day maximum	823.1	1.016	818.7	1.019	0.5%	0.3%
90-day maximum	373.1	0.9611	371.1	0.9632	0.5%	0.2%
Average Score					0.4%	0.1%
					0	0
Parameter Group #3						
Date of minimum	109.7	0.2301	109.7	0.2301	0.0%	0.0%
Date of maximum	185.4	0.264	185.4	0.2639	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Parameter Group #4						
Low pulse count	8.588	0.3297	8.6	0.3292	0.1%	0.2%
Low pulse duration	10.3	0.3899	10.29	0.3908	0.1%	0.2%
High pulse count	2.638	0.9917	2.65	0.9868	0.5%	0.5%
High pulse duration	4.849	0.4993	4.815	0.5057	0.7%	1.3%
Average Score					0.3%	0.5%
					0	0
Parameter Group #5						
Rise rate	153.7	1.123	153.7	1.128	0.0%	0.4%
Fall rate	-46.96	-1.053	-46.84	-1.055	0.3%	0.2%
Number of reversals	59.95	0.1762	59.96	0.177	0.0%	0.5%
Average Score					0.1%	0.4%
					0	0
Total Point					0	
Classification					1	
note:					Un-impacted condition	

Table 9.39 DHRAM results at Control Point CP14 (Naturalized vs. Regulated Flows under SB3 EFS with Junior and Senior Priority)

Control Point:	CP14		Period: 1934-2013			
IHA statistics group	Naturalized		Regulated (Junior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1546	1.107	1425	1.08	7.8%	2.4%
February	1665	1.003	1549	1	7.0%	0.3%
March	1387	0.7822	1244	0.8533	10.3%	9.1%
April	1681	1.048	1466	1.039	12.8%	0.9%
May	2449	1.088	2197	1.109	10.3%	1.9%
June	2429	1.411	2194	1.328	9.7%	5.9%
July	1487	1.669	1433	1.641	3.6%	1.7%
August	803.3	1.044	742	1.019	7.6%	2.4%
September	1551	1.242	1343	1.281	13.4%	3.1%
October	2085	1.479	1902	1.45	8.8%	2.0%
November	1932	1.333	1828	1.291	5.4%	3.2%
December	1516	1.095	1481	0.9808	2.3%	10.4%
Average Score					8.3%	3.6%
					0	0
Parameter Group #2						
1-day minimum	48.59	1.383	48.69	0.87	0.2%	37.4%
3-day minimum	55.96	1.342	56.87	1.07	1.6%	20.4%
7-day minimum	73.24	1.329	75.03	1.16	2.4%	12.4%
30-day minimum	214.9	1.023	251.50	0.85	17.0%	17.2%
90-day minimum	494.5	0.716	502.70	0.58	1.7%	19.1%
1-day maximum	16910	0.9098	14400	0.96	14.8%	5.7%
3-day maximum	15120	0.8723	12710	0.91	15.9%	4.6%
7-day maximum	12880	0.8619	10860	0.91	15.7%	5.9%
30-day maximum	6357	0.7906	5624	0.82	11.5%	3.4%
90-day maximum	3578	0.7132	3262	0.76	8.8%	6.5%
Average Score					9.0%	13.3%
					0	0
Parameter Group #3						
Date of minimum	205.3	0.2309	193.5	0.2481	5.7%	7.4%
Date of maximum	188.6	0.2587	180.3	0.2632	4.4%	1.7%
Average Score					5.1%	4.6%
					0	0
Parameter Group #4						
Low pulse count	4.6	0.6251	6.65	0.4207	44.6%	32.7%
Low pulse duration	21.77	0.9537	13.9	0.9574	36.2%	0.4%
High pulse count	2.763	0.7965	3.863	0.8765	39.8%	10.0%
High pulse duration	9.087	0.5568	8.2	0.7889	9.8%	41.7%
Average Score					32.6%	21.2%
					0	0
Parameter Group #5						
Rise rate	401.5	0.6933	339.2	0.6779	15.5%	2.2%
Fall rate	-176.1	-0.6437	-212.7	-0.6605	20.8%	2.6%
Number of reversals	51.25	0.1504	86.23	0.1985	68.3%	32.0%
Average Score					34.9%	12.3%
					0	0
Total Point					0	
Classification					1	
note:					Un-impacted condition	

Table 9.39 (Continued)

Control Point:	CP14	Period:	1934-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1546	1.107	1416	1.123	8.4%	1.4%
February	1665	1.003	1545	1.039	7.2%	3.6%
March	1387	0.7822	1289	0.8951	7.1%	14.4%
April	1681	1.048	1528	1.043	9.1%	0.5%
May	2449	1.088	2259	1.104	7.8%	1.5%
June	2429	1.411	2204	1.34	9.3%	5.0%
July	1487	1.669	1424	1.703	4.2%	2.0%
August	803.3	1.044	732.1	1.173	8.9%	12.4%
September	1551	1.242	1388	1.324	10.5%	6.6%
October	2085	1.479	1873	1.51	10.2%	2.1%
November	1932	1.333	1798	1.345	6.9%	0.9%
December	1516	1.095	1415	1.07	6.7%	2.3%
Average Score					8.0%	4.4%
					0	0
Parameter Group #2						
1-day minimum	48.59	1.383	16.65	1.248	65.7%	9.8%
3-day minimum	55.96	1.342	27.3	1.186	51.2%	11.6%
7-day minimum	73.24	1.329	59.82	1.238	18.3%	6.8%
30-day minimum	214.9	1.023	189	1.008	12.1%	1.5%
90-day minimum	494.5	0.716	432.9	0.7229	12.5%	1.0%
1-day maximum	16910	0.9098	15490	0.9263	8.4%	1.8%
3-day maximum	15120	0.8723	13510	0.8708	10.6%	0.2%
7-day maximum	12880	0.8619	11420	0.8655	11.3%	0.4%
30-day maximum	6357	0.7906	5806	0.7836	8.7%	0.9%
90-day maximum	3578	0.7132	3347	0.7429	6.5%	4.2%
Average Score					20.5%	3.8%
					0	0
Parameter Group #3						
Date of minimum	205.3	0.2309	174.7	0.2751	14.9%	19.1%
Date of maximum	188.6	0.2587	180.2	0.2563	4.5%	0.9%
Average Score					9.7%	10.0%
					1	0
Parameter Group #4						
Low pulse count	4.6	0.6251	13.04	0.4184	183.5%	33.1%
Low pulse duration	21.77	0.9537	9.382	1.33	56.9%	39.5%
High pulse count	2.763	0.7965	4.063	0.8312	47.1%	4.4%
High pulse duration	9.087	0.5568	8.045	0.773	11.5%	38.8%
Average Score					74.7%	28.9%
					2	0
Parameter Group #5						
Rise rate	401.5	0.6933	466.8	0.5262	16.3%	24.1%
Fall rate	-176.1	-0.6437	-336.4	-0.5148	91.0%	20.0%
Number of reversals	51.25	0.1504	128.2	0.1629	150.1%	8.3%
Average Score					85.8%	17.5%
					2	0
Total Point					5	
Classification					3	
note:					Moderate risk of impact	

Table 9.40 DHRAM results at Control Point CP15 (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	CP15	Period:	1934-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	1599	1.104	1443	1.147	9.8%	3.9%
February	1703	1.001	1568	1.048	7.9%	4.7%
March	1445	0.7707	1324	0.8797	8.4%	14.1%
April	1742	1.006	1570	1.015	9.9%	0.9%
May	2516	1.059	2297	1.088	8.7%	2.7%
June	2544	1.428	2292	1.386	9.9%	2.9%
July	1575	1.616	1479	1.684	6.1%	4.2%
August	855	1.033	754.3	1.203	11.8%	16.5%
September	1585	1.216	1398	1.311	11.8%	7.8%
October	2124	1.417	1900	1.461	10.5%	3.1%
November	1989	1.305	1838	1.333	7.6%	2.1%
December	1565	1.067	1446	1.065	7.6%	0.2%
Average Score					9.2%	5.3%
					0	0
Parameter Group #2						
1-day minimum	59.18	1.461	12.03	1.246	79.7%	14.7%
3-day minimum	67.62	1.421	19.01	1.132	71.9%	20.3%
7-day minimum	86	1.378	55.46	1.263	35.5%	8.3%
30-day minimum	238.1	0.9643	194.4	0.9696	18.4%	0.5%
90-day minimum	535.8	0.6979	451.4	0.7204	15.8%	3.2%
1-day maximum	15310	0.8621	14060	0.8467	8.2%	1.8%
3-day maximum	14200	0.8593	12910	0.8446	9.1%	1.7%
7-day maximum	12460	0.8599	11190	0.8541	10.2%	0.7%
30-day maximum	6431	0.8038	5898	0.7999	8.3%	0.5%
90-day maximum	3657	0.7115	3413	0.7464	6.7%	4.9%
Average Score					26.4%	5.7%
					0	0
Parameter Group #3						
Date of minimum	197.3	0.2327	177.4	0.2783	10.1%	19.6%
Date of maximum	187.8	0.2567	182.7	0.2567	2.7%	0.0%
Average Score					6.4%	9.8%
					0	0
Parameter Group #4						
Low pulse count	5.025	0.6041	15.03	0.3324	199.1%	45.0%
Low pulse duration	20.01	1.01	6.91	0.9899	65.5%	2.0%
High pulse count	2.75	0.7923	4.313	0.8247	56.8%	4.1%
High pulse duration	9.291	0.5614	7.628	0.8483	17.9%	51.1%
Average Score					84.8%	25.5%
					2	0
Parameter Group #5						
Rise rate	373.8	0.6418	467.6	0.4761	25.1%	25.8%
Fall rate	-168.3	-0.6072	-326.5	-0.4774	94.0%	21.4%
Number of reversals	53.58	0.1281	118.2	0.1346	120.6%	5.1%
Average Score					79.9%	17.4%
					1	0
Total Point Classification					3	2
note:					<u>Low risk of impact</u>	

Table 9.41 DHRAM results at Control Point P38241 (Naturalized vs. Regulated Flows
under SB3 EFS with Junior and Senior Priority)

Control Point:	P3824		Period: 1934-2013			
IHA statistics group	Naturalized		Regulated (Junior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	79.87	1.334	79.87	1.334	0.0%	0.0%
February	88.76	1.141	88.64	1.135	0.1%	0.5%
March	89.55	1.008	89.33	1.004	0.2%	0.4%
April	100.4	1.12	100.3	1.118	0.1%	0.2%
May	130.9	0.9941	130.7	0.9932	0.2%	0.1%
June	164.2	1.789	163.8	1.783	0.2%	0.3%
July	119.1	2.09	118.8	2.085	0.3%	0.2%
August	106.1	2.402	105.9	2.403	0.2%	0.0%
September	98.33	1.622	98.2	1.616	0.1%	0.4%
October	124.6	1.302	124.3	1.295	0.2%	0.5%
November	94.91	1.238	94.79	1.238	0.1%	0.0%
December	77.88	1.141	77.84	1.141	0.1%	0.0%
Average Score					0.2%	0.2%
					0	0
Parameter Group #2						
1-day minimum	3.278	1.322	3.28	1.32	0.0%	0.0%
3-day minimum	3.51	1.301	3.51	1.30	0.0%	0.0%
7-day minimum	4.29	1.243	4.29	1.24	0.0%	0.0%
30-day minimum	11.67	0.9616	11.67	0.96	0.0%	0.0%
90-day minimum	27.76	0.9195	27.76	0.92	0.0%	0.0%
1-day maximum	4346	1.049	4339	1.05	0.2%	0.2%
3-day maximum	1837	1.027	1835	1.03	0.1%	0.2%
7-day maximum	1008	1.023	1006	1.02	0.2%	0.2%
30-day maximum	429.1	0.9627	428	0.96	0.2%	0.2%
90-day maximum	235.7	0.8723	235	0.87	0.2%	0.2%
Average Score					0.1%	0.1%
					0	0
Parameter Group #3						
Date of minimum	167.6	0.2877	167.6	0.2877	0.0%	0.0%
Date of maximum	185.8	0.26	185.8	0.26	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Parameter Group #4						
Low pulse count	5.125	0.5124	5.125	0.5124	0.0%	0.0%
Low pulse duration	19.47	1.101	19.47	1.101	0.0%	0.0%
High pulse count	4.613	0.6989	4.625	0.6978	0.3%	0.2%
High pulse duration	1.767	0.9643	1.755	0.9532	0.7%	1.2%
Average Score					0.2%	0.3%
					0	0
Parameter Group #5						
Rise rate	214.4	0.9197	213.5	0.915	0.4%	0.5%
Fall rate	-40.44	-0.877	-40.4	-0.8766	0.1%	0.0%
Number of reversals	52.2	0.2098	52.45	0.2118	0.5%	1.0%
Average Score					0.3%	0.5%
					0	0
Total Point					0	
Classification					1	
note:					<u>Un-impacted condition</u>	

Table 9.41 (Continued)

Control Point:	P3824	Period:	1934-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	79.87	1.334	79.73	1.335	0.2%	0.1%
February	88.76	1.141	88.5	1.14	0.3%	0.1%
March	89.55	1.008	89.38	1.007	0.2%	0.1%
April	100.4	1.12	99.99	1.116	0.4%	0.4%
May	130.9	0.9941	130.5	0.9935	0.3%	0.1%
June	164.2	1.789	163.8	1.786	0.2%	0.2%
July	119.1	2.09	118.9	2.093	0.2%	0.1%
August	106.1	2.402	105.9	2.401	0.2%	0.0%
September	98.33	1.622	98	1.621	0.3%	0.1%
October	124.6	1.302	124.4	1.302	0.2%	0.0%
November	94.91	1.238	94.71	1.237	0.2%	0.1%
December	77.88	1.141	77.8	1.141	0.1%	0.0%
Average Score					0.2%	0.1%
					0	0
Parameter Group #2						
1-day minimum	3.278	1.322	3.278	1.322	0.0%	0.0%
3-day minimum	3.51	1.301	3.51	1.301	0.0%	0.0%
7-day minimum	4.29	1.243	4.29	1.243	0.0%	0.0%
30-day minimum	11.67	0.9616	11.67	0.9616	0.0%	0.0%
90-day minimum	27.76	0.9195	27.74	0.9188	0.1%	0.1%
1-day maximum	4346	1.049	4337	1.051	0.2%	0.2%
3-day maximum	1837	1.027	1833	1.029	0.2%	0.2%
7-day maximum	1008	1.023	1005	1.024	0.3%	0.1%
30-day maximum	429.1	0.9627	428	0.9628	0.3%	0.0%
90-day maximum	235.7	0.8723	235.1	0.8725	0.3%	0.0%
Average Score					0.1%	0.1%
					0	0
Parameter Group #3						
Date of minimum	167.6	0.2877	167.6	0.2877	0.0%	0.0%
Date of maximum	185.8	0.26	185.8	0.26	0.0%	0.0%
Average Score					0.0%	0.0%
					0	0
Parameter Group #4						
Low pulse count	5.125	0.5124	5.138	0.5152	0.3%	0.5%
Low pulse duration	19.47	1.101	19.44	1.102	0.2%	0.1%
High pulse count	4.613	0.6989	4.613	0.6989	0.0%	0.0%
High pulse duration	1.767	0.9643	1.764	0.9663	0.2%	0.2%
Average Score					0.1%	0.2%
					0	0
Parameter Group #5						
Rise rate	214.4	0.9197	208.1	0.9156	2.9%	0.4%
Fall rate	-40.44	-0.877	-40.6	-0.8778	0.4%	0.1%
Number of reversals	52.2	0.2098	54.76	0.2238	4.9%	6.7%
Average Score					2.7%	2.4%
					0	0
Total Point					0	
Classification					1	
note:					Un-impacted condition	

Table 9.42 DHRAM results at Control Point CP28 (Naturalized vs. Regulated Flows under SB3 EFS with Junior and Senior Priority)

Control Point:	CP28		Period: 1934-2013			
IHA statistics group	Naturalized		Regulated (Junior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	202.5	1.375	119.1	1.3	41.2%	5.5%
February	216.5	1.106	128	1.127	40.9%	1.9%
March	194.9	0.928	107.6	0.8855	44.8%	4.6%
April	235.4	1.156	138.2	1.309	41.3%	13.2%
May	299.9	1.038	167.6	1.133	44.1%	9.2%
June	410.9	2.149	273.4	2.853	33.5%	32.8%
July	272.2	2.104	157.7	2.061	42.1%	2.0%
August	213.6	1.914	131	1.71	38.7%	10.7%
September	260.9	1.529	170.4	1.687	34.7%	10.3%
October	307.1	1.299	200	1.669	34.9%	28.5%
November	242.8	1.353	145.8	1.498	40.0%	10.7%
December	204	1.341	125.6	1.384	38.4%	3.2%
Average Score					39.5%	11.0%
					1	0
Parameter Group #2						
1-day minimum	18.11	1.027	3.63	1.52	80.0%	48.4%
3-day minimum	19.07	1.006	9.03	1.20	52.6%	19.5%
7-day minimum	21.18	0.9715	13.96	1.05	34.1%	8.4%
30-day minimum	37.68	0.844	30.04	0.71	20.3%	15.3%
90-day minimum	73.36	0.7922	50.53	0.60	31.1%	24.0%
1-day maximum	5041	0.9953	3611	1.13	28.4%	13.9%
3-day maximum	3533	1.009	2281	1.25	35.4%	23.5%
7-day maximum	2414	1.064	1581	1.28	34.5%	20.0%
30-day maximum	1010	1.093	656	1.39	35.0%	27.0%
90-day maximum	554.8	0.9184	340	1.14	38.7%	24.1%
Average Score					39.0%	22.4%
					0	0
Parameter Group #3						
Date of minimum	195.4	0.2762	155.8	0.238	20.3%	13.8%
Date of maximum	174.7	0.2499	184.7	0.2659	5.7%	6.4%
Average Score					13.0%	10.1%
					1	0
Parameter Group #4						
Low pulse count	4.488	0.7162	7.9	0.5145	76.0%	28.2%
Low pulse duration	21.36	0.7974	10.54	0.9908	50.7%	24.3%
High pulse count	4.338	0.8413	4.425	0.8981	2.0%	6.8%
High pulse duration	3.118	0.6602	2.57	0.8257	17.6%	25.1%
Average Score					36.6%	21.1%
					1	0
Parameter Group #5						
Rise rate	258.7	0.9127	120.6	0.9699	53.4%	6.3%
Fall rate	-53.13	-0.8737	-53.53	-0.8933	0.8%	2.2%
Number of reversals	64.41	0.2866	92.3	0.2342	43.3%	18.3%
Average Score					32.5%	8.9%
					0	0
Total Point Classification					3	2
note:					<u>Low risk of impact</u>	

Table 9.42 (Continued)

Control Point:	CP28	Period:	1934-2013			
IHA statistics group	Naturalized	Regulated (Senior)		Absolute Chages		
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	202.5	1.375	161.6	0.9816	20.2%	28.6%
February	216.5	1.106	168.8	0.8424	22.0%	23.8%
March	194.9	0.928	155.4	0.6914	20.3%	25.5%
April	235.4	1.156	196.7	1.024	16.4%	11.4%
May	299.9	1.038	218.2	0.8807	27.2%	15.2%
June	410.9	2.149	307.4	2.284	25.2%	6.3%
July	272.2	2.104	200.5	1.745	26.3%	17.1%
August	213.6	1.914	161.1	1.534	24.6%	19.9%
September	260.9	1.529	200.5	1.404	23.2%	8.2%
October	307.1	1.299	231.6	1.097	24.6%	15.6%
November	242.8	1.353	176.1	1.167	27.5%	13.7%
December	204	1.341	161.3	1.119	20.9%	16.6%
Average Score					23.2%	16.8%
					1	0
Parameter Group #2						
1-day minimum	18.11	1.027	23.11	0.8496	27.6%	17.3%
3-day minimum	19.07	1.006	26.34	0.8608	38.1%	14.4%
7-day minimum	21.18	0.9715	29.38	0.8983	38.7%	7.5%
30-day minimum	37.68	0.844	45.89	0.8062	21.8%	4.5%
90-day minimum	73.36	0.7922	75.58	0.6971	3.0%	12.0%
1-day maximum	5041	0.9953	3788	0.9549	24.9%	4.1%
3-day maximum	3533	1.009	2464	0.9361	30.3%	7.2%
7-day maximum	2414	1.064	1691	1.046	30.0%	1.7%
30-day maximum	1010	1.093	698.9	1.16	30.8%	6.1%
90-day maximum	554.8	0.9184	385.7	0.9083	30.5%	1.1%
Average Score					27.6%	7.6%
					0	0
Parameter Group #3						
Date of minimum	195.4	0.2762	195.4	0.2602	0.0%	5.8%
Date of maximum	174.7	0.2499	185.4	0.2322	6.1%	7.1%
Average Score					3.1%	6.4%
					0	0
Parameter Group #4						
Low pulse count	4.488	0.7162	5.625	0.8151	25.3%	13.8%
Low pulse duration	21.36	0.7974	16.63	0.9694	22.1%	21.6%
High pulse count	4.338	0.8413	4.863	0.8159	12.1%	3.0%
High pulse duration	3.118	0.6602	2.569	0.6806	17.6%	3.1%
Average Score					19.3%	10.4%
					0	0
Parameter Group #5						
Rise rate	258.7	0.9127	139.9	0.7863	45.9%	13.8%
Fall rate	-53.13	-0.8737	-52.2	-0.8772	1.8%	0.4%
Number of reversals	64.41	0.2866	94.79	0.2831	47.2%	1.2%
Average Score					31.6%	5.2%
					0	0
Total Point					1	
Classification					2	
note:					Low risk of impact	

Table 9.43 DHRAM results at Control Point CP29 (Naturalized vs. Regulated Flows under SB3 EFS with Junior and Senior Priority)

Control Point:	CP29		Period: 1934-2013			
IHA statistics group	Naturalized		Regulated (Junior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	336.7	1.394	199.2	1.681	40.8%	20.6%
February	344.1	1.001	193	1.181	43.9%	18.0%
March	283.4	0.8359	142.1	0.9389	49.9%	12.3%
April	343	0.9815	198.8	1.19	42.0%	21.2%
May	475.6	1.029	296.9	1.2	37.6%	16.6%
June	615.6	1.844	434.4	2.314	29.4%	25.5%
July	359.7	1.99	224.6	2.119	37.6%	6.5%
August	272.6	1.938	165.4	2.202	39.3%	13.6%
September	423.5	1.518	284.6	1.882	32.8%	24.0%
October	489	1.253	336.1	1.564	31.3%	24.8%
November	407.9	1.378	275.8	1.642	32.4%	19.2%
December	314.3	1.314	203.1	1.535	35.4%	16.8%
Average Score					37.7%	18.3%
					1	0
Parameter Group #2						
1-day minimum	20.53	1.405	1.04	0.73	94.9%	47.9%
3-day minimum	21.47	1.387	1.93	1.06	91.0%	23.8%
7-day minimum	24.13	1.343	5.05	1.50	79.1%	11.3%
30-day minimum	45.31	1.22	24.54	1.02	45.8%	16.3%
90-day minimum	102.1	0.87	58.13	0.83	43.1%	4.5%
1-day maximum	7854	0.919	6989	0.98	11.0%	7.0%
3-day maximum	5249	0.9317	4136	1.05	21.2%	13.0%
7-day maximum	3634	0.9624	2798	1.09	23.0%	13.2%
30-day maximum	1548	0.9543	1147	1.14	25.9%	19.8%
90-day maximum	835.4	0.834	567	1.02	32.1%	21.8%
Average Score					46.7%	17.9%
					1	0
Parameter Group #3						
Date of minimum	198.9	0.2258	76.73	0.2266	61.4%	0.4%
Date of maximum	189.9	0.2433	182	0.2556	4.2%	5.1%
Average Score					32.8%	2.7%
					2	0
Parameter Group #4						
Low pulse count	4.45	0.7379	11	0.4819	147.2%	34.7%
Low pulse duration	22.49	0.9741	11.8	2.744	47.5%	181.7%
High pulse count	4.988	0.7505	5.338	0.8191	7.0%	9.1%
High pulse duration	2.792	0.5961	2.294	0.6469	17.8%	8.5%
Average Score					54.9%	58.5%
					1	1
Parameter Group #5						
Rise rate	380.7	0.8352	243.7	0.8648	36.0%	3.5%
Fall rate	-91.22	-0.7876	-112.4	-0.7652	23.2%	2.8%
Number of reversals	61.84	0.2761	92.16	0.2749	49.0%	0.4%
Average Score					36.1%	2.3%
					0	0
Total Point Classification					6	3
note:					<u>Moderate risk of impact</u>	

Table 9.43 (Continued)

Control Point:	CP29	Period:	1934-2013			
IHA statistics group	Naturalized	Regulated (Senior)		Absolute Chages		
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	336.7	1.394	268.7	1.235	20.2%	11.4%
February	344.1	1.001	259.9	0.8633	24.5%	13.8%
March	283.4	0.8359	217.5	0.6602	23.3%	21.0%
April	343	0.9815	276.4	0.9009	19.4%	8.2%
May	475.6	1.029	353.8	1.011	25.6%	1.7%
June	615.6	1.844	482.1	1.941	21.7%	5.3%
July	359.7	1.99	268.8	1.83	25.3%	8.0%
August	272.6	1.938	206.8	1.75	24.1%	9.7%
September	423.5	1.518	325.5	1.608	23.1%	5.9%
October	489	1.253	387.2	1.219	20.8%	2.7%
November	407.9	1.378	322	1.343	21.1%	2.5%
December	314.3	1.314	255.5	1.221	18.7%	7.1%
Average Score					22.3%	8.1%
					1	0
Parameter Group #2						
1-day minimum	20.53	1.405	20.24	1.413	1.4%	0.6%
3-day minimum	21.47	1.387	27.98	1.237	30.3%	10.8%
7-day minimum	24.13	1.343	30.34	1.22	25.7%	9.2%
30-day minimum	45.31	1.22	50.96	1.129	12.5%	7.5%
90-day minimum	102.1	0.87	98.1	0.8288	3.9%	4.7%
1-day maximum	7854	0.919	6974	0.9511	11.2%	3.5%
3-day maximum	5249	0.9317	4217	0.9708	19.7%	4.2%
7-day maximum	3634	0.9624	2831	0.9927	22.1%	3.1%
30-day maximum	1548	0.9543	1168	1.036	24.5%	8.6%
90-day maximum	835.4	0.834	617.2	0.8726	26.1%	4.6%
Average Score					17.7%	5.7%
					0	0
Parameter Group #3						
Date of minimum	198.9	0.2258	188.6	0.2187	5.2%	3.1%
Date of maximum	189.9	0.2433	189.1	0.2603	0.4%	7.0%
Average Score					2.8%	5.1%
					0	0
Parameter Group #4						
Low pulse count	4.45	0.7379	7.3	0.9442	64.0%	28.0%
Low pulse duration	22.49	0.9741	15.02	1.311	33.2%	34.6%
High pulse count	4.988	0.7505	5.388	0.7642	8.0%	1.8%
High pulse duration	2.792	0.5961	2.186	0.5465	21.7%	8.3%
Average Score					31.7%	18.2%
					0	0
Parameter Group #5						
Rise rate	380.7	0.8352	254.3	0.7573	33.2%	9.3%
Fall rate	-91.22	-0.7876	-105.4	-0.7886	15.5%	0.1%
Number of reversals	61.84	0.2761	108.6	0.291	75.6%	5.4%
Average Score					41.5%	5.0%
					0	0
Total Point					1	
Classification					2	
note:					Low risk of impact	

Table 9.44 DHRAM results at Control Point CP32 (Naturalized vs. Regulated Flows under SB3 EFS with Junior and Senior Priority)

Control Point:	CP32		Period: 1934-2013			
IHA statistics group	Naturalized		Regulated (Junior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	389.9	1.42	255.6	1.618	34.4%	13.9%
February	400.2	0.9972	254.7	1.102	36.4%	10.5%
March	326.8	0.7825	188.6	0.8287	42.3%	5.9%
April	405.9	0.9906	260.5	1.145	35.8%	15.6%
May	555.9	0.937	368.4	1.031	33.7%	10.0%
June	728.8	1.757	530.5	2.128	27.2%	21.1%
July	434	1.695	285.2	1.715	34.3%	1.2%
August	343.2	1.674	236.4	1.741	31.1%	4.0%
September	501.7	1.387	359.4	1.595	28.4%	15.0%
October	555.6	1.191	403.6	1.385	27.4%	16.3%
November	476.6	1.34	345.6	1.503	27.5%	12.2%
December	363.3	1.238	255.7	1.346	29.6%	8.7%
Average Score					32.3%	11.2%
					1	0
Parameter Group #2						
1-day minimum	35.68	0.9582	17.19	0.63	51.8%	34.7%
3-day minimum	37.31	0.9518	19.16	0.60	48.6%	37.5%
7-day minimum	40.89	0.9096	23.61	0.68	42.3%	25.8%
30-day minimum	71.03	0.8185	48.90	0.67	31.2%	18.5%
90-day minimum	143.3	0.6597	96.29	0.57	32.8%	14.1%
1-day maximum	4866	0.9222	4044	0.99	16.9%	7.7%
3-day maximum	4403	0.9237	3360	1.05	23.7%	14.1%
7-day maximum	3602	0.9432	2724	1.08	24.4%	14.0%
30-day maximum	1697	0.9229	1281	1.09	24.5%	17.6%
90-day maximum	944.2	0.8146	673	0.94	28.7%	15.5%
Average Score					32.5%	19.9%
					0	0
Parameter Group #3						
Date of minimum	194	0.2631	173.7	0.2584	10.5%	1.8%
Date of maximum	193.5	0.2506	187.8	0.25	2.9%	0.2%
Average Score					6.7%	1.0%
					0	0
Parameter Group #4						
Low pulse count	4.7	0.6355	8.95	0.5019	90.4%	21.0%
Low pulse duration	19.43	0.7268	9.885	0.7986	49.1%	9.9%
High pulse count	2.838	0.9032	3.613	0.8759	27.3%	3.0%
High pulse duration	6.941	0.5747	4.922	0.7027	29.1%	22.3%
Average Score					49.0%	14.0%
					1	0
Parameter Group #5						
Rise rate	166.1	0.8625	140.2	0.7917	15.6%	8.2%
Fall rate	-50.34	-0.7761	-60.94	-0.7334	21.1%	5.5%
Number of reversals	52.54	0.1982	88.53	0.1769	68.5%	10.7%
Average Score					35.1%	8.2%
					0	0
Total Point					2	
Classification					2	
note:					<u>Low risk of impact</u>	

Table 9.44 (Continued)

Control Point:	CP32	Period:	1934-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	389.9	1.42	321.1	1.278	17.6%	10.0%
February	400.2	0.9972	315.4	0.8852	21.2%	11.2%
March	326.8	0.7825	259.4	0.6351	20.6%	18.8%
April	405.9	0.9906	333.5	0.9214	17.8%	7.0%
May	555.9	0.937	424.4	0.8984	23.7%	4.1%
June	728.8	1.757	578.1	1.839	20.7%	4.7%
July	434	1.695	333.7	1.511	23.1%	10.9%
August	343.2	1.674	275.3	1.464	19.8%	12.5%
September	501.7	1.387	396.4	1.419	21.0%	2.3%
October	555.6	1.191	447.7	1.138	19.4%	4.5%
November	476.6	1.34	387.9	1.289	18.6%	3.8%
December	363.3	1.238	303.4	1.131	16.5%	8.6%
Average Score					20.0%	8.2%
					1	0
Parameter Group #2						
1-day minimum	35.68	0.9582	35.62	0.9888	0.2%	3.2%
3-day minimum	37.31	0.9518	42.78	0.8983	14.7%	5.6%
7-day minimum	40.89	0.9096	46.48	0.8651	13.7%	4.9%
30-day minimum	71.03	0.8185	74.66	0.7771	5.1%	5.1%
90-day minimum	143.3	0.6597	135.8	0.6101	5.2%	7.5%
1-day maximum	4866	0.9222	4035	0.9122	17.1%	1.1%
3-day maximum	4403	0.9237	3330	0.9366	24.4%	1.4%
7-day maximum	3602	0.9432	2733	0.9785	24.1%	3.7%
30-day maximum	1697	0.9229	1302	0.9927	23.3%	7.6%
90-day maximum	944.2	0.8146	718.3	0.8325	23.9%	2.2%
Average Score					15.2%	4.2%
					0	0
Parameter Group #3						
Date of minimum	194	0.2631	196.6	0.2621	1.3%	0.4%
Date of maximum	193.5	0.2506	193.1	0.2413	0.2%	3.7%
Average Score					0.8%	2.0%
					0	0
Parameter Group #4						
Low pulse count	4.7	0.6355	5.688	0.7174	21.0%	12.9%
Low pulse duration	19.43	0.7268	16.2	0.8749	16.6%	20.4%
High pulse count	2.838	0.9032	3.763	0.9082	32.6%	0.6%
High pulse duration	6.941	0.5747	4.677	0.6419	32.6%	11.7%
Average Score					25.7%	11.4%
					0	0
Parameter Group #5						
Rise rate	166.1	0.8625	140.9	0.75	15.2%	13.0%
Fall rate	-50.34	-0.7761	-59.79	-0.7273	18.8%	6.3%
Number of reversals	52.54	0.1982	98.28	0.2412	87.1%	21.7%
Average Score					40.3%	13.7%
					0	0
Total Point					1	
Classification					2	
note:					<u>Low risk of impact</u>	

Table 9.45 DHRAM results at Control Point CP35 (Naturalized vs. Regulated Flows
under SB3 EFS with Senior Priority)

Control Point:	CP35	Period:	1934-2013			
IHA statistics group	Naturalized		Regulated (Senior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	94.84	2.25	94.23	2.262	0.6%	0.5%
February	96.71	1.479	95.74	1.488	1.0%	0.6%
March	65.14	1.775	63.7	1.806	2.2%	1.7%
April	141	1.841	139.1	1.859	1.3%	1.0%
May	204.2	1.613	201.1	1.631	1.5%	1.1%
June	211.8	1.976	208.4	2.001	1.6%	1.3%
July	135	2.715	133	2.745	1.5%	1.1%
August	60.42	2.136	59.11	2.165	2.2%	1.4%
September	159.5	1.97	157.8	1.986	1.1%	0.8%
October	163.8	1.942	162.5	1.954	0.8%	0.6%
November	113.6	1.855	112.7	1.866	0.8%	0.6%
December	94.16	2.414	93.53	2.427	0.7%	0.5%
				Average Score	1.3%	0.9%
					0	0
Parameter Group #2						
1-day minimum	2.164	1.2	2.164	1.2	0.0%	0.0%
3-day minimum	2.337	1.153	2.337	1.153	0.0%	0.0%
7-day minimum	2.828	1.081	2.828	1.081	0.0%	0.0%
30-day minimum	7.356	0.8235	7.298	0.8192	0.8%	0.5%
90-day minimum	20.65	0.6713	20.19	0.6677	2.2%	0.5%
1-day maximum	2775	1.109	2768	1.111	0.3%	0.2%
3-day maximum	2293	1.045	2286	1.047	0.3%	0.2%
7-day maximum	1659	1.01	1653	1.014	0.4%	0.4%
30-day maximum	690.2	0.9501	686.1	0.9551	0.6%	0.5%
90-day maximum	321.9	0.8916	318.9	0.8968	0.9%	0.6%
				Average Score	0.5%	0.3%
					0	0
Parameter Group #3						
Date of minimum	180.6	0.2573	180.6	0.2573	0.0%	0.0%
Date of maximum	198.4	0.2498	200.2	0.253	0.9%	1.3%
				Average Score	0.5%	0.6%
					0	0
Parameter Group #4						
Low pulse count	7.213	0.5371	7.5	0.543	4.0%	1.1%
Low pulse duration	12.74	0.69	12.41	0.6586	2.6%	4.6%
High pulse count	2.925	0.8511	2.925	0.8511	0.0%	0.0%
High pulse duration	5.651	0.6739	5.627	0.6786	0.4%	0.7%
				Average Score	1.7%	1.6%
					0	0
Parameter Group #5						
Rise rate	99.08	0.9965	96.6	1.004	2.5%	0.8%
Fall rate	-25.62	-0.8949	-25.81	-0.8943	0.7%	0.1%
Number of reversals	63.75	0.1421	68.25	0.1297	7.1%	8.7%
				Average Score	3.4%	3.2%
					0	0
Total Point					0	
Classification					1	
note:					<u>Un-impacted condition</u>	

Table 9.46 DHRAM results at Control Point CP37 (Naturalized vs. Regulated Flows under SB3 EFS with Junior and Senior Priority)

Control Point:	CP37		Period: 1934-2013			
IHA statistics group	Naturalized		Regulated (Junior)		Absolute Chages	
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	545.3	1.393	423.6	1.506	22.3%	8.1%
February	598.3	1.103	464.3	1.2	22.4%	8.8%
March	470.3	1.013	338	1.191	28.1%	17.6%
April	653.2	1.158	512.3	1.3	21.6%	12.3%
May	959.4	1.063	776.6	1.141	19.1%	7.3%
June	1164	1.746	968	1.946	16.8%	11.5%
July	740.1	1.905	592.5	2.016	19.9%	5.8%
August	423.8	1.504	320.1	1.607	24.5%	6.8%
September	910.7	1.724	773.9	1.928	15.0%	11.8%
October	957.4	1.512	816.2	1.681	14.7%	11.2%
November	729.4	1.377	607.4	1.481	16.7%	7.6%
December	525.8	1.523	426.9	1.664	18.8%	9.3%
Average Score					20.0%	9.8%
					1	0
Parameter Group #2						
1-day minimum	55.35	0.7719	23.82	1.00	57.0%	29.7%
3-day minimum	57.47	0.7669	30.77	0.89	46.5%	16.7%
7-day minimum	62.65	0.764	42.92	0.76	31.5%	0.1%
30-day minimum	101.6	0.7637	78.69	0.72	22.5%	5.8%
90-day minimum	195.6	0.6416	149.20	0.61	23.7%	5.2%
1-day maximum	7615	1.066	7068	1.12	7.2%	4.8%
3-day maximum	6839	1.022	6169	1.09	9.8%	6.2%
7-day maximum	5854	1.043	5221	1.12	10.8%	7.0%
30-day maximum	2975	0.9302	2590	1.00	12.9%	7.8%
90-day maximum	1569	0.8147	1317	0.88	16.1%	8.0%
Average Score					23.8%	9.1%
					0	0
Parameter Group #3						
Date of minimum	210.6	0.2656	204.5	0.2329	2.9%	12.3%
Date of maximum	197.8	0.2435	197.8	0.2396	0.0%	1.6%
Average Score					1.4%	7.0%
					0	0
Parameter Group #4						
Low pulse count	3.788	0.6463	6.6	0.5174	74.2%	19.9%
Low pulse duration	24.92	0.7837	15.46	1.029	38.0%	31.3%
High pulse count	1.963	0.9811	2.05	0.9597	4.4%	2.2%
High pulse duration	10.75	0.5514	9.509	0.5374	11.5%	2.5%
Average Score					32.0%	14.0%
					0	0
Parameter Group #5						
Rise rate	168.7	0.9088	155.1	0.8717	8.1%	4.1%
Fall rate	-70.4	-0.8615	-76.07	-0.8259	8.1%	4.1%
Number of reversals	43.9	0.1467	69.63	0.1505	58.6%	2.6%
Average Score					24.9%	3.6%
					0	0
Total Point Classification					1	2
note:					<u>Low risk of impact</u>	

Table 9.46 (Continued)

Control Point:	CP37	Period:	1934-2013			
IHA statistics group	Naturalized	Regulated (Senior)		Absolute Chages		
	Mean(cfs)	CV	Mean(cfs)	CV	Mean(%)	CV(%)
Parameter Group #1						
January	545.3	1.393	481.8	1.319	11.6%	5.3%
February	598.3	1.103	519.5	1.079	13.2%	2.2%
March	470.3	1.013	402.9	1.001	14.3%	1.2%
April	653.2	1.158	580.9	1.154	11.1%	0.3%
May	959.4	1.063	829.4	1.072	13.6%	0.8%
June	1164	1.746	1015	1.802	12.8%	3.2%
July	740.1	1.905	640.4	1.888	13.5%	0.9%
August	423.8	1.504	357.5	1.373	15.6%	8.7%
September	910.7	1.724	810.7	1.842	11.0%	6.8%
October	957.4	1.512	856.3	1.556	10.6%	2.9%
November	729.4	1.377	646.7	1.367	11.3%	0.7%
December	525.8	1.523	470.1	1.503	10.6%	1.3%
Average Score					12.4%	2.9%
					0	0
Parameter Group #2						
1-day minimum	55.35	0.7719	46.83	0.8259	15.4%	7.0%
3-day minimum	57.47	0.7669	58.91	0.7809	2.5%	1.8%
7-day minimum	62.65	0.764	67.41	0.7669	7.6%	0.4%
30-day minimum	101.6	0.7637	104.7	0.7539	3.1%	1.3%
90-day minimum	195.6	0.6416	187.7	0.6235	4.0%	2.8%
1-day maximum	7615	1.066	7062	1.095	7.3%	2.7%
3-day maximum	6839	1.022	6169	1.062	9.8%	3.9%
7-day maximum	5854	1.043	5248	1.093	10.4%	4.8%
30-day maximum	2975	0.9302	2626	0.9716	11.7%	4.5%
90-day maximum	1569	0.8147	1363	0.8336	13.1%	2.3%
Average Score					8.5%	3.2%
					0	0
Parameter Group #3						
Date of minimum	210.6	0.2656	215.3	0.2399	2.2%	9.7%
Date of maximum	197.8	0.2435	194.8	0.2487	1.5%	2.1%
Average Score					1.9%	5.9%
					0	0
Parameter Group #4						
Low pulse count	3.788	0.6463	4.6	0.6449	21.4%	0.2%
Low pulse duration	24.92	0.7837	20.67	0.8932	17.1%	14.0%
High pulse count	1.963	0.9811	2.125	0.96	8.3%	2.2%
High pulse duration	10.75	0.5514	9.03	0.5629	16.0%	2.1%
Average Score					15.7%	4.6%
					0	0
Parameter Group #5						
Rise rate	168.7	0.9088	157.9	0.8738	6.4%	3.9%
Fall rate	-70.4	-0.8615	-73.36	-0.8332	4.2%	3.3%
Number of reversals	43.9	0.1467	70.21	0.2199	59.9%	49.9%
Average Score					23.5%	19.0%
					0	0
Total Point					0	
Classification					1	
note:					Un-impacted condition	

The annual median values at the fifteen control points are extracted from the naturalized and two different regulated flows under SB3 EFSs with a junior and a senior priority to all other water rights. The three duration curves are plotted to compare the hydrologic states of each flow for the period 1934-2013 at the fifteen control points.

Figure 9.11 shows the hydrologic states of the three different flows at the control point CP01. The flow duration curves of both the regulated flows are slightly different from the naturalized flows, but both are almost similar. This means that the regulated flow regime is not changed by EFS with a senior priority at the control point.

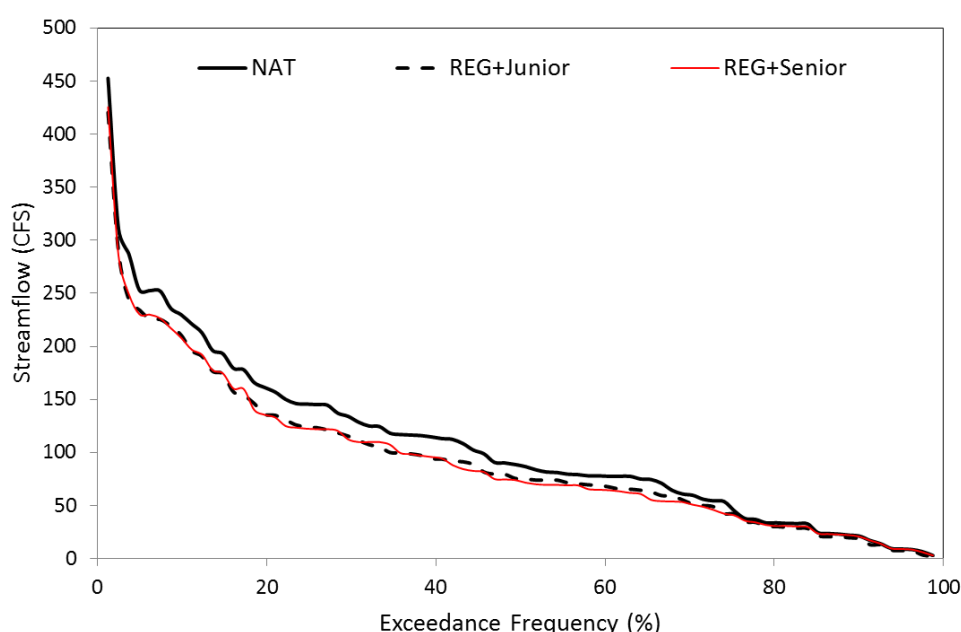


Figure 9.11 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point CP01

Figure 9.12 shows the hydrologic states of the three different flows at the control point CP02. The flow duration curves of both the regulated flows are nearly similar to the naturalized flows, and both are also same like the control point CP01. This is because there are a few existing water rights upstream of the control point.

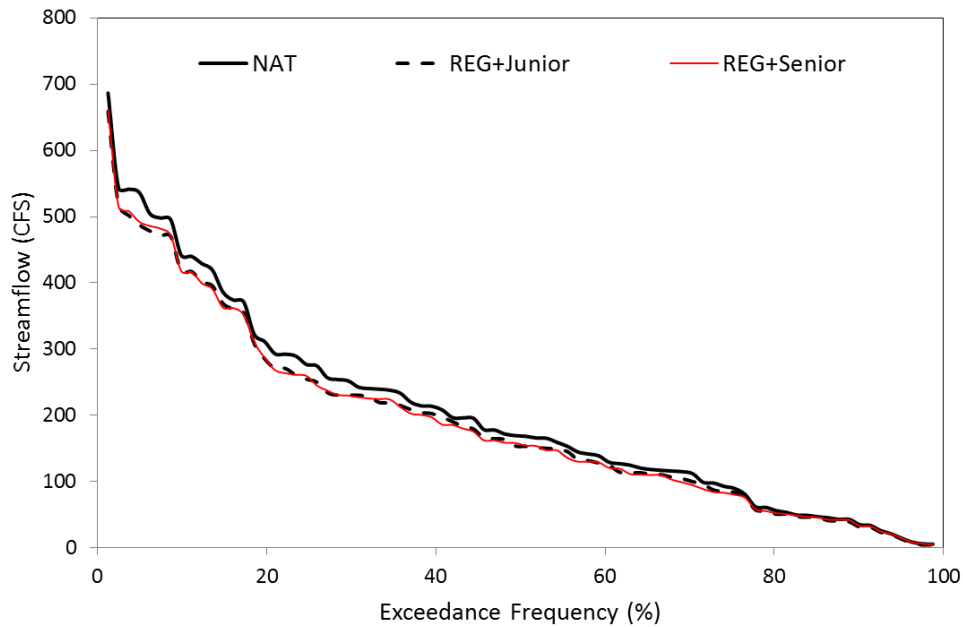


Figure 9.12 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point CP02

Figure 9.13 presents the hydrologic states of the three different flows at the control point CP08. The flow duration curves of both the regulated flows are very similar to the naturalized flows, and both are perfectly same. This are considered there are no existing water rights upstream of the control point.

Figure 9.14 presents the hydrologic states of the three different flows at the control point CP10. The flow duration curves of both the regulated flows are very similar to the naturalized flows, and both are almost same. This means that there are no existing water uses upstream of the control point.

Figure 9.15 presents the hydrologic states of the three different flows at the control point CP11. The flow duration curves of both the regulated flows are very similar to the naturalized flows, and both are almost similar like CP08 and CP10.

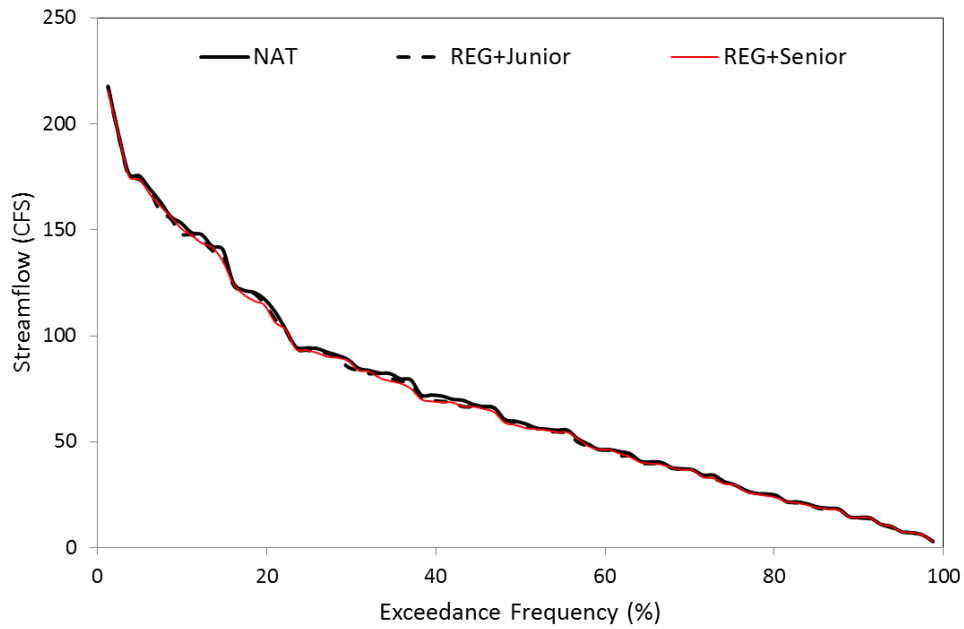


Figure 9.13 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point CP08

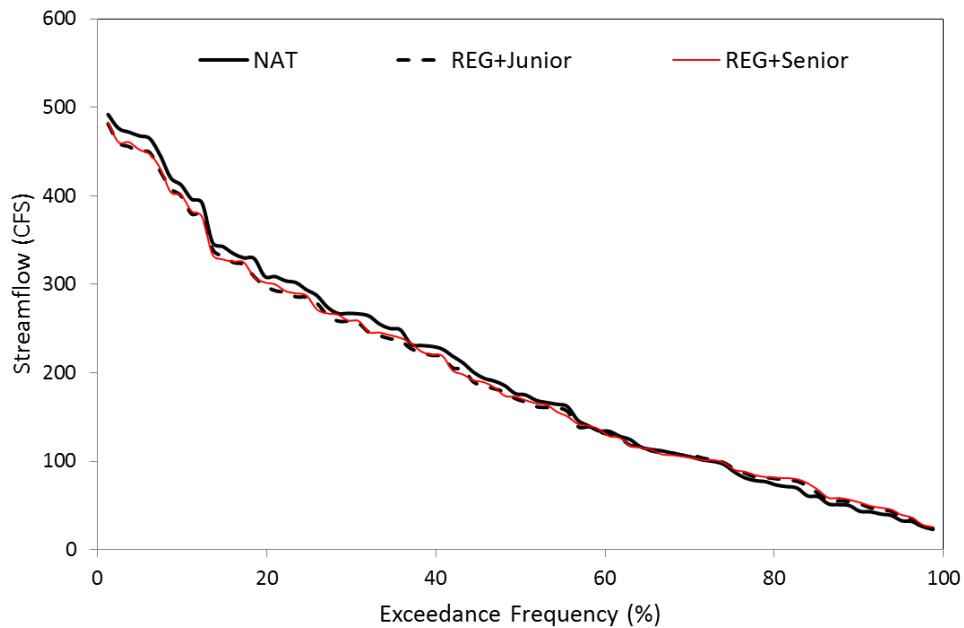


Figure 9.14 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point CP10

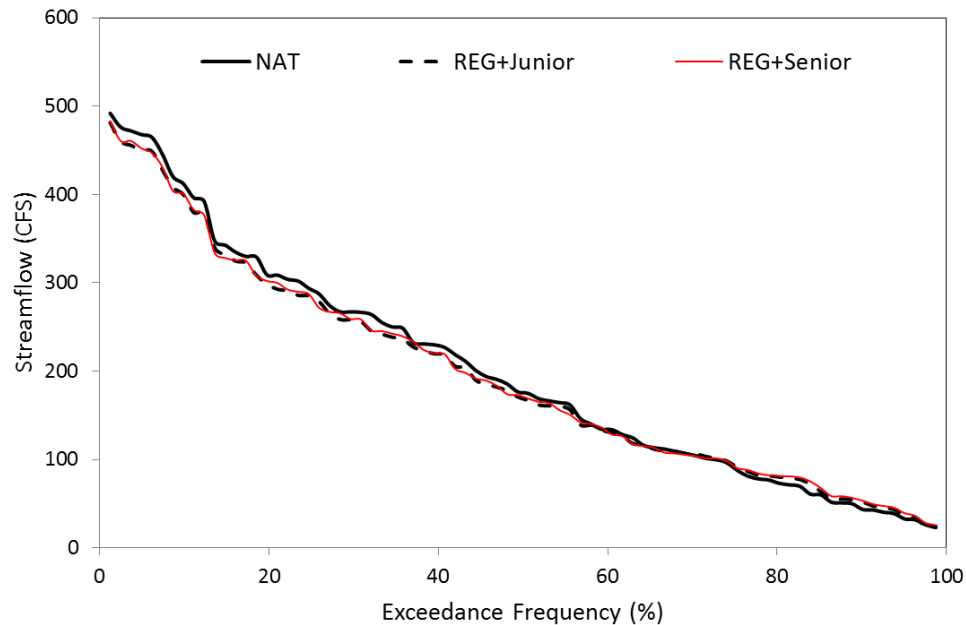


Figure 9.15 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point CP11

Figure 9.16 presents the hydrologic states of the three different flows at the control point C38461. The flow duration curves of both the regulated flows are totally different from the naturalized flows, but both are also different. There are several hydropower water rights at the Canyon Lake and related lakes that are located upstream of the control points C38461, CP14, and CP15 in the Guadalupe River Basin. Most flows, used for hydropower should be fully returned to the downstream, and these flows can be used for other water rights. This shows that SB3 EFS can tremendously improve the flow regimes of regulated flow by modifying a senior priority. However, the alteration between the naturalized flows and regulated flows with a senior priority is higher than with a junior priority in the DHRAM analysis at the control point.

Figure 9.17 presents the hydrologic states of the three different flows at the control point CP17. The flow duration curves of both the regulated flows are very similar to the naturalized flows, and both are almost same. There are no effects by SB EFS at the control point.

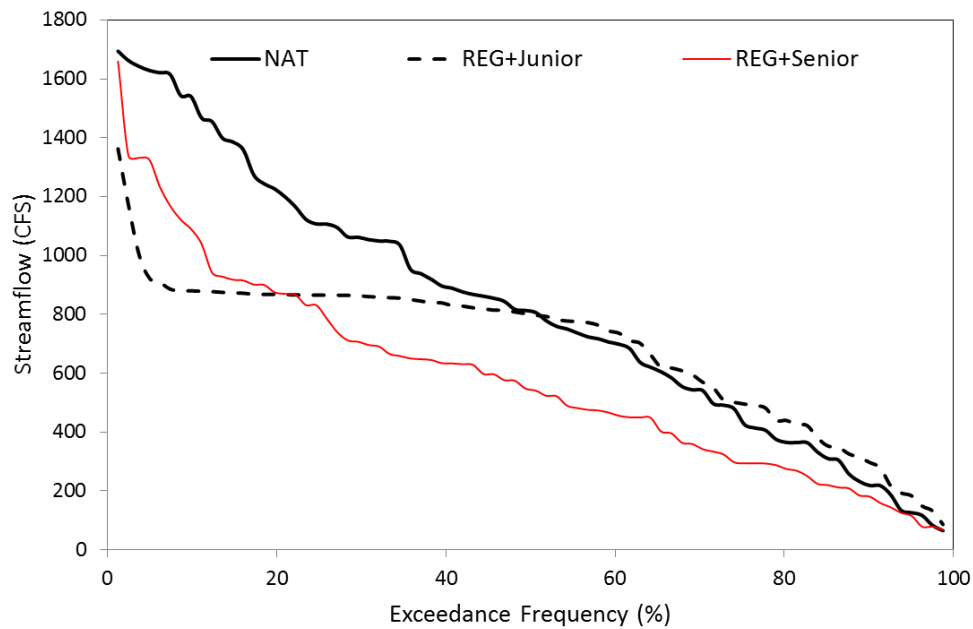


Figure 9.16 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point C38461

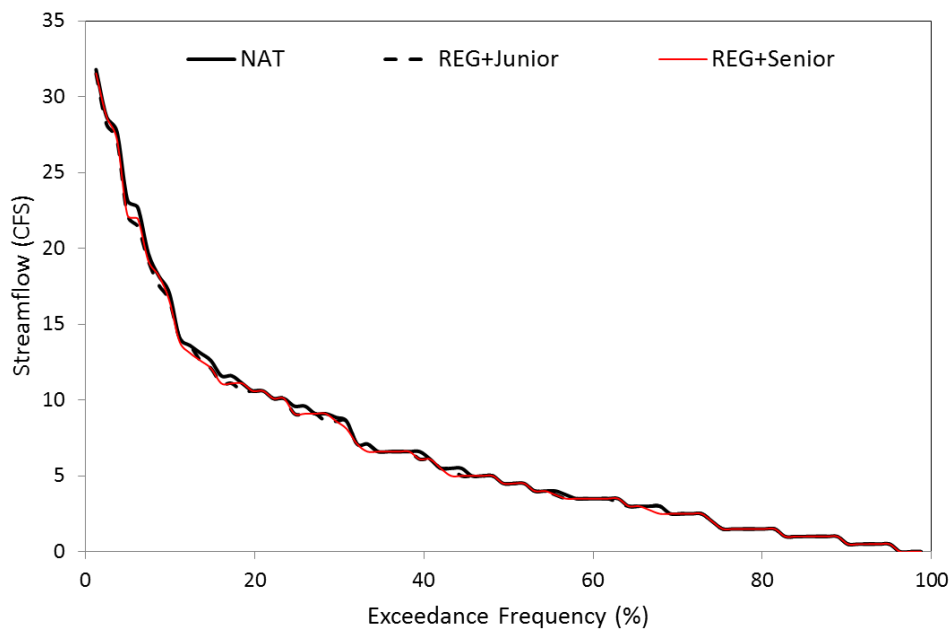


Figure 9.17 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point CP13

Figure 9.18 presents the hydrologic states of the three different flows at the control point CP14. The flow duration curves of both the regulated flows are totally different from the naturalized flows, but both are also different. This shows that SB3 EFS makes the flow regimes of regulated flow with a senior priority more similar to the naturalized flow.

Figure 9.19 presents the hydrologic states of the three different flows at the control point CP15. The flow duration curves of both the regulated flows are totally different from the naturalized flows, but both are also different. This shows that SB3 EFS makes the flow regimes of regulated flow with a senior priority more similar to the naturalized flow.

Figure 9.20 presents the hydrologic states of the three different flows at the control point P382411. The flow duration curves of both the regulated flows are very similar to the naturalized flows, and both are almost same. This means that there are a few water rights at the control point and other upstream control points.

Figure 9.21 presents the hydrologic states of the three different flows at the control point CP28. SB3 EFS with a senior priority makes the flow regime of the regulated flow much closer to the naturalized flows than the regulated flow with a junior priority at the control point. This attributes that the timing of Medina Lake release is changes to meet the EFS at the downstream control points.

Figures 9.22 and 9.23 present the hydrologic states of the three different flows at the control points CP29 and CP32, respectively. SB3 EFS with a senior priority makes the flow regime of the regulated flow much closer to the naturalized flows than the regulated flow with a junior priority at the control points like the control point CP28. This attributes that the flow regime at the control point CP 28 directly influences the flow regime at the control points CP29 and CP32.

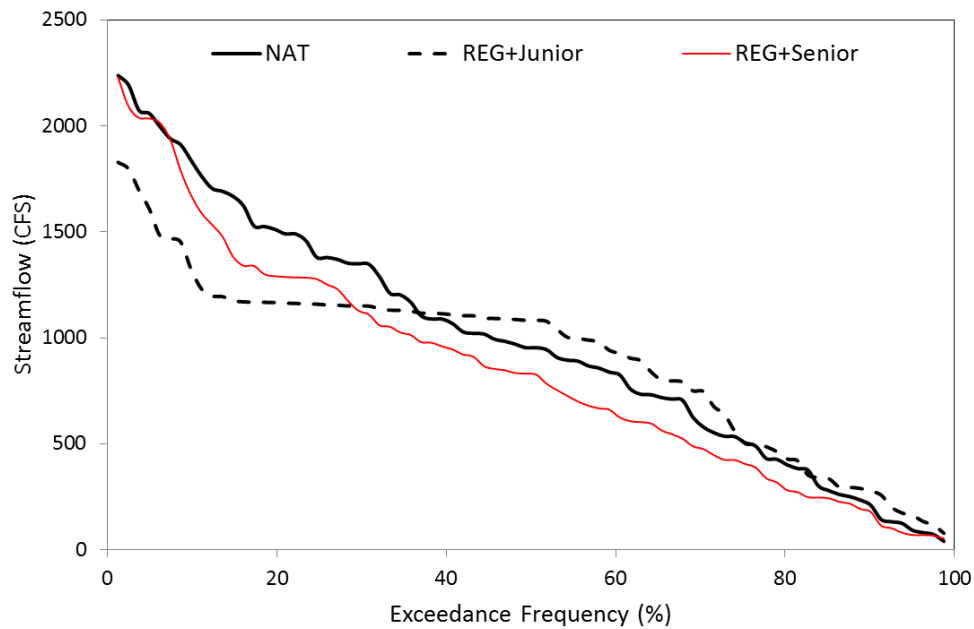


Figure 9.18 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point CP14

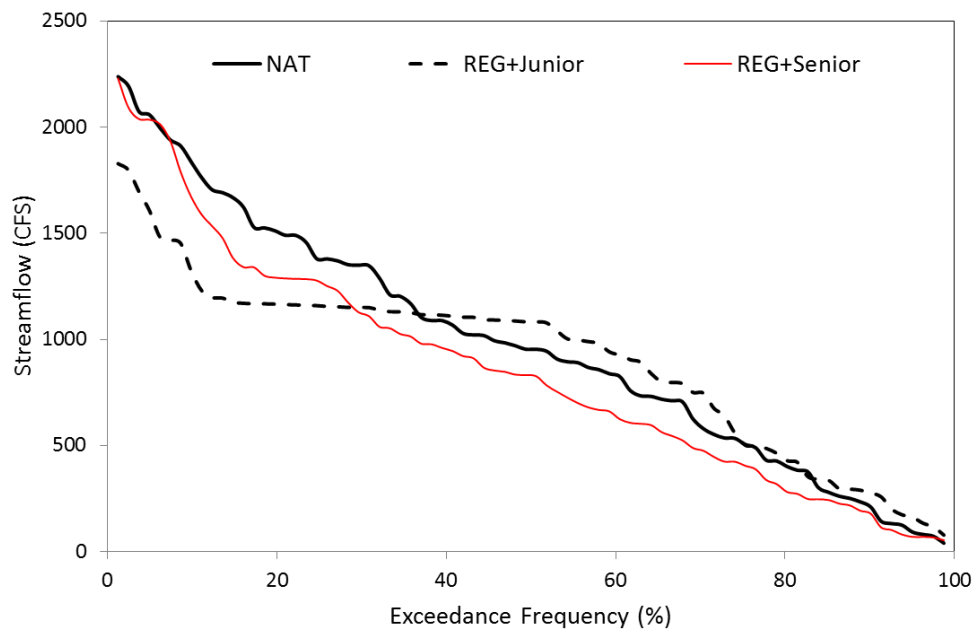


Figure 9.19 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point CP15

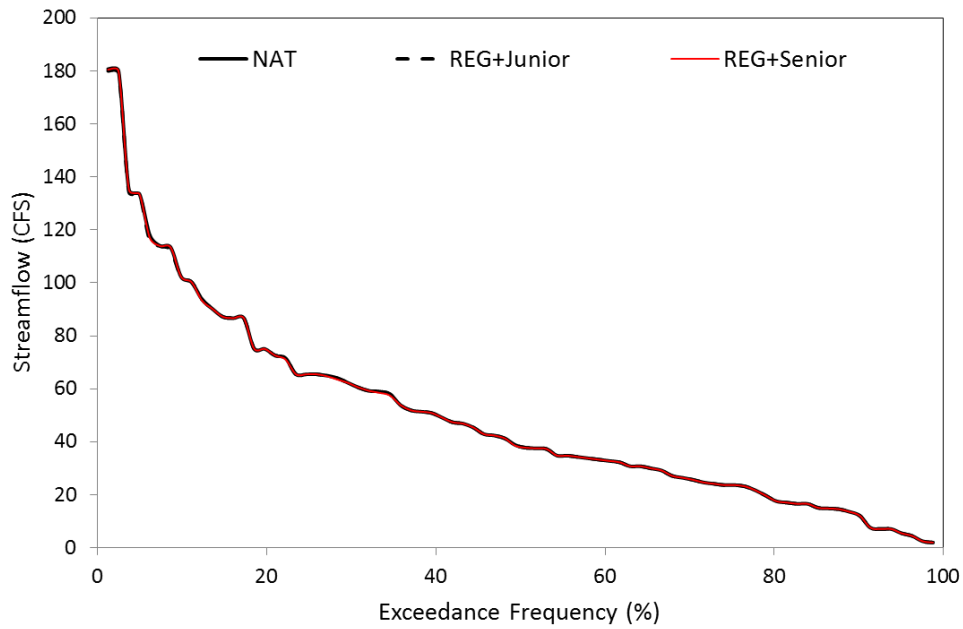


Figure 9.20 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point P38241

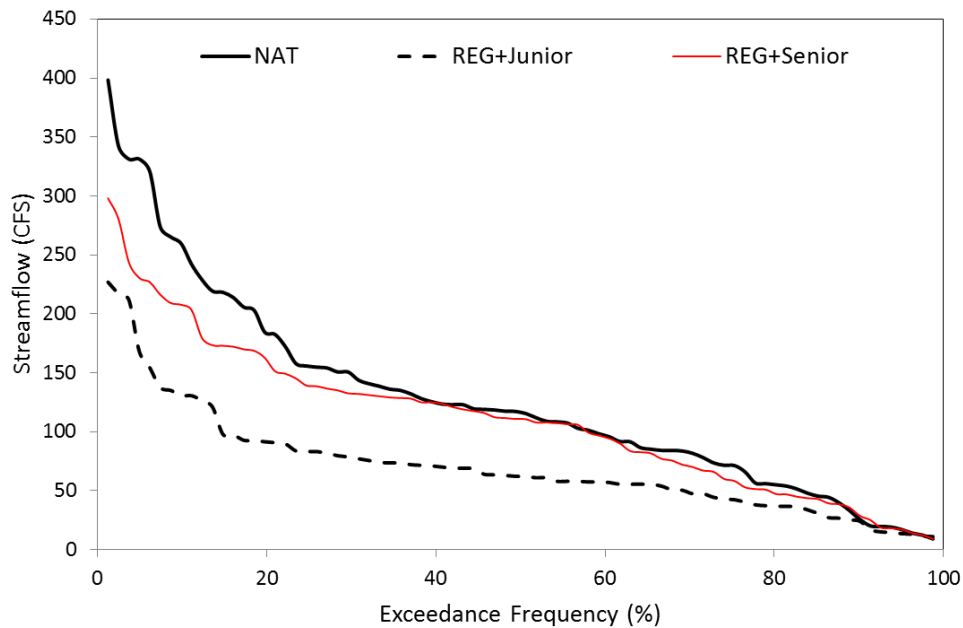


Figure 9.21 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point CP28

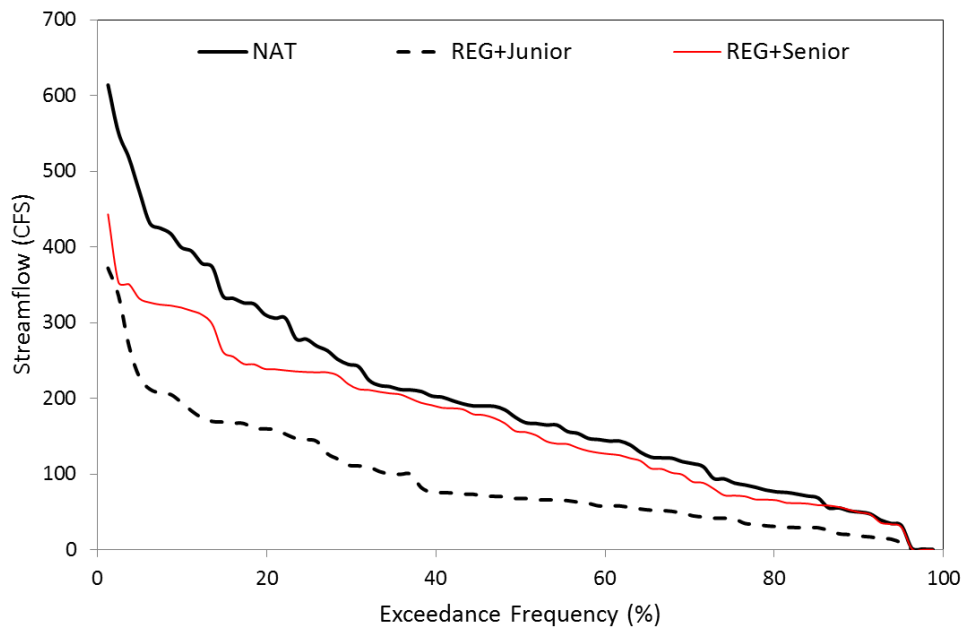


Figure 9.22 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point CP29

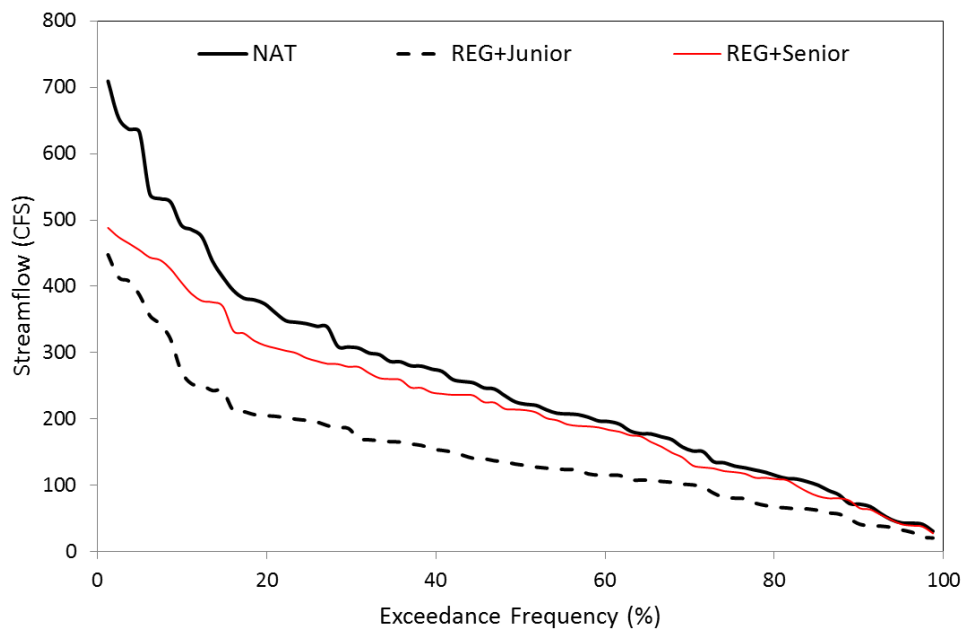


Figure 9.23 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point CP32

Figure 9.24 presents the hydrologic states of the three different flows at the control point CP35. The flow duration curves of both the regulated flows are very similar to the naturalized flows, and both are almost same. This is because there are a few water rights at the control point and other upstream control points.

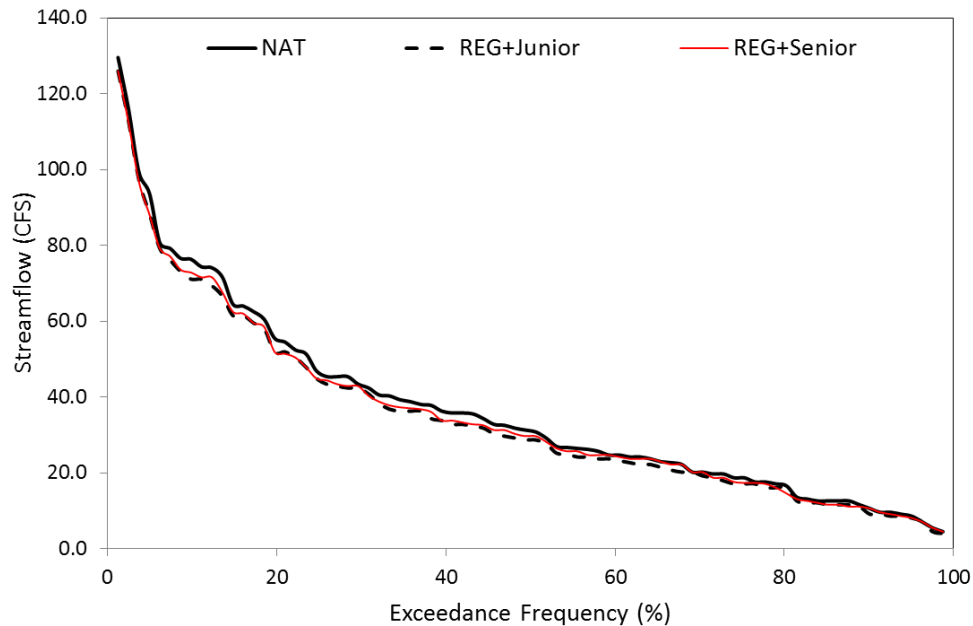


Figure 9.24 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point CP35

Figure 9.25 presents the hydrologic states of the three different flows at the control point CP37. SB3 EFS with a senior priority improves the flow regime of the regulated flow much closer to the naturalized flows than the regulated flow with a junior priority at the control point. The operation rule of Medina Lake by the EFS with a senior priority results in this alteration of the regulated flows at the control point like the control points, located upstream of the control point CP37.

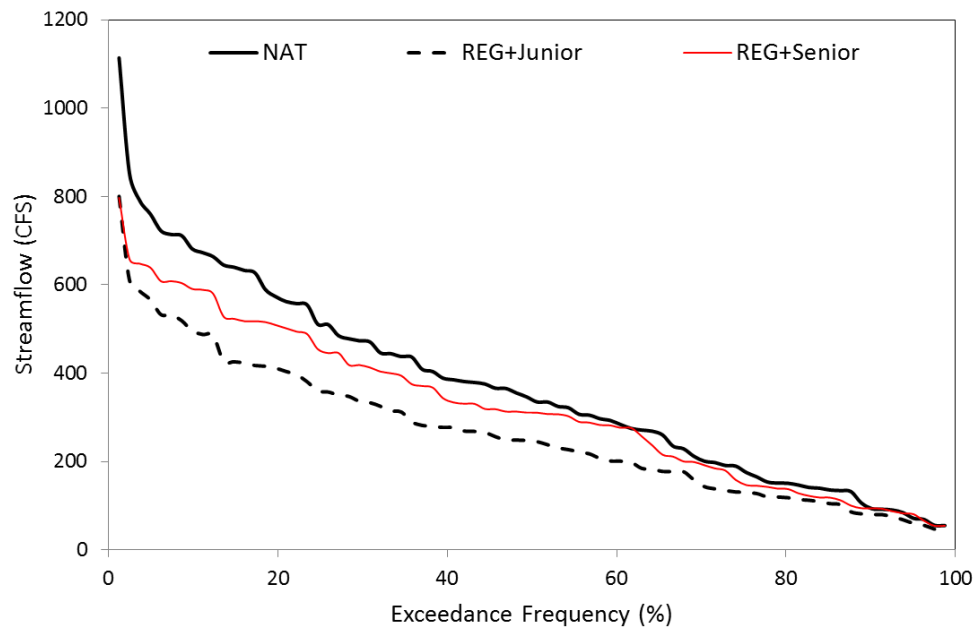


Figure 9.25 Annual Median Flow Duration Curves (NAT: Naturalized flow, REG+Junior: Regulated flow under EFS with a junior priority, REG+Senior: Regulated flow under EFS with a senior priority) at the Control Point CP37

CHAPTER X

SUMMARY AND CONCLUSIONS

The WRAP/WAM modeling system, as well as many other river/reservoir system management models reported in the literature, simulate system behavior under specified sets of conditions to analyze strategies for flood control, water supply, hydropower generation, navigation, recreation, and maintenance of environmental flows. Naturalized or otherwise homogeneous sequences of river flows are fundamental input required for these river/reservoir system models. The river flow input datasets capture the hydrologic characteristics of a river basin including severe multiple-year droughts, major floods, and the full range of more normal flow fluctuations. Incorporation of environmental flow considerations in river system management and associated modeling has been growing in importance in recent years in Texas and throughout the world.

Results and conclusions regarding the main modeling and analysis tasks covered in the preceding chapters of this dissertation are summarized in this final chapter. The dissertation research combines databases and computational techniques provided by the Water Rights Analysis Package (WRAP), Texas Water Availability Modeling (WAM) System, Soil and Water Assessment Tool (SWAT) modeling system, and National Water Information System (NWIS) to investigate and improve capabilities for developing and analyzing long sequences of homogenous river flows at multiple sites in major river/reservoir systems. The research investigates: (1) methods for developing naturalized or otherwise homogenous river flows representing specified conditions of basin development for input to models for simulating river/reservoir system operations and (2) characteristics of observed, naturalized, and simulated regulated river flows of relevance to water management and modeling thereof.

SWAT is used for a variety of purposes in this dissertation research. Alternative SWAT calibration strategies are investigated. SWAT results with and without calibration are compared for case study river systems. Other methodologies are combined with the SWAT and WRAP/WAM modeling systems to perform various tasks involved in developing

naturalized flow datasets and analyzing flow characteristics. Alternative naturalized flow datasets are incorporated in the three case study WRAP/WAM models (called WAMs) for the Sabine, Neches, and Guadalupe and San Antonio (GSA) River Basins. The WRAP/WAM simulation studies performed in the research focus on environmental flow standards and their effects on regulated and unappropriated flows as well as on developing naturalized flow input datasets.

10.1 Infilling Missing Naturalized Flows Using the SWAT Model

Sequences of naturalized monthly flows at all primary control points covering a specified period-of-analysis are a necessary component of the input dataset for a WRAP simulation. The naturalized monthly flows in the WAM System were developed primarily by adjusting observed flows at USGS gauging stations to remove the effects of human activity. Gauge records often do not cover the entire model simulation period-of-analysis. Filling in gaps of missing data is a fundamental issue in developing naturalized flow datasets. The research investigated the use of SWAT and the maintenance of variance method, type 2, called MOVE2 (Hirsch, 1982) for filling in missing monthly naturalized flows. SWAT can be applied without MOVE2, MOVE2 can be applied without SWAT, or SWAT and MOVE2 can be applied in combination as proposed here.

The following strategy for applying SWAT and MOVE2 in combination to fill in gaps and/or extend sequences of monthly naturalized flows was developed and tested by application to the three case study WAMs. The monthly SWAT model synthesizes the monthly flow sequences for the period-of-analysis at a target site that has missing data. This model is calibrated with the available monthly flow data. The calibrated model generates the monthly flow sequence for a missing period at the site. The MOVE2 method firstly estimates population mean and variance of the flow sequence including a missing period at a target site. The method transfers the flow sequence for a missing period at a target site based on the flow sequence synthesized by the SWAT model and the estimated statistical parameters. The SWAT model plays the role of generating a highly correlated flow sequence at a gauged site. The MOVE2 method transfers the flow sequence for a

missing period while preserving their statistical parameters at a target site. This means that the MOVE2 can transfer flow sequences to preserve the homogeneity at a target site.

The case studies test and demonstrate the performance of the method. The method is applied to monthly naturalized datasets at 17 control points for the three WAMs. The assumed missing periods are 18 years (1981-1998) for the Sabine WAM, 16 years (1981-1996) for the Neches WAM, and 14 years (1976 -1989) for the GSA WAM, respectively. The validity and accuracy of the method are evaluated by the Nash-Sutcliff efficiency (NSE) between the infilling naturalized data and naturalized flow dataset at the control points and by the comparison of flow duration curves for the assumed missing periods.

The following conclusions are derived from the case studies:

- The performances of a proposed method (the combination of the SWAT model and MOVE2 method) for infilling missing data are satisfactory in the evaluation by NSE at the 16 control points except for the control point CP05 in the Guadalupe River Basin. However, the method considerably improves NSE values from -3.03 to 0.48 as compared to applying only the synthesized flows sequences from the SWAT model at the control point CP05.
- The comparisons of the flow frequency metrics show that the infilled flow sequences for assumed missing periods have very similar hydrologic characteristics to the original flow dataset.
- The proposed method reliably fills in gaps of missing period while preserving homogeneity of flow sequences.
- If there are no highly correlated flow sequences around the target site that has a missing period, the method is an effective alternative.
- The proposed method can be applicable for developing new datasets or revising or extending existing datasets.

10.2 Refinement of Methods for Developing Naturalized Flows at Ungauged Sites Based on Flows at Gauged Sites

The correction factor K and an exponent ϕ of Equation 2.9 was added to the conventional equation of drainage area ratio method to remove or reduce the transfer errors when drainage ratios are less than 0.3 or more than 1.5. A new method to get representative K and ϕ for a basin is introduced in this research. Linear regression equations without intercept are developed based on two monthly naturalized flow sequences from control point pairs, and then K and ϕ are optimized from the assumption that drainage ratios with K and ϕ are equal to slopes (B) of the linear equations.

The reasons why a regional statistical method would generate transfer errors are revealed based on statistical concepts in this research. A new approach to select statistical parameters is suggested to address this issue on the method.

The performances of both the proposed methods are evaluated by the NSE between the developed flow data and naturalized flow dataset at the same control points. The following conclusions are derived from the case studies:

- The drainage area ratio (DAR) without a correction factor K and an exponent ϕ leads to 24 unsatisfactory (NSE below 0.5) performances and 5 unacceptable (negative value of NSE) performances.
- But, the DAR with K and ϕ , derived from a new method in this research, results in only 2 unsatisfactory performances out of 357 performances in total.
- The new method is simple to implement and can be an additional option in the WRAP model.

In the regional statistic method, the performances by a new approach and existing approach are very similar, if statistical parameters have relatively strong linear relationship with regional variables like the Sabine and Neches WAM. However, if not, the new approach tends to enhance the performance of the regional statistical method for flows in the GSA and Trinity WAMs.

10.3 Disaggregation of Monthly to Daily Naturalized Flows Using the SWAT Model

The daily WRAP simulation model needs daily flow patterns for disaggregation of monthly naturalized flows to daily while preserving the monthly volumes. The SWAT models are developed to synthesize the daily flow patterns at all relevant control points for the three WAMs. The models are calibrated alternatively with two different available datasets: (1) WAM monthly naturalized flow datasets for periods-of-analysis and (2) USGS daily recorded flow data for un-impacted periods (1951-1960). Each of these datasets have pros and cons in the daily SWAT model calibration.

Two different daily naturalized flow sequences, disaggregated based on two different daily flow patterns that are synthesized by the two calibrated SWAT models, are evaluated by the following four methods: NSE, flow frequency metric, DHRAM (IHA), and annual median flow duration curves. These four methods represent streamflow timing, flow regime, hydrologic characteristics, and overall hydrologic state of a flow, respectively.

The daily flow sequences, used for the comparison of the disaggregated daily naturalized flows, are the USGS daily recorded data for un-impacted periods. The selected un-impacted periods are commonly before 1960. More appropriated disaggregated daily flow sequences are selected for daily WRAP models through the four different evaluations.

The case studies support the following conclusions:

- SWAT models, calibrated with either monthly naturalized or daily recorded flows are able to simultaneously synthesize daily flow patterns that are used for the daily WRAP model at multiple-sites.
- More appropriate daily synthesized flow sequences for the daily WRAP model can be selected through a systematic evaluation strategy developed in this research, depending on basin characteristics such as soil and land cover, rainfall patterns, and interaction between surface and groundwater.
- In this research, the SWAT models, calibrated with USGS daily recorded flows for un-impacted periods are more appropriate for the Sabine and Neches WAMs,

representing conditions of high rainfall, forested watershed, minimal surface/groundwater interactions, and relatively low population densities.

- The SWAT model, calibrated with monthly naturalized flows for period-of-analysis is more appropriate for the GSA WAM, representing conditions of lower rainfall, major surface/groundwater interactions, and much higher population densities.
- The combination of the SWAT model and the new systematic evaluation strategy can be an effective approach for disaggregating monthly naturalized flow to daily.

10.4 Evaluating Environmental Flow Standards

The Dundee Hydrological Regime Alteration Method (DHRAM), which incorporates the Indicators of Hydrologic Alteration (IHA) methodology, assesses the hydrological alterations of daily river flow sequences between user-defined impacted and un-impacted periods. USGS recorded flows are divided into the two periods before and after dam construction and associated initial reservoir storage that is considered to be the most significant point-specific human influence for the DHRAM analysis. Naturalized flows are theoretically un-impacted flows, and regulated flows are impacted flows in the WAM system. The hydrological alterations between both the flows at the same sites are also evaluated by the DHRAM. If there are the similarities between the two results of the DHRAM analyses at the same site, the capabilities of the WRAP model are employed to quantify the alterations of river flow regime under SB3 environmental flow standards.

SB3 environmental flow standards (EFS) do not impact existing water rights because these standards are assigned junior priorities. The environmental flow standards were established to preserve the current flow regimes from future water right applicants. However, for purposes of comparison, alternative daily WRAP simulations are performed with the priority of the EFS set as the most senior versus most junior water rights in the WAMs.

The daily WRAP simulation model computes sequences of regulated and unappropriated flows based on naturalized flow datasets for the hydrologic period-of-analysis. The impacts on future water rights by EFS are evaluated by comparing

unappropriated flows at a control point with and without the EFS. Unappropriated flow frequency metrics with and without EFS are used for the evaluation.

Also as another comparative evaluation, the alterations between naturalized and regulated flows are assessed with the EFS assigned the most senior versus junior priorities. The alterations between alternative flows generated by the WRAP simulation model with the different EFS priorities are compared by two different methods, the DHRAM method and annual mean flow duration curve. The control points where SB3 EFSs were incorporated are chosen for the comparison of flow alterations because flow alterations at the control points with EFSs are typically more apparent than at other control points.

The case studies lead to the following conclusions:

- The results of the DHRAM analyses based on USGS recorded daily flows divided into the un-impacted and impacted periods are closely similar to the results based on the naturalized and regulated flows for periods-of-analysis at the same sites for the Sabine and Neches WAMs. However, there are differences in the two comparative results of the DHRAM analysis for the GSA WAM. This is apparently attributed to return flows from municipal ground water use and the Medina Lake operation.
- These studies indicate that the alteration of river flow regime under SB3 environmental flow standards can be evaluated based on naturalized, regulated, and unappropriated flows from the WRAP model simulations.
- Unappropriated flows are slightly decreased by SB3 environmental flow standards at most control points in the three WAMs.
- Although the environmental flow standards have most senior priorities than all other water rights, these cannot contribute to recover the flow regimes of the regulated flows to the naturalized flows in the Sabine and Neches WAMs. This is because the environmental flow targets are engaged based on the regulated flows, and the volumes of the regulated flows considerably decrease than the naturalized flows due to consumptive water rights, such as diversions and refilling reservoirs.
- The environmental flow standards with the most senior priority significantly contribute to restore the flow regimes of the regulated flows to the naturalized flows

in the GSA WAM. This is attributed to relatively a few consumptive water rights than other two basins. More than 30 percent of the total authorized consumptive diversions from the two rivers and their tributaries are located below their confluence (Wurbs *et al.*, 2014c). Water rights for hydropower, modeled as diversions with 100% return flows, account for 83.1 percent of the total diversions (Wurbs *et al.*, 2014c).

The results of this research can provide feedback regarding the existing SB3 environmental flow standards and useful information for establishing environmental flow standards in other river basins. The research can contribute to expanding and refining WRAP/WAM modeling capabilities to support addressing environmental flow considerations in river system management while meeting growing water uses.

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